

Block 4 Report

Sydney Metro C&SW - Traffic and Interchange Monitoring

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Block 4 Report

Sydney Metro C&SW - Traffic and Interchange Monitoring

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Terms and abbreviations

Term	Definition
AECOM	AECOM Australia Pty Ltd
Block 4	The fourth study block of the traffic and interchange monitoring program
BOAM	Bus Opal Assignment Model
CBD	Central Business District
СоА	Conditions of Approval
Condition D12	Refers to Item D12 of the Sydney Metro City & Southwest Chatswood to Sydenham conditions of approval, which specifies requirements for traffic operational monitoring of the Sydney Metro City & Southwest Chatswood to Sydenham.
CSSI	Critical State Significant Infrastructure
IAP	Interchange Access Plan
LOS	Level of Service
post-opening	denotes post-opening scenarios of the Sydney Metro City & Southwest line operating between Chatswood to Sydenham
pre-opening	denotes pre-opening scenarios of the Sydney Metro City & Southwest line operating between Chatswood to Sydenham
PTIPS	Public Transport Information and Priority Systems
SCATS	Sydney Coordinated Adaptive Traffic System
SIDRA Intersection	SIDRA Intersection modelling software, the modelling software used to assess the traffic performance.
SHB	Sydney Harbour Bridge
Sydney Metro	A New South Wales Government Agency constituted under the <i>Transport Administration Act 1988 (NSW)</i>).
Sydney Metro City & Southwest	The metro railway between Chatswood and Bankstown, including 15.5 kilometres of twin metro railway tunnels from Chatswood to Marrickville under Sydney Harbour.
Sydney Metro Northwest	The former Northwest Rail Link, i.e. operating metro railway between Tallawong Station at Rouse Hill and Chatswood.
Sydney Metro West	The metro railway that will connect the Sydney CBD and Parramatta, linking communities along the way with a new underground railway.
Sydney Metro Western Sydney Airport	The metro railway that will link St Marys to the Western Sydney International (Nancy Bird Walton) airport and the Aerotropolis.
TfNSW	Transport for NSW (A New South Wales Government Agency constituted under the <i>Transport Administration Act 1988 (NSW)</i>).
the Project	Traffic and interchange monitoring assessments for the Sydney Metro City & Southwest Chatswood to Sydenham
TCS	Traffic Control Signal
TSN	Transit Stop Number

1.0 Introduction

This section provides an introduction of the traffic and interchange monitoring for the Sydney Metro City & Southwest between Chatswood Station and Sydenham Station (the Project), including the project overview, project objectives and overall scope of works covered under this Project.

1.1 **Project overview**

Sydney Metro is the largest public transport project in Australia, designed to address congestion, enhance connectivity, and meet the evolving needs of Sydney's population and economy. It encompasses four major metro lines: Sydney Metro Northwest, Sydney Metro West, Sydney Metro Western Sydney Airport, and Sydney Metro City & Southwest.

AECOM Australia Pty Ltd (AECOM) has been appointed by Sydney Metro to conduct traffic and interchange monitoring assessments for the Sydney Metro City & Southwest between Chatswood Station and Sydenham Station (the Project).

The purpose of this assessment is to evaluate the impact of the Sydney Metro City & Southwest (Chatswood to Sydenham) operations on the nine stations and their surrounding intersections and interchange facilities. The study involves evaluating the performance of these intersections and interchange both before and after the introduction of the metro line. This assessment is crucial for fulfilling the requirements of the Critical State Significant Infrastructure (CSSI) application Conditions of Approval (CoA) overseen by the NSW Department of Planning and Environment.

Traffic and interchange monitoring will be conducted in six study blocks, spanning a period of over 12months before the commencement of the CSSI operations (pre-opening) and 12-months after the commencement (post-opening). This comprehensive monitoring approach will provide insights into the traffic and interchange dynamics during different stages of the Sydney Metro City & Southwest Line (Chatswood to Sydenham), allowing for a thorough and robust impact assessment.

Figure 1-1 presents a timeline overview of the study blocks, highlighting the specific periods under observation. Sydney Metro City & Southwest (Chatswood to Sydenham) commenced operations on 19 August 2024.



Figure 1-1 Traffic and interchange monitoring program

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The Sydney Metro City & Southwest Chatswood to Sydenham – Traffic and Interchange Operation Monitoring report (this report) has been prepared to meet the requirements of Condition D12 of the CoA (outlined in Section 2.2).

This report provides traffic and interchange operation assessments of the nine stations along the Sydney Metro City & Southwest Line (Chatswood to Sydenham) during the monitoring timeframe of September 2024 (Block 4). Block 4 represents for first study block following the commencement of operations of the Sydney Metro City & Southwest Line (Chatswood to Sydenham).

1.3 Scope of this study

The overall scope of works for the Block 4 study covers the following:

- **Traffic monitoring**: Intersection surveys (including re-surveys) were conducted in August/September 2024, including:
 - classified intersection count surveys conducted continuously for a one-week period, including light vehicles, heavy vehicles, buses, cyclist and pedestrian counts
 - vehicular queue length surveys (at the signal change to green for signalised intersections and aggregated every two minutes for priority intersections) conducted for two hour periods around the identified weekday AM, PM and weekend peak hours.
- **Transport interchange monitoring:** Chatswood Station, Crows Nest Station, Victoria Cross Station, Barangaroo Station, Waterloo Station and Sydenham Station were considered for the Block 4 study. Interchange operation surveys were conducted at these six stations continuously for the same one-week period as the intersection surveys in August/September 2024. Interchange operation surveys collected the following information for taxi, bus stop, accessible parking and kiss and ride facilities at each station:
 - vehicle counts
 - vehicle occupancy (boarding and alighting passengers only)
 - vehicle dwell time
 - vehicle queue length outside the bay on a lane-by-lane basis.
- **Site observations**: Site visits were undertaken in conjunction with the traffic and interchange operation monitoring for at least one weekday AM peak period, one weekday PM peak period, and one weekend peak period at each station.
- Intersection assessment: To assess the intersection operation performance during Block 4, a combination of isolated and network traffic modelling assessments were undertaken using SIDRA Intersection modelling software (SIDRA Intersection). The following data was obtained from Sydney Metro for developing the SIDRA Intersection models:
 - Sydney Coordinated Adaptive Traffic System (SCATS) traffic detector count data
 - SCATS traffic signal data and sub-systems information.
- **Stakeholder consultation:** Key findings of the Block 4 study were provided to Sydney Metro and the following key stakeholders in January 2025 for review and feedback:
 - Transport for NSW (TfNSW)
 - Willoughby City Council
 - North Sydney City Council
 - City of Sydney
 - Inner West Council.

Additionally, Block 4 study findings were presented to TfNSW, North Sydney Council and Inner West Council. Appendix A provides the minutes from these stakeholder meetings.

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1.4 Structure of this report

This report is structured as follows:

- Section 1.0 provides an introduction to the Project
- Section 2.0 provides the context and background of the Project
- Section 3.0 outlines the study area of the Project
- Section 4.0 describes the methodology adopted for the traffic and interchange operation assessments
- Section 5.0 details the traffic monitoring and intersection performance
- Section 6.0 details the interchange monitoring performance
- Section 7.0 provides a summary of the traffic and interchange monitoring.

2.0 Context and background

This section provides an overview of the strategic context of the Project within the overall Sydney Metro program and the background of the CSSI CoA for the Sydney Metro City & Southwest Line (Chatswood to Sydenham).

2.1 Context

Sydney Metro is Australia's largest public transport project, aiming to alleviate congestion, improve connectivity, and support the growing population and economic needs of Sydney. The main objectives of Sydney Metro are to enhance the overall transport experience, establish a robust and sustainable transport system, increase public transport usage and enhance the resilience of the transport network.

By 2032, Sydney Metro is expected to create a network of four metro lines (Northwest, City & Southwest, West, and Western Sydney Airport), spanning 113 kilometres, and encompassing 46 stations.

2.1.1 Sydney Metro Northwest

Sydney Metro Northwest marked the initial phase of the Sydney Metro project, commencing operations in May 2019. Spanning approximately 36 kilometres from Tallawong to Chatswood, this line consists of 13 stations.

2.1.2 Sydney Metro City & Southwest

Sydney Metro City & Southwest further extends the constructed Sydney Metro Northwest from Chatswood to Bankstown via the Sydney Central Business District (CBD) with 30 kilometres of metro rail. Sydney Metro City & Southwest between Chatswood and Sydenham commenced operations on 19 August 2024, with seven new metro stations and 11 upgraded stations as shown in Figure 2-1. This will establish connectivity between metro stations in the city and southwest with those further west, including future metro stations on the Sydney Metro West and Sydney Metro Western Sydney Airport.

Sydney Metro City & Southwest project consists of two phases: Chatswood to Sydenham; and Sydenham to Bankstown. This study focuses on the assessments for the Chatswood to Sydenham phase of the Sydney Metro City & Southwest project.



Figure 2-1 Sydney Metro City & Southwest overview

2.1.3 Sydney Metro West

Sydney Metro West is an upcoming 24-kilometre metro line that will establish a vital connection between Greater Parramatta and the Sydney CBD, linking the communities along its route. This line will incorporate nine new metro stations, located at key destinations including Westmead, Parramatta, Sydney Olympic Park, The Bays Precinct, and the Sydney CBD.

Construction for the Sydney Metro West project commenced in 2020 and is currently in progress.

2.1.4 Sydney Metro Western Sydney Airport

Sydney Metro Western Sydney Airport line is an upcoming 23-kilometre line and will link the new Western Sydney International (Nancy-Bird Walton) Airport with the Western Sydney Aerotropolis, and St Marys. The Sydney Metro Western Sydney Airport project includes the construction of six new metro stations and will provide connectivity to the existing Sydney Trains suburban T1 Western Line.

Construction for the Sydney Metro Western Sydney Airport project commenced in 2020 and is currently in progress.

2.2 Background

On 10 January 2017, the NSW Minister for Planning granted approval to the CSSI application for the Sydney Metro City & Southwest Chatswood to Sydenham. The infrastructure approval, which is regulated under Section 115ZB of the *Environmental Planning and Assessment Act 1979*, is subject to the Minister's conditions of approval for the CSSI.

The Conditions of Approval are administered by the NSW Department of Planning and Environment (previously the NSW Department of Planning, Industry and Environment) and delivered by the Proponent – Sydney Metro.

Part D of the Conditions of Approval outlines conditions for environmental management during operations of the project. Condition D12 specifies the requirement for traffic operational monitoring of the Project as per the following requirement:

"Traffic on local roads around each station must be monitored 12 months before the CSSI commences operation and for a period of no less than 12 months after commencement of operation. If monitoring indicates unacceptable traffic intrusion on local roads/streets as a result of operation of the CSSI beyond those that could reasonably be predicted in the EIS and/or Interchange Access Plan(s) in Condition E92, appropriate traffic management measures to mitigate the monitored impacts must be implemented following consultation with the Sydney Coordination Office and Relevant Road Authorities."

3.0 Study area

This section provides an overview of the study area for both traffic and interchange monitoring, which was identified by Sydney Metro in consultation with key stakeholders (as listed in Section 1.3) during late 2022.

3.1 Overview

The Sydney Metro City & Southwest Line (Chatswood to Sydenham) includes a total of nine stations. For ease of referencing, each station has been assigned a three-character identifier based on the TfNSW Asset Reference Codes Register¹. Table 3-1 displays the list of these stations along with their corresponding identifiers.

Table 3-1 Station three-character identifiers

Station	Station ID ¹
Chatswood	CIMD3
Chatswood Dive Site ²	
Crows Nest	CST
Victoria Cross	VIC
Barangaroo	BGU
Martin Place	MPL
Gadigal (formerly Pitt Street)	PIT
Central	CEN
Waterloo	WLO
Sydenham	SYD

Notes:

1. <u>TfNSW Asset Codes Register</u> TS 01499:2.00 Version 2 has been used as a reference.

2. Chatswood Dive Site is not a station

3. CWD refers to Chatswood Dive Site in the context of the traffic assessment and Chatswood Station in the context of the interchange operation monitoring assessment.

The Crows Nest Station, Victoria Cross Station, Barangaroo Station, Waterloo Station and Sydenham Station had both intersection and interchange monitoring for Block 4, while the other stations had either traffic monitoring or interchange monitoring.

Table 3-2 outlines the type of assessment undertaken for each station in the Block 4 study.

|--|

Station	Traffic monitoring	Interchange monitoring	Remarks	
Chatswood	×	\checkmark	No changes to road network	
Chatswood Dive Site	~	×	No new kerbside usage proposed	
Crows Nest	~	~	Interchanges operational from Block 4	
Victoria Cross	~	\checkmark	Interchanges operational from Block 4	
Barangaroo	~	~	Interchanges operational from Block 4	
Martin Place	\checkmark	×	No new kerbside usage proposed	

Station	Traffic monitoring	Interchange monitoring	Remarks	
Gadigal	~	×	No new kerbside usage proposed	
Central	~	×	No new kerbside usage proposed	
Waterloo	~	<	Interchanges operational from Block 4	
Sydenham	~	~	Nil	

3.2 Traffic monitoring

The study area for traffic monitoring comprises a total of 65 intersections spread across the nine stations. To facilitate ease of reference, each intersection is assigned two unique identifiers:

- Intersection ID: A five-character code formed by combining the three-character identifier of the corresponding station (as listed in Table 3-1) with the index of the intersection within the study area surrounding that station. For example, CEN03 represents the third intersection in the Central Station study area.
- S.ID: A two-character identifier used to index all intersections within the Project study area.

Table 3-3 outlines each intersection's S.ID, Intersection ID, traffic control signal (TCS) ID designated by TfNSW, name, and control type. Of the 65 intersections within the study area, 63 intersections were assessed using SIDRA Intersection modelling during Block 4. The following intersections were excluded from the Block 4 analysis:

- CWD02 Pedestrian Bridge Crossing along Mowbray Road: This location was solely included in traffic surveys for data collection and was not modelled
- CEN04 New Pedestrian Mid-block Crossing at Randle Lane: This location was not operational during Block 4 and was therefore excluded from the analysis.

Figure 3-1 to Figure 3-9 depict the location of each intersection within each station's study area based on their Intersection ID.

S.ID	Intersection ID	TCS ID	Intersection name	Intersection control type
01	CWD01	3037	Mowbray Road/Hampden Road	Signal
02	CWD02	-	Pedestrian Bridge Crossing along Mowbray Road	Pedestrian only - Bridge Crossing
03	CST01	768	Pacific Highway/Albany Street	Signal
04	CST02	767	Pacific Highway/Oxley Street	Signal
05	CST03	766	Pacific Highway/Hume Street	Signal
06	CST04	765	Pacific Highway/Falcon Street/ Shirley Road	Signal
07	CST05	-	Clarke Street/Oxley Street	Priority - Give Way
08	CST06	-	Clarke Street/Hume Street	Priority - Give Way
09	CST07	-	Clarke Street/Willoughby Road	Priority - Give Way
10	CST08	516	Albany Street/Willoughby Road	Signal
11	CST09	-	Albany Street/Oxley Street	Roundabout
12	CST10	-	Albany Street/Clarke Lane	Priority - Give Way

Table 3-3 Traffic assessment intersections

S.ID	Intersection ID	TCS ID	Intersection name	Intersection control type
13	CST11	-	Oxley Street/Clarke Lane	Priority - Stop
14	CST12	-	Hume Street/Clarke Lane	Priority - Stop
15	CST13	763	Pacific Highway/Alexander Street	Signal
16	CST14	764	Falcon Street/Alexander Street	Signal
17	VIC01	1206	Pacific Highway/Berry Street	Signal
18	VIC02	874	Miller Street/Berry Street	Signal
19	VIC03	1156	Miller Street/McLaren Street	Signal
20	VIC04	630	Pacific Highway/Miller Street	Signal
21	BGU01	-	Hickson Road/Towns Place	Priority - Give Way
22	BGU02	-	Dalgety Road/Towns Place	Roundabout
23	BGU03	-	Kent Street/Argyle Street	Priority - Give Way
24	BGU04	4272	Pedestrian Mid-block Crossing at Kent Street near Gas Lane	Pedestrian only - Signal
25	BGU05	4272	Kent Street/Sydney Harbour Bridge (SHB) On-ramp	Signal
26	BGU06	4625	Hickson Road/Napoleon Street/ Sussex Street	Signal
27	BGU07	308	Margaret Street/Kent Street/ Napoleon Street	Signal
28	BGU08	319	Margaret Street/Clarence Street	Signal
29	BGU09	3042	Margaret Street/York Street	Signal
30	BGU10	3939	Pedestrian Mid-block Crossing at Sussex Street under Exchange Place	Pedestrian only - Signal
31	BGU11	4109	Pedestrian Mid-block Crossing at Kent Street near Margaret Street	Pedestrian only - Signal
32	BGU12	310	Sussex Street/Erskine Street	Signal
33	BGU13	307	Kent Street/Erskine Street	Signal
34	BGU14	284	Sussex Street/King Street	Signal
35	BGU15	283	Kent Street/King Street	Signal
36	BGU16	-	New Pedestrian Mid-block Crossing at Hickson Road (north of Metro Station)	Pedestrian only - Zebra
37	BGU17	-	New Pedestrian Mid-block Crossing at Hickson Road (south of Metro Station)	Pedestrian only - Zebra
38	BGU18	305	Shelley Street/Erskine Street	Signal
39	MPL01	244	Hunter Street/Castlereagh Street/ Bligh Street	Signal
40	MPL02	302	Hunter Street/Elizabeth Street/ Chifley Square	Signal
41	MPL03	1412	Bent Street/Bligh Street	Signal

Signal

Bent Street/Phillip Street

MPL04

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S.ID	Intersection ID	TCS ID	Intersection name	Intersection control type	
43	MPL05	245	Pedestrian Mid-block Crossing at Castlereagh Street	Pedestrian only - Signal	
44	MPL06	287	Pedestrian Mid-block Crossing at Elizabeth Street	Pedestrian only - Signal	
45	PIT01	2312	Pitt Street/Bathurst Street	Signal	
46	PIT02	2281	Castlereagh Street/Bathurst Street	Signal	
47	PIT03	250	Park Street/Castlereagh Street	Signal	
48	PIT04	235	Park Street/Pitt Street	Signal	
49	CEN01	293	Elizabeth Street/Eddy Avenue	Signal	
50	CEN02	293	Elizabeth Street/Foveaux Street	Signal	
51	CEN03	-	Elizabeth Street/Cooper Street	Priority - Give Way	
52	CEN04	-	New Pedestrian Mid-block Crossing at Randle Lane	Pedestrian only - Zebra	
53	CEN05	2916	Elizabeth Street/Randle Street	Signal	
54	WLO01	47	Botany Road/Raglan Street/ Henderson Road	Signal	
55	WLO02	5057	Raglan Street/Cope Street	Signal	
56	WLO03	137	Botany Road/Wellington Street/ Buckland Street	Signal	
57	WLO04	-	Cope Street/Wellington Street	Priority - Stop	
58	WLO05	55	Wyndham Street/Henderson Road	Signal	
59	WLO06	-	New Pedestrian Mid-block Crossing at Cope Street	Pedestrian only - Zebra	
60	SYD01	3320	Railway Parade/Gleeson Avenue	Signal	
61	SYD02	1152	Burrows Avenue/Gleeson Avenue	Signal	
62	SYD03	-	Burrows Avenue/George Street	Priority - Give Way	
63	SYD04	4946	Railway Parade/Sydenham Road	Signal	
64	SYD05	-	Marrickville Road/Buckley Street	Priority - Give Way	
65	SYD06	-	Sydenham Road/Buckley Street	Priority - Give Way	



Figure 3-1 Chatswood Dive Site traffic study area



Figure 3-2 Crows Nest Station traffic study area



Figure 3-3 Victoria Cross Station traffic study area



Figure 3-4 Barangaroo Station traffic study area



Figure 3-5 Martin Place Station traffic study area



Figure 3-6 Gadigal Station traffic study area



Figure 3-7 Central Station traffic study area



Figure 3-8 Waterloo Station traffic study area



Figure 3-9 Sydenham Station traffic study area

The transport interchange monitoring study area includes taxi, bus stop, accessible parking and kiss and ride facilities located near the stations along the City & Southwest Line (Chatswood to Sydenham). In Block 4, surveys were conducted for interchange facilities that were operational, namely Chatswood Station, Crows Nest Station, Victoria Cross Station, Barangaroo Station, Waterloo Station and Sydenham Station.

Similar to the intersections in the traffic study area, a five-character identifier was assigned to each taxi, bus stop, kiss and ride and accessible parking facility for ease of referencing, with the first three-characters matching the station identifiers in Table 3-1. The fourth character identifies the type of interchange facility and the fifth character indexes it.

Table 3-4 outlines the interchange facilities assessed in the Block 4 study, including the associated type, identifier, station, street and side of road location, and number of bays.

Туре	ID	Station	Street	Side of road	Number of bays
Kiss and ride	CWDK1	Chatswood	Railway Street	West	1
Kiss and ride	CWDK2	Chatswood	Albert Avenue	North	2
Kiss and ride	CWDK3	Chatswood	Endeavour Street	North	2
Taxi	CWDT1	Chatswood	Victoria Avenue	North	11
Taxi	CWDT2	Chatswood	Endeavour Street	North	2
Kiss and ride	CSTK1	Crows Nest	Oxley Street	East	3
Kiss and ride	CSTK2	Crows Nest	Oxley Street	West	3
Kiss and ride	CSTK3	Crows Nest	Clarke Street	South	1
Taxi	CSTT1	Crows Nest	Clarke Street	South	2
Bus	VICB1	Victoria Cross	Miller Street	East	2
Bus	VICB2	Victoria Cross	Miller Street	West	2
Kiss and ride	VICK1	Victoria Cross	McLaren Street	North	5
Taxi	VICT1	Victoria Cross	McLaren Street	North	2
Accessible parking	VICA1	Victoria Cross	McLaren Street	North	1
Bus	BGUB1	Barangaroo	Hickson Road	East	1
Bus	BGUB2	Barangaroo	Hickson Road	West	1
Kiss and ride	BGUK1	Barangaroo	Hickson Road	West	1
Тахі	BGUT1	Barangaroo	Hickson Road	West	1
Bus	WLOB1	Waterloo	Raglan Street	South	2
Kiss and ride	WLOK1	Waterloo	Cope Street	West	4
Тахі	WLOT1	Waterloo	Cope Street	West	2
Accessible parking	WLOA1	Waterloo	Cope Street	West	1
Bus ¹	SYDB1	Sydenham	Railway Parade	South	3
Kiss and ride	SYDK1	Sydenham	Burrows Avenue	North	4
Kiss and ride	SYDK2	Sydenham	Sydenham Road	East	2
Taxi	SYDT1	Sydenham	Burrows Avenue	North	2

Table 3-4 Block 4 – interchange facilities

Туре	ID	Station	Street	Side of road	Number of bays
Accessible parking	SYDA1	Sydenham	Bolton Street	North	2
Notes:					

1. SYDB1 encompasses transit stop number (TSN) 220421, TSN 2204125 and TSN 220450.

Figure 3-10 to Figure 3-15 depict the location of each taxi, bus stop, accessible parking and kiss and ride facility assessed surrounding the six interchanges.



Figure 3-10 Chatswood Station interchange study area



Figure 3-11 Crows Nest Station interchange study area



Figure 3-12 Victoria Cross Station interchange study area



Figure 3-13 Barangaroo Station interchange study area



Figure 3-14 Waterloo Station interchange study area



Figure 3-15 Sydenham Station interchange assessment study area

4.0 Assessment methodology

This section details the traffic and transport interchange monitoring assessment methodology undertaken for the intersections within study area and the park and ride facilities surrounding the stations identified in Section 3.2 and Section 3.3, respectively.

4.1 Traffic monitoring

Figure 4-1 provides an overview of the adopted methodology for the traffic monitoring, with further clarifications and details provided in the subsequent sections.



Figure 4-1 Traffic assessment methodology overview

4.1.1 Traffic surveys

Classified intersection counts were undertaken for 64 of the 65 study area intersections (as outlined in **Section 3.2**). The traffic surveys were carried out over a one-week period, and the data was aggregated in 15-minute intervals. In cases where data was corrupted or unavailable due to vandalism, re-surveys were conducted. The survey dates were as follows:

- Traffic surveys: Saturday 31 August 2024 to Friday 6 September 2024
- Re-surveys: Monday 16 September 2024 to Sunday 22 September 2024.

During the traffic surveys, data was gathered for various vehicle types including light vehicles, heavy vehicles, and buses, as well as for cyclists and pedestrians. In addition, queue lengths were also documented during the traffic surveys to aid in validating the SIDRA Intersection models.

AECOM conducted site observations in conjunction with the traffic surveys, ensuring at least one observation was carried out for each intersection during each peak period specified in Section 4.1.2 (excluding the Monday and Friday). The site observations were conducted to observe various aspects, including vehicle behaviours, any changes in lane geometry or capacity, and the condition of the traffic survey cameras to ensure that they were properly set up and not vandalised.

SCATS traffic detector count data was provided by Sydney Metro, for the same dates the traffic surveys were undertaken. The traffic survey data was reviewed against the SCATS traffic detector count data to identify any potential outliers. Intersections with traffic survey volumes greater than or less than 10 per cent of the SCATS volumes underwent additional investigation and/or recounting of the traffic surveys. Once the traffic survey data were reviewed and finalised, additional data analysis was conducted as detailed in the subsequent sections.

4.1.2 Peak hour identification

Each intersection was modelled as either an isolated site or as part of a network, as described in Appendix B. In the case of intersections modelled as an isolated site, the peak hour was determined by

considering the total hourly volume (light vehicles, heavy vehicles and buses) at the intersection. For intersections modelled as part of a network, the peak hour was determined by considering the total hourly volume across the network at approaches connecting to the external network.

In consultation with Sydney Metro, the time periods used to determine the intersection or network peak hour were updated for Block 3 onwards to consider the 24-hour traffic monitoring period during the survey period listed in Section 4.1.1. The revised time periods are listed below:

- weekday AM peak: 3am to 12pm
- weekday PM peak: 12pm to 3am
- weekend peak: 3am Saturday to 3am Monday.

It is important to note that the identified peak hours vary between different locations. However, the peak hours fall within the time periods listed above.

4.1.3 Network flow diagrams

A review was undertaken to identify any variations in peak hour traffic volumes between mid-blocks connecting adjacent intersections within the same network. These variations were primarily due to minor counting discrepancies or due to side streets, property and parking access. Survey volumes were used for the intersection modelling. Additionally, considering the fixed schedule of bus routes, adjustments were made to bus volumes whenever large discrepancies were observed.

The resulting peak hour volumes were utilised as the turning volume inputs for the SIDRA Intersection models. The network flow diagrams used to inform the traffic and pedestrian volume inputs for SIDRA Intersection modelling are included in Appendix C.

4.1.4 SCATS signal and sub-systems data

In addition to the SCATS detector count data, SCATS traffic signal data was also provided for each intersection during their respective peak hours, which aligned with the traffic survey dates.

The SCATS traffic signal data included historical information on the signal phase sequence and signal phase time frequency, as well as sub-system information for signalised intersections modelled as a part of a network. Furthermore, the signal phase sequence was reviewed against traffic survey footage to determine if any signal phases were not executed or ran in a different order. Moreover, the traffic survey footage was also examined to ascertain whether the early cut-off or late-start movements observed during site visits also occurred during the peak hours modelled.

4.1.5 SIDRA Intersection modelling

The performance of the intersections was assessed using either the site or network function (refer to Appendix B) of the SIDRA Intersection software, adopting the peak hour volumes and SCATS traffic signal data. Detailed SIDRA Intersection modelling was conducted for the intersections within the study area. The geometry of the intersections was established using desktop aerial imagery from sources such as Nearmap and Google Streetview, which was then validated through on-site observations. The models were specifically developed for the identified peak hours within the peak periods (Section 4.1.2), incorporating the peak volume inputs derived from the network flow diagrams (Section 4.1.3), as well as the SCATS signal data and sub-systems information (Section 4.1.4).

The modelled queues were validated against the queue length surveys and traffic survey footage.

4.1.6 Intersection performance assessment

The standard measure of intersection performance is vehicle delay, which is used to assess the efficiency of an intersection. SIDRA Intersection adopts the TfNSW Traffic Modelling Guidelines which categorises average intersection delay into six bands of average delay per vehicle (seconds per vehicle). These bands are determined based on the criteria outlined in Table 4-1. By analysing the average delay, SIDRA Intersection determines the level of service (LOS) for the intersection, a measure of the intersection performance.

LOS	Average delay (seconds per vehicle)	Criteria for traffic signals	Criteria for give way and stop signs
А	< 14	Good operation	Good operation
В	15 to 28	Good operation with acceptable delays and spare capacity	Good operation with acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	42 to 56	Near capacity	Near capacity and accident study required
E	57 to 70	At capacity; at signals, incidents will cause excess delays	At capacity, requires other control mode
F	> 70	Extra capacity required	At capacity, requires other control mode

Table 4-1 Intersection LOS criteria

Source: TfNSW Traffic Modelling Guidelines, LOS definitions for vehicles (NSW method) based on delay only

It is noted that the critical movement for LOS at a roundabout or priority-controlled intersection is the movement with the worst delay, whereas for a signalised intersection, the average delay over all movements is adopted.

4.2 Transport interchange monitoring

Figure 4-2 provides an overview of the adopted methodology for the interchange monitoring, with further clarifications and details are provided in the subsequent sections.



Figure 4-2 Interchange assessment methodology overview

4.2.1 Interchange surveys

Interchange surveys were undertaken at taxi, bus stop, kiss and ride and/or accessible parking facilities located at Chatswood Station, Crows Nest Station, Victoria Cross Station, Barangaroo Station, Waterloo Station and Sydenham Station (as outlined in Section 3.3).

The interchange surveys were carried out over a one-week period, similar to the intersection surveys. In cases where data was corrupted or unavailable due to vandalism, resurveys were conducted. The survey dates were as follows:

- Interchange Surveys: Saturday 31 August 2024 to Friday 6 September 2024
- Re-surveys: Monday 16 September 2024 to Sunday 22 September 2024.

The key data captured as part of the interchange surveys includes:

- vehicle counts
- vehicle occupancy (boarding and alighting passengers only)
- vehicle dwell time
- vehicle queue length outside the bay.

In consultation with Sydney Metro, the time periods used to determine the interchange peak demand have been updated for Block 3 onwards to consider the 24-hour traffic monitoring period during the survey period listed in Section 4.1.1. The revised time periods are listed below:

- weekday AM peak: 3am to 12pm
- weekday PM peak: 12pm to 3am
- weekend peak: 3am Saturday to 3am Monday.

Site observations were completed in conjunction with the interchange surveys, ensuring at least one observation was carried out for each interchange facility during the above time periods. These observations aimed to monitor several aspects, such as kerbside lane usage, queuing outside the bays and the condition of the interchange survey cameras, ensuring they were correctly set up and not subject to vandalism.

4.2.2 Aggregation and analysis

The interchange survey data was consolidated and analysed, categorising the data based on facility type (taxi, bus stop, kiss and ride or accessible parking) to understand usage patterns at the interchange facilities near the stations. A high-level exploratory analysis of the combined data was conducted to identify the daily vehicle trends for the key data types outlined in Section 4.2.1.

To ensure the accuracy and reliability of the findings, the identified trends were compared with the survey footage. In cases where discrepancies were detected, the survey data was recounted and/or rechecked to provide reliable results. The findings from this analysis are reported in Section 6.0.

5.0 Traffic monitoring and intersection performance

This section summarises the traffic monitoring and intersection performance outputs from traffic survey data and SIDRA Intersection modelling undertaken across the Block 4 study area.

The SIDRA Intersection movement summary outputs for all modelled intersections during each peak hour are shown in Appendix D.

5.1 Chatswood Dive Site

The Chatswood Dive Site was used as a temporary underground site facilitating excavation and construction works for the City & Southwest Line tunnel portal from Chatswood Station. Although not accessible to the general public, the Chatswood Dive Site facilitates the movement of workers and equipment to access the underground areas where crucial tunnelling and other metro construction operations take place. Following the opening of Sydney Metro City & Southwest Line (Chatswood to Sydenham), the Chatswood Dive Site will be used as a service facility for the operation of the Sydney Metro rail line between Chatswood and the Sydney CBD (and beyond).

The Chatswood Dive Site is located south of Chatswood Station and north of Artarmon Station, bound by the Pacific Highway (A1), Mowbray Road and Nelson Street in Chatswood. Bus services are available within approximately 200 metres west of the Chatswood Dive Site on the Pacific Highway (A1) and Mowbray Road. Artarmon Station, approximately 600 metres south of the Chatswood Dive Site, offers the nearest rail service. The bridge crossing along Mowbray Road over the rail line connects residents to the east with the Pacific Highway (A1), facilitating walking and cycling in addition to general traffic.

The Chatswood Dive Site study area consists of two study sites; however, the pedestrian bridge crossing along Mowbray Road (CWD02) was not modelled given it does not function as an intersection or mid-block crossing. Table 5-1 presents the peak hours utilised for modelling the intersections.

Table 5-2 provides a summary of the intersection LOS, while Figure 5-1 visualises a geospatial summary of the intersection LOS within the Chatswood Dive Site study area.

Network	Intersection ID	Weekday AM peak hour		Weekday PM peak hour		Weekend peak hour	
ID		Day	Start time	Day	Start time	Day	Start time
-	CWD01	Tuesday	8:00am	Friday	4:45pm	Saturday	11:30am
-	CWD02	No modelling was undertaken					

Table 5-1 Block 4 – Chatswood Dive Site peak hours modelled

Table 5-2 Block 4 – Chatswood Dive Site intersection performance summary

Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
CWD01	Mowbray Road/Hampden Road (Signal)	LOS B	LOS B	LOS B	
CWD02	Pedestrian Bridge Crossing along Mowbray Road (Bridge)	No modelling was undertaken.			

Overall, the intersection performance in the Chatswood Dive Site study area during the peak hours is satisfactory, operating at LOS B.


Figure 5-1 Block 4 – Chatswood Dive Site intersection performance summary

5.1.1 CWD01 – Mowbray Road/Hampden Road

The signalised intersection, composed of Mowbray Road, Hampden Road and the Chatswood Dive Site egress, is located directly south of the Chatswood Dive Site. This intersection serves as a connection point for the local road of Hampden Road, linking Chatswood and Artarmon, and the regional road of Mowbray Road, linking Willoughby to Lane Cove. Furthermore, the Chatswood Dive Site exits on to Mowbray Road at this intersection. The pedestrian bridge crossing along Mowbray Road (CWD02) connects with the eastern approach of this intersection.

Figure 5-2 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-2 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CWD01

Table 5-3 presents a performance summary of this intersection.

Table 5-3 Block 4 – Intersection performance summary of CWD0	1
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.516	58.6	99.3	LOS E
		East	0.459	17.8	147.5	LOS B
	Weekday AM	North	0.009	39.0	0.5	LOS C
Mowbray		West	0.478	9.4	133.1	LOS A
Road/		Total	0.516	16.9	147.5	LOS B
Hampden Road	Weekday	South	0.419	54.0	85.5	LOS D
Road		East	0.506	20.3	170.1	LOS B
(Signal)		North	0.009	38.9	0.5	LOS C
	1 101	West	0.447	10.0	116.8	LOS A
		Total	0.506	18.3	170.1	LOS B
	Weekend	South	0.532	50.2	120.3	LOS D

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		East	0.548	22.7	182.7	LOS B
		North	0.010	38.4	0.5	LOS C
		West	0.501	10.6	129.6	LOS A
		Total	0.548	19.3	182.7	LOS B

Overall, the intersection of Mowbray Road and Hampden Road performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Mowbray Road extends from the east approach to the intersection of Elizabeth Street/Orchard Road and from the west approach to the intersection of the Pacific Highway/Mowbray Road during all peak hours.

5.1.2 CWD02 – Pedestrian Bridge Crossing along Mowbray Road

The pedestrian bridge, located east of the intersection of Mowbray Road and Hampden Road and south of the Chatswood Dive Site, provides passage along Mowbray Road for pedestrians, cyclists, and general traffic over the T1 North Shore & Western and T9 Northern rail lines. Mowbray Road is an east-west thoroughfare that connects Willoughby in the east to Lane Cove in the west, intersecting with key roads including the Pacific Highway (A1).

The pedestrian bridge was not modelled in SIDRA Intersection as it does not function as an intersection or mid-block crossing. Rather it forms an extension of the eastern approach of the intersection of Mowbray Road and Hampden Road (CWD01, refer to Section 5.1.1).

5.1.3 Comparison with previous study blocks

Figure 5-3 provides a comparison of the total peak hourly traffic volumes recorded at CWD01 for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are relatively lower during the AM and PM peak hours, and generally similar in the weekend peak hours compared to pre-opening conditions.



Figure 5-3 Study block comparison – Chatswood Dive Site peak hourly traffic volumes at CWD01



Figure 5-4 Study block comparison – Chatswood Dive Site intersection performance summary

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5.2 Crows Nest Station

Crows Nest Station is a new underground station and the second stop along the City & Southwest Line (towards Sydenham). It is located in the south-east area of the St Leonards strategic centre, bounded by the Pacific Highway (A1), Oxley Street and Clarke Street in Crows Nest.

Crows Nest Station has accesses from both the Pacific Highway and Clarke Street. The two entrances are listed below:

- Clarke Street Entry; along Clarke Street, west of Hume Street
- Pacific Highway Entry; along Pacific Highway, south of Oxley Street.

Bus services are available within approximately 150 metres of Crows Nest Station. Bus stops located on the Pacific Highway (A1) facilitate connections to the external Sydney network, while bus stops on Willoughby Road connect to the internal Crows Nest centre. St Leonards Station, approximately 500 metres north-west from Crows Nest Station, offers the nearest rail service. Within a 50-metre distance of Crows Nest Station, a cycleway runs along Oxley Street, Hume Street and Clarke Street and pedestrian footpaths are available throughout Crows Nest.

The Crows Nest Station study area consists of 14 intersections, although the intersection of Hume Street and Clarke Lane was still under construction for a part of Block 4. Table 5-4 presents the peak hours utilised for modelling the intersections. Table 5-5 provides a summary of the intersection LOS, while Figure 5-5 visualises a geospatial summary of the intersection LOS within the Crows Nest Station study area.

Table 5-4 Block 4 – Crows	Nest Station	peak hours	modelled
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Network	Intersection	Weekday / hou	Weekday AM peak Weekday PM hour hour		PM peak ır	M peak r Weekend	
ID	ID	Day	Start time	Day	Start time	Day	Start time
	CST01						
	CST02						
	CST03						12:00pm
	CST04			n Thursday	5:00pm	Saturday	
	CST05		8:00am				
	CST06	Tuesday					
C31-N1	CST09	Tuesday					
	CST10						
	CST11						
	CST12						
	CST13						
	CST14						
-	CST07	Tuesday	8:15am	Wednesday	6:00pm	Saturday	6:30pm
-	CST08	Friday	8:00am	Wednesday	5:00pm	Saturday	11:30am

Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
CST01	Pacific Highway/Albany Street (Signal)	LOS B	LOS B	LOS B	
CST02	Pacific Highway/Oxley Street (Signal)	LOS A	LOS B	LOS B	
CST03	Pacific Highway/Hume Street (Signal)	LOS A	LOS A	LOS A	
CST04	Pacific Highway/Falcon Street/Shirley Road (Signal)	LOS C	LOS C	LOS C	
CST05	Clarke Street/Oxley Street (Priority – Give Way)	LOS A	LOS A	LOS A	
CST06	Clarke Street/Hume Street (Priority – Give Way)	LOS A	LOS B	LOS A	
CST07	Clarke Street/Willoughby Road (Priority – Give Way)	LOS A	LOS A	LOS A	
CST08	Albany Street/Willoughby Road (Signal)	LOS B	LOS B	LOS B	
CST09	Albany Street/Oxley Street (Roundabout)	LOS A	LOS B	LOS A	
CST10	Albany Street/Clarke Lane (Priority – Give Way)	LOS A	LOS A	LOS A	
CST11	Oxley Street/Clarke Lane (Priority – Stop)	LOS A	LOS A	LOS A	
CST12	Hume Street/Clarke Lane (Priority – Stop)	Under construction	LOS A	Under construction	
CST13	Pacific Highway/Alexander Street (Signal)	LOS B	LOS A	LOS A	
CST14	Falcon Street/Alexander Street	LOS B	LOS B	LOS B	

Table 5-5 Block 4 – Crows Nest Station intersection performance summary

Overall, the intersection performance in the Crows Nest Station study area during the peak hours is satisfactory, operating at LOS C or better.



Figure 5-5 Block 4 – Crows Nest Station intersection performance summary

5.2.1 CST01 – Pacific Highway/Albany Street

The signalised intersection, composed of the Pacific Highway and Albany Street, is located north-west of Crows Nest Station. It connects the State road of the Pacific Highway (A1), linking Wahroonga and North Sydney, with the local road of Albany Street, linking Crows Nest and St Leonards.

Figure 5-6 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-6 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST01

able 5-6 Block 4 – Intersection performance summary of CST01							
Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS	
		South-east	0.892	10.6	81.2	LOS A	
	Weekday	East	0.678	56.6	49.0	LOS E	
	AM	North-west	0.557	15.8	128.7	LOS B	
		Total	0.892	19.4	128.7	LOS B	
Pacific	Weekday PM	South-east	0.864	19.1	140.3	LOS B	
Highway/ Albanv		East	0.738	56.4	49.0	LOS D	
Street		North-west	0.419	10.7	72.2	LOS A	
(Signal)		Total	0.864	21.6	140.3	LOS B	
(South-east	0.856	12.1	71.9	LOS A	
	Weekend	East	0.726	56.8	49.0	LOS E	
		North-west	0.394	9.7	67.3	LOS A	
		Total	0.856	19.1	71.9	LOS B	

Table 5-6 presents a performance summary of this intersection.

Table 5-6 Block / - Intersection	norformanco	cummary	1 of CSTO
Table 5-0 block 4 - Intersection	periormance	Summary	01 0010

Overall, the intersection of the Pacific Highway (A1) and Albany Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Albany Street (east approach) extends back to Clarke Lane during all peak hours, and extends back on the Pacific Highway (south-east approach) to Oxley Street during the PM peak hour.

It was also noted that Block 4 pedestrian volumes at Pacific Highway (A1) and Albany Street significantly increased during the weekday AM and PM peak hour compared to pre-opening conditions. Given that CST01 is located along the frontage of Crows Nest station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.2.2 CST02 – Pacific Highway/Oxley Street

The signalised intersection, composed of the Pacific Highway and Oxley Street, is located directly north-west of Crows Nest Station. It connects the local road of Oxley Street, linking St Leonards and Naremburn through Crows Nest, with the State road of the Pacific Highway (A1), linking Wahroonga and North Sydney.

During Block 4, the kerbside approach lane on Pacific Highway (A1) (south-east approach) was closed during the weekday AM peak hour due to construction works. Additionally, the pedestrian crossing on Pacific Highway (A1) (south-east approach) was closed during all peak hours.

Figure 5-7 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-7 Block 4 – PM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST02

Table 5-7 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Pacific Highway/ Oxley Street	v/ eet Weekday AM	South-east	0.579	7.9	91.7	LOS A
		North-east	0.246	47.6	38.2	LOS D
		North-west	0.345	1.4	23.4	LOS A
(Signal)		South-west	0.505	58.2	66.8	LOS E

Table 5-7 Block 4 – Intersection performance summary of CST02

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		Total	0.579	13.3	91.7	LOS A
		South-east	0.447	3.8	53.4	LOS A
		North-east	0.239	45.5	34.5	LOS D
	Weekday	North-west	0.252	12.0	69.0	LOS A
	I IVI	South-west	0.754	62.2	79.7	LOS E
		Total	0.754	19.1	79.7	LOS B
		South-east	0.349	3.6	36.6	LOS A
	Weekend	North-east	0.318	47.3	40.8	LOS D
Week		North-west	0.237	11.6	63.8	LOS A
		South-west	0.529	54.8	62.5	LOS D
		Total	0.529	17.4	63.8	LOS B

Overall, the intersection of the Pacific Highway (A1) and Oxley Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue on Oxley Street extends from the north-east approach to Clarke Lane during all peak hours and from the south-west approach back to Nicholson Street during the weekday PM peak hour.

It was also noted that Block 4 pedestrian volumes at Pacific Highway (A1) and Oxley Street significantly increased during all peak hours compared to pre-opening conditions. Given that CST02 is located along the frontage of Crows Nest station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.2.3 CST03 – Pacific Highway/Hume Street

The signalised intersection, composed of the Pacific Highway and Hume Street, is located directly south-west of Crows Nest Station. It connects the State road of the Pacific Highway (A1), linking Wahroonga and North Sydney, with the local road of Hume Street, linking Crows Nest and Wollstonecraft. A dedicated cycleway runs along both sides of Hume Street; however, it was not considered for this assessment.

Hume Street (north-east leg) was previously closed during Block 1 to 3 as part of the Sydney Metro construction works. During Block 4, access to Hume Street (north-eastern approach) was closed during the identified AM and weekend peak hours due to Sydney Metro construction; however, it was reopened to traffic during the identified PM peak hour. Additionally, the pedestrian crossing on Pacific Highway (south-east approach) was closed during all peak hours.

Figure 5-8 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-8 Block 4 – PM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST03

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.433	0.7	10.2	LOS A
		North-east		Closed due to	o construction	
	Weekday AM	North-west	0.334	3.2	73.7	LOS A
	7 (17)	South-west	0.501	64.9	46.0	LOS E
		Total	0.501	5.0	73.7	LOS A
	Weekday PM	South-east	0.294	6.4	59.8	LOS A
Pacific Highway/		North-east	0.139	54.5	13.9	LOS D
Hume Street		North-west	0.359	6.5	77.8	LOS A
(Signal)		South-west	0.328	58.9	31.0	LOS E
(Olghai)		Total	0.359	9.7	77.8	LOS A
		South-east	0.331	7.0	66.4	LOS A
		North-east		Closed due to	o construction	
	Weekend	North-west	0.321	5.3	63.9	LOS A
		South-west	0.349	61.0	27.6	LOS E
		Total	0.349	8.4	66.4	LOS A

Table 5-8 presents a performance summary of this intersection.

Table 5-8 Block 4 – Intersection performance summary of CST03

Overall, the intersection of the Pacific Highway (A1) and Hume Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Pacific Highway (A1) and Hume Street significantly increased during all peak hours compared to pre-opening conditions. Given that CST03 is located along the frontage of Crows Nest station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.2.4 CST04 – Pacific Highway/Falcon Street/Shirley Road

The signalised intersection, composed of the Pacific Highway, Falcon Street and Shirley Road, is located south-east of Crows Nest Station. It connects the State road of the Pacific Highway (A1), linking Wahroonga to North Sydney, with the State road of Falcon Street, linking Crows Nest and Neutral Bay, and Shirley Road, linking Crows Nest and Wollstonecraft. Willoughby Road is an unsignalised approach, serving as an exit only route onto Falcon Street from the Crows Nest centre.

Figure 5-9 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-9 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST04

Table 5-9 presents a performance summary of this intersection.

Table 5-9 Bloc	k 4 – Intersection	performance summ	ary of CST04
		•	

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.638	29.9	173.9	LOS C
		East	0.921	61.6	130.6	LOS E
Pacific Highway/	Weekday AM	North	0.035	3.8	1.6	LOS A
Falcon		North-west	0.730	22.1	173.8	LOS B
Street/		South-west	0.928	77.5	211.7	LOS F
Road		Total	0.928	41.0	211.7	LOS C
(Signal) Weekd		South-east	0.779	33.4	161.3	LOS C
	Weekday PM	East	0.878	38.1	130.6	LOS C
	PIVI	North	0.050	3.8	2.6	LOS A

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North-west	0.668	43.2	149.2	LOS D
		South-west	0.534	46.8	131.2	LOS D
		Total	0.878	39.1	161.3	LOS C
		South-east	0.557	31.1	105.3	LOS C
		East	0.915	43.1	130.6	LOS D
	Weekend	North	0.060	3.8	2.9	LOS A
		North-west	0.665	39.4	144.4	LOS C
		South-west	0.647	47.9	131.8	LOS D
		Total	0.915	38.7	144.4	LOS C

Overall, the intersection of the Pacific Highway (A1), Falcon Street, and Shirley Road performs at LOS C or better during all peak hours. The 95th percentile queue on the Pacific Highway (A1) (south-east approach) extends back to Alexander Street during the weekday AM and PM peak hours. Similarly, the 95th percentile queue on Falcon Street (east approach) extends back to Alexander Street during all peak hours.

5.2.5 CST05 – Clarke Street/Oxley Street

The priority intersection, composed of Oxley Street and Clarke Street, is located directly north of Crows Nest Station. It connects the local roads of Clarke Street in Crows Nest and Oxley Street, linking Wollstonecraft and Naremburn through Crows Nest. A dedicated cycleway runs along the eastern side of Oxley Street (north approach) and the northern side of Clarke Street; however, it was not considered for this assessment.

Figure 5-10 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-10 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST05

Table 5-10 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.085	5.9	2.1	LOS A
	Weekday	North	0.147	4.4	0.0	LOS A
	AM	South-west	0.107	4.8	3.0	LOS A
		Total	0.085	5.9	2.1	LOS A
Clarke	Weekday PM	South-east	0.068	5.9	1.7	LOS A
Street/ Oxley Street		North	0.136	4.4	0.0	LOS A
(Drienity)		South-west	0.139	4.9	3.3	LOS A
(Priority – Give Way)		Total	0.068	5.9	1.7	LOS A
Week		South-east	0.076	5.7	1.8	LOS A
		North	0.154	4.4	0.0	LOS A
	Weekend	South-west	0.106	4.9	2.7	LOS A
		Total	0.076	5.7	1.8	LOS A

Table 5-10 Block 4 – Intersection performance summary of CST05

Overall, the intersection of Clarke Street and Oxley Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.2.6 CST06 – Clarke Street/Hume Street

The priority intersection, composed of Clarke Street and Hume Street, is located directly north-east of Crows Nest Station. It connects the local roads of Clarke Street in Crows Nest and Hume Street, linking Crows Nest and Wollstonecraft. A dedicated cycleway runs along the northern side of Clarke Street and both sides of Hume Street (south-west approach); however, it was not considered for this assessment.

During Block 4, access to Hume Street (south-west approach) was limited to local area access and construction access only in the AM and weekend peak hours; however, it was open to general traffic during the PM peak hour.

Figure 5-11 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)



Table 5-11 presents a performance summary of this intersection	•
Table 5-11 Block 4 – Intersection performance summary of CST06	

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.054	3.7	0.8	LOS A
	Weekday AM	North-west	0.090	4.1	0.2	LOS A
Clarke	7 (17)	Total	0.090	4.1	0.2	LOS A
Street/		South-east	0.201	6.5	6.3	LOS A
Hume Street	Weekday PM	North-west	0.413	21.1	16.7	LOS B
(Priority – Give Way) Wee	I IVI	Total	0.413	21.1	16.7	LOS B
		South-east	0.053	3.8	0.8	LOS A
	Weekend	North-west	0.095	4.1	0.1	LOS A
		Total	0.095	4.1	0.1	LOS A

Overall, the intersection of Clarke Street and Hume Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Clarke Street and Hume Street significantly increased during the weekday AM and weekend peak hours compared to pre-opening conditions. Given that CST06 is located along the frontage of Crows Nest station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.2.7 CST07 – Clarke Street/Willoughby Road

The priority intersection, composed of Clarke Street and Willoughby Road, is located east of Crows Nest Station. It connects the local roads of Clarke Street in Crows Nest and Willoughby Road, linking

Crows Nest and Willoughby. A dedicated cycleway runs along the northern side of Clarke Street; however, it was not considered for this assessment.

Figure 5-12 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-12 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST07

Table 5-12 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.177	5.2	6.1	LOS A
	Weekday	North	0.065	8.1	2.1	LOS A
	AM	West	0.154	6.3	3.8	LOS A
		Total	0.065	8.1	2.1	LOS A
Clarke Street/	Weekday PM	South	0.192	5.6	6.7	LOS A
Willoughby		North	0.176	8.5	5.4	LOS A
Road		West	0.233	7.7	6.1	LOS A
(Priority –		Total	0.176	8.5	5.4	LOS A
Give way)		South	0.218	5.8	7.7	LOS A
		North	0.214	9.1	6.7	LOS A
	vveekend	West	0.212	8.4	5.3	LOS A
		Total	0.214	9.1	6.7	LOS A

Table 5-12 Block 4 – Intersection performance summary of CST07

Overall, the intersection of Clarke Street and Willoughby Road performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.2.8 CST08 – Albany Street/Willoughby Road

The signalised intersection, composed of Albany Street and Willoughby Road, is located north-east of Crows Nest Station. It connects the local roads of Albany Street, linking Crows Nest and St Leonards, and Willoughby Road, linking Crows Nest and Willoughby.

During Block 4, the southern departure lane of Willoughby Road was closed during the AM peak hour due to construction works.

Figure 5-13 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-13 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST08

Table 5-13 presents a performance summary of this intersection.

Table 5-13 Block 4 - Intersection pe	erformance summary of CST08
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.038	23.3	2.8	LOS B
		East	0.521	13.5	46.9	LOS A
Albany	Weekday AM	North	0.270	19.4	28.0	LOS B
Street/		West	0.462	19.3	55.5	LOS B
Willoughby Road		Total	0.521	16.9	55.5	LOS B
(Signal)	Weekday PM	South	0.207	24.4	19.3	LOS B
		East	0.447	15.0	39.6	LOS B
		North	0.437	22.6	38.3	LOS B
		West	0.493	20.3	64.9	LOS B

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		Total	0.493	19.5	64.9	LOS B
		South	0.166	21.1	15.4	LOS B
		East	0.567	14.7	60.8	LOS B
	Weekend	North	0.391	19.9	34.1	LOS B
		West	0.397	19.6	45.2	LOS B
		Total	0.567	18.0	60.8	LOS B

Overall, the intersection of Albany Street and Willoughby Road performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.2.9 CST09 – Albany Street/Oxley Street

The roundabout, composed of Albany Street and Oxley Street, is located north of Crows Nest Station. It connects the local roads of Albany Street, linking Crows Nest and St Leonards, and Oxley Street, linking Wollstonecraft and Naremburn through Crows Nest.

Figure 5-14 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-14 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST09

Table 5-14 presents a performance summary of this intersection.

Table 5-14 Block 4 – Intersection performance summary of CST09

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.236	11.8	11.2	LOS A

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		East	0.528	11.9	26.4	LOS A
	Weekday	North	0.356	12.1	17.2	LOS A
	AM	West	0.479	9.7	27.6	LOS A
		Total	0.356	12.1	17.2	LOS A
	Weekday	South	0.443	12.7	21.5	LOS A
Albany		East	0.824	16.4	48.6	LOS B
Street/Oxley		North	0.369	12.4	18.4	LOS A
Street	I IVI	West	0.579	11.4	37.6	LOS A
(Roundabout)		Total	0.824	16.4	48.6	LOS B
		South	0.267	11.4	12.2	LOS A
		East	0.445	10.6	19.9	LOS A
	Weekend	North	0.275	11.2	12.1	LOS A
		West	0.402	9.7	22.3	LOS A
		Total	0.267	11.4	12.2	LOS A

Overall, the intersection of Albany Street and Oxley Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.2.10 CST10 – Albany Street/Clarke Lane

The priority intersection, composed of Albany Street and Clarke Lane, is located north-west of Crows Nest Station. It connects the local roads of Clarke Lane in Crows Nest with Albany Street, linking Crows Nest and St Leonards.

Figure 5-15 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-15 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST10

Table 5-15 presents a performance summary of this intersection.

Table 5-15 Block 4 – Intersection performance summary of CST10

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.062	13.6	27.5	LOS A
	Weekday	East	0.226	0.0	65.9	LOS A
	AM	West	0.250	0.0	0.0	LOS A
		Total	0.062	13.6	27.5	LOS A
Albany	Weekday PM	South-east	0.049	14.4	6.0	LOS A
Street/ Clarke Lane		East	0.125	0.0	63.6	LOS A
(Dei seite)		West	0.270	0.0	0.0	LOS A
(Priority – Give Way)		Total	0.049	14.4	6	LOS A
		South-east	0.043	12.8	20.3	LOS A
		East	0.212	0.0	61.1	LOS A
	vveekend	West	0.226	0.0	0.0	LOS A
		Total	0.043	12.8	20.3	LOS A

Overall, the intersection of Albany Street and Clarke Lane performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.2.11 CST11 – Oxley Street/Clarke Lane

The priority intersection, composed of Oxley Street and Clarke Lane, is located directly north-west of Crows Nest Station. It connects the local roads of Clarke Lane in Crows Nest and Oxley Street, linking Wollstonecraft and Naremburn through Crows Nest.

Clarke Street (south-east approach) was previously closed during Block 1 to 3 as part of the Sydney Metro construction works. During Block 4, this road was reopened to traffic.

Figure 5-16 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.

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Source: Nearmap (September 2024)

Figure 5-16 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST11

Carke LIT (SE)

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS	
		South-east	0.007	12.1	0.2	LOS A	
		North-east	0.170	3.5	0.3	LOS A	
	Weekday AM	North-west	0.023	6.3	0.5	LOS A	
		South-west	0.088	2.8	0.0	LOS A	
		Total	0.007	12.1	0.2	LOS A	
Ovlov		South-east	0.010	9.2	0.2	LOS A	
Street/		North-east	0.120	4.9	0.5	LOS A	
Clarke Lane	Weekday DM	North-west	0.020	6.4	0.5	LOS A	
(Priority –	FIVI	South-west	0.121	2.8	0.0	LOS A	
Stop)		Total	0.010	9.2	0.2	LOS A	
		South-east	0.005	9.0	0.1	LOS A	
		North-east	0.109	3.6	3.7	LOS A	
	Weekend	North-west	0.016	6.7	0.5	LOS A	
		South-west	0.086	2.8	0.0	LOS A	
		Total	0.005	9.0	0.1	LOS A	

Table 5-16 presents a performance summary of this intersection.

Table 5-16 Block 4 – Intersection performance summary of CST11

Overall, the intersection of Oxley Street and Clarke Lane performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Oxley Street and Clarke Lane significantly increased during all peak hours compared to pre-opening conditions. Given that CST11 is located along the frontage of Crows Nest station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.2.12 CST12 – Hume Street/Clarke Lane

The priority intersection, composed of Hume Street and Clarke Lane, is located directly south-east of Crows Nest Station. It connects the local roads of Clarke Lane in Crows Nest and Hume Street, linking Crows Nest and Wollstonecraft. A dedicated cycleway runs along both sides of Hume Street; however, it was not considered for this assessment.

During Block 4, the intersection was closed due to construction works during the AM and weekend peak hours and, as such, was only modelled for the PM peak hour assessment as part of the Block 4 study.

Figure 5-17 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-17 Block 4 – PM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST12

Table 5-17 presents a performance summary of this intersection.

Fable 5-17 Block 4 – Intersection	n performance summar	y of CST12
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS	
Hume		South-east					
	Weekday AM	North-east	Closed due to construction				
		Total					
Street/ Clarke Lane	Weekday PM	South-east	0.006	6.7	0.0	LOS A	
		North-east	0.013	3.4	0.0	LOS A	
(Priority - Stop)		Total	0.006	6.7	0.0	LOS A	
		South-east					
	vveekend	North-east	Closed due to construction				

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		Total				

Overall, the intersection of Hume Street and Clarke Lane performs satisfactorily at LOS A during the PM peak hour. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.2.13 CST13 – Pacific Highway/Alexander Street

The signalised intersection, composed of the Pacific Highway, Alexander Street and Hayberry Street, is located south-east of Crows Nest Station. It connects the State road of the Pacific Highway (A1), linking Wahroonga to North Sydney, with the local roads of Alexander Street and Hayberry Street in Crows Nest. Hayberry Street was not modelled.

Figure 5-18 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-18 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST13

Table 5-18 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Decifie	Weekday AM	South-east	0.545	9.7	74.9	LOS A
Highway/		North	0.707	45.7	88.1	LOS D
Alexander Street (Signal)		North-west	0.679	12.0	126.1	LOS A
		Total	0.707	15.0	126.1	LOS B
		South-east	0.464	8.8	72.1	LOS A

Table 5-18 Block 4 – Intersection performance summary of CST13

6.1

43.1

3.4

10.6

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday PM	North	0.979	39.1	57.1	LOS C
		North-west	0.454	5.2	53.0	LOS A
		Total	0.979	11.3	72.1	LOS A

Overall, the intersection of the Pacific Highway (A1) and Alexander Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue on the Pacific Highway (A1) (north-west approach) extends back to Shirley Road and Falcon Street during the weekday AM peak hour. Similarly, the 95th percentile queue on Alexander Street (north approach) extends back to Falcon Street during the AM peak hour.

0.311

0.883

0.359

0.883

South-east

North

North-west

Total

5.2.14 CST14 – Falcon Street/Alexander Street

Weekend

The signalised intersection, comprised of Falcon Street and Alexander Street, is located south-east of Crows Nest Station. It connects the local road of Alexander Street in Crows Nest with the State road of Falcon Street, linking Crows Nest and Neutral Bay.

Figure 5-19 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-19 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CST14

Table 5-19 presents a performance summary of this intersection.

LOS A

LOS D

LOS A

LOS A

41.7

55.5

26.9

55.5

56

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.478	59.5	71.0	LOS E
		East	0.672	12.1	127.1	LOS A
	Weekday AM	North	0.767	80.2	121.7	LOS F
	7.111	West	0.558	5.9	58.4	LOS A
		Total	0.767	23.8	127.1	LOS B
Falcon	Weekday PM	South	0.468	61.1	64.5	LOS E
Street/		East	0.515	22.9	111.2	LOS B
Alexander		North	0.439	48.6	64.3	LOS D
Olicci		West	0.355	2.0	19.4	LOS A
(Signal)		Total	0.515	21.5	111.2	LOS B
		South	0.478	59.5	71.0	LOS E
		East	0.670	20.2	167.5	LOS B
	Weekend	North	0.767	79.8	121.7	LOS F
		West	0.558	5.7	58.4	LOS A
		Total	0.767	26.7	167.5	LOS B

Table 5-19 Block 4 – Intersection performance summary of CST14

Overall, the intersection of Falcon Street and Alexander Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Alexander Street (south approach) extends back to the Pacific Highway (A1) during the weekday AM peak hour.

5.2.15 Comparison with previous study blocks

Figure 5-20 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are slightly lower during all peak hours compared to pre-opening conditions.



Figure 5-20 Study block comparison - Crows Nest Station peak hourly traffic volume across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-21 and Figure 5-22. All intersections in the Crows Nest Station study area perform at LOS C or better during Block 4, which is generally similar to previous study blocks.

Pacific Highway/Falcon Street/Shirley Road (CST04) had a notable change in LOS, whereby the intersection improved from a LOS D to a LOS C in the AM peak hour compared to Block 3. This change in LOS for CST04 was due to lower traffic volumes at this intersection in Block 4 during the AM peak hour.

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and a second	C\$T01 - PACIFIC HWY / ALBANY \$T	CST02 - PACIFIC HWY / OXLEY ST
Atchison Street	AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
Albany Lane CST01 CST01 CST01 CST08 CST	Los B Los B	LOS A LOS A LOS A LOS A LOS B LOS B
Pole Lane Chaper L	BLOCK 1 BLOCK 3 BLOCK 4 BLOCK 4 BLOCK 4 BLOCK 4 BLOCK 4 BLOCK 3 BLOCK 3 BLOCK 3 BLOCK 4	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4 BLOCK 4 BLOCK 4 BLOCK 2 BLOCK 2 BLOCK 3 BLOCK 4
CST00	CST03 - PACIFIC HWY / HUME ST	CST04 - PACIFIC HWY / FALCON ST / SHIRLEY RD
	AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
CST02 CLarke Charles And Homos Additional Street	LOSA LOSA LOSA LOSA LOSA LOSA LOSA LOSA	LOS C LOS C LOS C LOS C LOS C LOS C LOS C LOS C LOS C LOS C
CST03 CST06 CST07	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 1 BLOCK 3 BLOCK 3 BLOCK 1 BLOCK 3 BLOCK 3 BLOCK 4	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4 BLOCK 2 BLOCK 4 BLOCK 4 BLOCK 2 BLOCK 3 BLOCK 4 BLOCK 3 BLOCK 4
	C \$T05 - CLARKE \$T / OXLEY \$T	CST06 - CLARKE ST / HUME ST
Hume Street N	AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
Christie	Losa Losa Losa Losa Losa Losa Losa Losa	LOSA LOSA LOSA LOSA LOSA LOSA LOSA LOSA
River Lane of The CST04	BLOCK 1 BLOCK 2 BLOCK 4 BLOCK 4 BLOCK 2 BLOCK 2 BLOCK 1 BLOCK 2 BLOCK 2 BLOCK 2 BLOCK 2 BLOCK 4	BLOCK 1 BLOCK 2 BLOCK 4 BLOCK 4 BLOCK 1 BLOCK 4 BLOCK 1 BLOCK 3 BLOCK 3 BLOCK 4
z ^{Sue}	CST07 - CLARKE ST / WILLOUGHBY RD	CST06 - ALDANT ST / WILLOUGHDT KD
Stre.	CST07 - CLARKE ST / WILLOUGHBY RD AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
River Roal Priority - Give Way	CST07 - CLARKE ST / WILLOUGHBY RD AM PEAK PM PEAK WE PEAK V <	AM PEAK PM PEAK WE PEAK 0

Figure 5-21 Study block comparison – Crows Nest Station intersection performance summary (CST01-CST08)

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Figure 5-22 Study block comparison – Crows Nest Station intersection performance summary (CST09-CST14)

5.3 Victoria Cross Station

Victoria Cross Station is a new underground station and the third stop on the City & Southwest Line (towards Sydenham). It is located in the centre of the North Sydney commercial centre and north of the existing North Sydney Station.

Victoria Cross Station has multiple accesses on Miller Street and Denison Street. The accesses are separated into Victoria Cross North and Victoria Cross South. The three station entrances are listed below:

- Victoria Cross North Entry; at the north-east corner of the intersection of Miller Street and McLaren Street
- Victoria Cross South Entry (Miller Street); along Miller Street, south of Berry Street
- Victoria Cross South Entry (Denison Street); along Denison Street, south of Berry Street.

Bus services are available within approximately 150 metres of Victoria Cross Station, located along Miller Street and Pacific Highway. Pedestrian footpaths are provided on both sides of Miller Street and Pacific Highway in the vicinity of Victoria Cross Station.

The Victoria Cross Station study area consists of four intersections. Note that during Block 4, traffic volumes across the Victoria Cross Station study area may have been impacted due to the ongoing construction works associated with the nearby Warringah Freeway Upgrade project.

Table 5-20 presents the peak hours utilised for modelling the intersections. Table 5-21 provides a summary of the intersection LOS while Figure 5-23 visualises a geospatial summary of the intersection LOS within the Victoria Cross Station study area.

Network	Intersection	Weekday AM peak hour		Weekday hou	PM peak ur	Weekend peak hour	
ID	ID	Day	Start time	Day	Start time	Day	Start time
	VIC01	Tuesday	7:45am	Friday	2:45pm	Saturday	12:00pm
	VIC02						
VIC-N1	VIC03						
	VIC04						

Table 5-20 Block 4 – Victoria Cross Station peak hours modelled

Table 5-21 Block 4 – Victoria Cross Station intersection performance summary

Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
VIC01	Pacific Highway/Berry Street (Signal)	LOS A	LOS A	LOS A	
VIC02	Miller Street/Berry Street (Signal)	LOS D	LOS C	LOS C	
VIC03	Miller Street/McLaren Street (Signal)	LOS B	LOS B	LOS B	
VIC04	Pacific Highway/Miller Street (Signal)	LOS C	LOS C	LOS C	

Overall, the intersection performance in the Victoria Cross Station study area during the peak hours is satisfactory, operating at LOS D or better.



Figure 5-23 Block 4 – Victoria Cross Station intersection performance summary

5.3.1 VIC01 – Pacific Highway/Berry Street

The signalised intersection, composed of the Pacific Highway and Berry Street, is located west of southern access to Victoria Cross Station. It connects the State road of the Pacific Highway (A1), linking Wahroonga and North Sydney, with the local road of Berry Street, linking North Sydney to the Warringah Freeway (M1). Berry Street (south-west approach) is not signalised; however, for modelling purposes, it has been simulated as a signalised approach operating in every phase.

Figure 5-24 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-24 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of VIC01

Table 5-22 presents a performance summary of this intersectio

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.852	9.9	60.8	LOS A
	Weekday	North-west	0.668	14.1	108.7	LOS A
	AM	South-west	0.114	5.9	4.4	LOS A
		Total	0.852	11.9	108.7	LOS A
Pacific	Weekday PM	South-east	0.855	12.9	55	LOS A
Highway/		North-west	0.461	12.2	79.5	LOS A
Berry Street		South-west	0.066	4.0	1.4	LOS A
(Signal)		Total	0.855	12.4	79.5	LOS A
		South-east	0.649	7.5	30.5	LOS A
		North-west	0.325	9.5	57.9	LOS A
	VVeekend	South-west	0.024	4.4	0.7	LOS A
		Total	0.649	8.4	57.9	LOS A

Table 5-22 Block 4 – Intersection	n performance summary	of VIC01
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Overall, the intersection of the Pacific Highway (A1) and Berry Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 vehicle volumes at Pacific Highway (A1) and Berry Street significantly decreased during the weekend peak hour compared to pre-opening conditions. Traffic diversions in place for the Warringah Freeway construction works may have caused the drop in vehicle volumes at VIC01.

5.3.2 VIC02 – Miller Street/Berry Street

The signalised intersection, composed of Miller Street and Berry Street, is located directly adjacent to the southern access to Victoria Cross Station. It connects the regional road of Miller Street, linking Cammeray and North Sydney, with the local road of Berry Street, linking North Sydney to the Warringah Freeway (M1).

During Block 4, the southern departure kerbside lane of Miller Street was partially closed off during the PM peak hour, due to Sydney Metro construction.

Figure 5-25 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-25 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of VIC02

 Table 5-23 presents a performance summary of this intersection.

 Table 5-23 Block 4 – Intersection performance summary of VIC02

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Miller Weekday Street/Berry AM Street	Weekday	South	0.974	56.6	186.2	LOS E
		North	0.793	52.7	135.2	LOS D
	AM	West	0.695	34.1	155.6	LOS C
	Total	0.974	44.0	186.2	LOS D	
(Signal)	Weekday PM	South	0.639	19.1	75.3	LOS B
		North	0.826	46.1	73.8	LOS D

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		West	0.481	40.9	91.6	LOS C
		Total	0.826	34.2	91.6	LOS C
	Weekend	South	0.682	24.1	69.1	LOS B
		North	0.839	48.0	63.4	LOS D
		West	0.431	31.9	74.6	LOS C
		Total	0.839	32.9	74.6	LOS C

Overall, the intersection of Miller Street and Berry Street performs satisfactorily at LOS D or better during all peak hours; noting however, it is close to capacity as indicated by the degree of saturation being close to 1.00. The 95th percentile queue on Miller Street (south approach) extends back to the Pacific Highway (A1) during the weekday AM peak hour. Similarly, the 95th percentile queue on Berry Street (west approach) extends back to the Pacific Highway (A1) during the AM peak hour.

It was also noted that Block 4 pedestrian volumes at Miller Street and Berry Street significantly increased during the weekend peak hour compared to pre-opening conditions. Given that VIC02 is located in close proximity of Victoria Cross station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.3.3 VIC03 – Miller Street/McLaren Street

The signalised intersection, composed of Miller Street and McLaren Street, is located directly adjacent to the northern access to Victoria Cross Station. It connects the regional road of Miller Street, linking North Sydney and Cammeray, with the local road of McLaren Street in North Sydney.

Figure 5-26 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-26 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of VIC03

Table 5-24 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday AM	South	0.437	13.6	88.6	LOS A
		East	0.752	76.3	47.7	LOS F
		North	0.388	15.2	84.4	LOS B
		West	0.783	52.1	55.6	LOS D
Miller		Total	0.783	27.1	88.6	LOS B
	Weekday PM	South	0.290	12.1	40.5	LOS A
		East	0.269	32.7	22.1	LOS C
Street		North	0.476	16.1	59.1	LOS B
(Signal)		West	0.440	34.3	25.4	LOS C
		Total	0.476	19.0	59.1	LOS B
	Weekend	South	0.317	10.7	49.3	LOS A
		East	0.485	45.3	23.1	LOS D
		North	0.402	14.4	62.5	LOS A
		West	0.330	34.9	23.8	LOS C
		Total	0.485	19.2	62.5	LOS B

Table 5-24 Block 4 – Intersection performance summary of VIC03

Overall, the intersection of Miller Street and McLaren Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Miller Street and McLaren Street significantly increased during the weekday PM peak hour compared to pre-opening conditions. Given that VIC03 is located in close proximity of Victoria Cross station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.3.4 VIC04 – Pacific Highway/Miller Street

The signalised intersection, composed of the Pacific Highway, Miller Street and Mount Street, is located south of the southern access to Victoria Cross Station. It connects the State road of the Pacific Highway (A1), linking Wahroonga and North Sydney, with the regional road of Miller Street, linking North Sydney and Cammeray. Additionally, it provides travel to the west of North Sydney via the Mount Street unsignalised egress-only approach.

Figure 5-27 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-27 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of VIC04

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday AM	South	0.663	45.0	77.3	LOS D
		South-east	0.882	43.5	171.8	LOS D
		North	0.405	14.8	33.5	LOS B
Pacific		North-west	0.488	33.0	71.5	LOS C
		Total	0.882	37.9	171.8	LOS C
		South	0.778	44.4	89.8	LOS D
		South-east	0.799	28.1	93.0	LOS B
Miller Street	Weekday PM	North	0.294	11.8	13.4	LOS A
(Signal)	1 171	North-west	0.505	35.9	63.2	LOS C
		Total	0.799	30.7	93.0	LOS C
	Weekend	South	0.573	35.8	55.8	LOS C
		South-east	0.676	31.1	86.6	LOS C
		North	0.478	26.4	36.5	LOS B
		North-west	0.413	29.7	56.9	LOS C
		Total	0.676	31.0	86.6	LOS C

mmon of this intersection

Overall, the intersection of the Pacific Highway (A1), Miller Street and Mount Street performs satisfactorily at LOS C during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.
It was also noted that Block 4 pedestrian volumes at Pacific Highway (A1) and Miller Street significantly increased during the weekday PM peak hour compared to pre-opening conditions. Given that VIC04 is located in close proximity of Victoria Cross station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.3.5 Comparison with previous study blocks

Figure 5-28 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are slightly higher during the AM peak hour and slightly lower in the PM and weekend peak hours compared to preopening conditions. The variability in traffic volumes in the area between study blocks may be due to the ongoing construction works associated with the nearby Warringah Freeway Upgrade project.



Figure 5-28 Study block comparison – Victoria Cross Station peak hourly traffic volume across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-29. All intersections in the Victoria Cross Station precinct perform at LOS D or better during Block 4, which is generally similar to previous study blocks.

Miller Street/Berry Street (VIC02) had a notable change in LOS, whereby the intersection reduced from a LOS C to a LOS D in the AM peak hour compared to Block 3. However, it should be noted that this intersection was on the border of LOS C and D during Block 3. This change in LOS for VIC02 was due to higher traffic volumes at this intersection in Block 4 during the AM peak hour.



Figure 5-29 Study block comparison – Victoria Cross Station intersection performance summary

5.4 Barangaroo Station

Barangaroo Station is a new underground station and the fourth stop on the City & Southwest Line (towards Sydenham). It is located at the northern area of Barangaroo, south of Munn Street, bounded by Hickson Road.

Barangaroo Station is accessed from Hickson Road via the two station entrances listed below:

- Escalator access only; along Hickson Road, south of Munn Street
- Lift access only; along Hickson Road, south of Munn Street.

Bus services are available within approximately 400 metres of Barangaroo Station, located along Hickson Road and Kent Street. Dedicated cycle lanes are provided along the Sydney Harbour Bridge on-ramp and Kent Street, south of the intersection of Kent Street, Clarence Street and the Sydney Harbour Bridge on-ramp. Around the station precinct, there will be two new bus stops on Hickson Road (one northbound travel and one southbound travel). Kiss and ride bays and taxi zones will be provided at the proposed Hickson Road interchange, and coach bays underneath Munn Street bridge.

The Barangaroo Station study area consists of 18 intersections. Table 5-26 presents the peak hours utilised for modelling the intersections. Table 5-27 provides a summary of the intersection LOS while Figure 5-30 visualises a geospatial summary of the intersection LOS within the Barangaroo Station study area.

Network	Intersection	Weekday AM peak hour		Weekday PM	peak hour	Weekend peak hour	
ID	ID	Day	Start time	Day	Start time	Day	Start time
	BGU01						
BGU- N1	BGU02	Tuesday	0:45 om		E 4E	Soturdov	6:00nm
	BGU16		0.40411	weunesuay	5.45pm	Saturuay	0.00pm
	BGU17						
BGU- N2	BGU04				5:30pm		6:00pm
	BGU05	Wednesday	8:15am	Wednesday		Saturday	
	BGU07						
	BGU08						
	BGU09						
	BGU06			Wednesday	5:30pm	Saturday	5:30pm
	BGU10						
BGU-	BGU11	Thursday					
N3	BGU12	Thursday	8:30am				
	BGU13						
	BGU18						
BGU-	BGU14	Thursday	0.20 arr	Thursday			C:00mm
N4	BGU15	inursday	8:30am		5:00pm	Saturday	6:00pm
-	BGU03	Tuesday	8:30am	Wednesday	5:15pm	Saturday	5:30pm

Table 5-26 Block 4 – Barangaroo Station peak hours modelled

Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
BGU01	Hickson Road/Towns Place (Priority – Give Way)	LOS A	LOS A	LOS A	
BGU02	Dalgety Road/Towns Place (Roundabout)	LOS A	LOS A	LOS A	
BGU03	Kent Street/Argyle Street (Priority – Give Way)	LOS B	LOS A	LOS B	
BGU04	Pedestrian Mid-block Crossing at Kent Street near Gas Lane (Pedestrian only - Signal)	LOS B	LOS B	LOS B	
BGU05	Kent Street/Sydney Harbour Bridge (SHB) On-ramp (Signal)	LOS B	LOS B	LOS B	
BGU06	Hickson Road/Napoleon Street/ Sussex Street (Signal)	LOS B	LOS B	LOS B	
BGU07	Margaret Street/Kent Street/ Napoleon Street (Signal)	LOS B	LOS B	LOS B	
BGU08	Margaret Street/Clarence Street (Signal)	LOS B	LOS B	LOS B	
BGU09	Margaret Street/York Street (Signal)	LOS B	LOS B	LOS B	
BGU10	Pedestrian Mid-block Crossing at Sussex Street under Exchange Place (Pedestrian only - Signal)	LOS A	LOS A	LOS A	
BGU11	Pedestrian Mid-block Crossing at Kent Street near Margaret Street (Pedestrian only - Signal)	LOS A	LOS A	LOS A	
BGU12	Sussex Street/Erskine Street (Signal)	LOS B	LOS B	LOS B	
BGU13	Kent Street/Erskine Street (Signal)	LOS B	LOS B	LOS C	
BGU14	Sussex Street/King Street (Signal)	LOS B	LOS B	LOS B	
BGU15	Kent Street/King Street (Signal)	LOS B	LOS B	LOS B	
BGU16	New Pedestrian Mid-block Crossing at Hickson Road (north of Metro Station) (Pedestrian only - Zebra)	LOS A	LOS A	LOS A	
BGU17	New Pedestrian Mid-block Crossing at Hickson Road (south of Metro Station) (Pedestrian only - Zebra)	LOS A	LOS A	LOS A	
BGU18	Shelley Street/Erskine Street (Signal)	LOS B	LOS B	LOS B	

Table 5-27 Block 4 – Barangaroo Station intersection performance summary

Overall, the intersection performance in the Barangaroo Station study area during the peak hours is satisfactory, operating at LOS C or better.



Figure 5-30 Block 4 – Barangaroo Station intersection performance summary

5.4.1 BGU01 – Hickson Road/Towns Place

The priority intersection, composed of Hickson Road and Towns Place, is located north of Barangaroo Station. It connects the local road of Towns Place with the regional road of Hickson Road which runs along the western waterfront of Barangaroo.

Figure 5-31 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-31 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU01

Table 5-28 presents a	performance summary	of this	intersection
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Table 5-28 Block 4 – Intersection perfo	ormance summary of BGU01
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		East	0.147	6.3	4.9	LOS A
	Weekday	North-west	0.246	8.5	7.0	LOS A
	AM	South-west	0.205	5.3	7.6	LOS A
		Total	0.246	8.5	7.0	LOS A
Hickson	Weekday PM	East	0.289	8.4	9.7	LOS A
Road/Towns Place		North-west	0.351	13.3	10.9	LOS A
(Dei seite)		South-west	0.319	5.7	12.2	LOS A
(Priority – Give Way)		Total	0.351	13.3	10.9	LOS A
		East	0.305	8.2	10.8	LOS A
		North-west	0.413	14.2	13.8	LOS A
	Weekend	South-west	0.351	4.7	14.7	LOS A
		Total	0.413	14.2	13.8	LOS A

Overall, the intersection of Hickson Road and Towns Place performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.2 BGU02 – Dalgety Road/Towns Place

The roundabout intersection, composed of Dalgety Road and Towns Place, is located north of Barangaroo Station. It connects the local roads of Dalgety Road and Towns Place in Barangaroo with the Barangaroo Reserve car park.

Figure 5-32 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-32 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU02

Table 5-29 presents a performance summary of this intersection.

Table 5-29 Block 4 – Intersection	performance summary of BGU02
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.138	7.0	6.0	LOS A
	Weekday	South-east	0.092	8.2	4.2	LOS A
	AM	West	0.004	1.5	0.2	LOS A
		Total	0.092	8.2	4.2	LOS A
Dalgety	Weekday PM	South	0.130	7.0	5.6	LOS A
Road/Towns		South-east	0.165	8.3	7.6	LOS A
Place		West	0.025	1.2	0.9	LOS A
(Roundabout)		Total	0.165	8.3	7.6	LOS A
		South	0.156	6.9	6.6	LOS A
		South-east	0.119	8.2	5.4	LOS A
	vveekend	West	0.010	1.4	0.4	LOS A
		Total	0.119	8.2	5.4	LOS A

Overall, the intersection of Dalgety Road and Towns Place performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.3 BGU03 – Kent Street/Argyle Street

The priority intersection, composed of Kent Street, Argyle Street and Argyle Place, is located north-east of Barangaroo Station. It connects the local roads of Argyle Street and Argyle Place in Barangaroo with Kent Street, a major local road that runs through the Sydney CBD.

Figure 5-33 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-33 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU03

Table 5-30 presents a performance summary of this intersection.

Table 5-30 Block 4 – Intersection p	performance summary of BGU03
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.750	18.3	55.6	LOS B
		East	0.349	5.0	13.6	LOS A
	Weekday AM	North	0.034	11.9	0.8	LOS A
		West	0.143	6.0	5.1	LOS A
		Total	0.750	18.3	55.6	LOS B
Kont Street/	Weekday PM	South	0.454	12.6	17.5	LOS A
Argyle		East	0.280	5.4	9.8	LOS A
Street		North	0.039	11.6	1.0	LOS A
(Priority –		West	0.186	5.9	6.5	LOS A
Give Way)		Total	0.454	12.6	17.5	LOS A
		South	0.586	16.7	24.9	LOS B
		East	0.304	6.3	10.9	LOS A
	Weekend	North	0.060	13.0	1.4	LOS A
		West	0.153	6.8	5.2	LOS A
		Total	0.586	16.7	24.9	LOS B

Overall, the intersection of Kent Street, Argyle Street and Argyle Place performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.4 BGU04 – Pedestrian Mid-block Crossing at Kent Street near Gas Lane

The signalised pedestrian mid-block crossing at Kent Street, near Gas Lane, is located south-east of Barangaroo Station. It offers a signalised pedestrian crossing over Kent Street near Gas Lane, a major local road that runs through the Sydney CBD. A dedicated cycleway runs along the east side of Kent Street; however, it was not considered for this assessment. The traffic signals at this intersection are coordinated with the intersection of Kent Street, Clarence Street and the Sydney Harbour Bridge on-ramp (BGU05).

Figure 5-34 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-34 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU04

Table 5-31 presents a performance summary of this intersection.

Table 5-31 Block 4 – Intersection performance summary of BGU04

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.486	4.8	52.6	LOS A
Pedestrian	Weekday AM	North	0.555	34.7	45.5	LOS C
Mid-block		Total	0.555	16.1	52.6	LOS B
Kent Street	Weekday PM	South	0.275	1.6	14.1	LOS A
near Gas		North	0.413	32.0	49.7	LOS C
(Pedestrian only – Signal)		Total	0.413	17.2	49.7	LOS B
	Weekend	South	0.342	4.9	30.5	LOS A
		North	0.482	29.5	36.1	LOS C
		Total	0.482	16.2	36.1	LOS B

Overall, the pedestrian mid-block crossing at Kent Street, near Gas Lane, performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Kent Street (south approach) extends back to the Kent Street/Clarence Street intersection during the weekday AM peak hour.

5.4.5 BGU05 – Kent Street/Sydney Harbour Bridge (SHB) On-ramp

The signalised intersection, composed of Kent Street, Clarence Street and the Sydney Harbour Bridge (SHB) on-ramp, is located south-east of Barangaroo Station. It connects the major local roads running through the Sydney CBD of Kent Street and Clarence Street with the Sydney Harbour Bridge on-ramp, providing northbound access to the M1 Motorway. A dedicated cycleway runs along the east side of Kent Street and the north side of the SHB on-ramp. Kent St (NE) cycleway was not assessed. The traffic signals at this intersection are co-ordinated with the pedestrian mid-block crossing at Kent Street, near Gas Lane (BGU04).

Figure 5-35 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-35 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU05

Table 5-32 presents a performance summary of this intersection	on.
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Table 5-32 Block 4 – Intersection performance summary of BGU05

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.563	15.1	82.7	LOS B
	Weekday	East	0.491	32.1	47.0	LOS C
Kent Street/	AM	North	0.509	35.0	40.4	LOS C
Sydney		Total	0.563	22.4	82.7	LOS B
Harbour Bridge	Weekday PM	South	0.441	9.6	63.4	LOS A
(SHB) On-		East	0.633	42.7	57.8	LOS D
ramp (Signal)		North	0.540	36.0	46.2	LOS C
		Total	0.633	21.6	63.4	LOS B
		South	0.376	16.7	43.4	LOS B
	Weekend	East	0.323	27.2	29.2	LOS B

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North	0.666	30.5	33.1	LOS C
		Total	0.666	22.5	43.5	LOS B

Overall, the intersection of Kent Street, Clarence Street and the SHB on-ramp performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.6 BGU06 – Hickson Road/Napoleon Street/Sussex Street

The signalised intersection, composed of Hickson Road, Napoleon Street, Sussex Street and a private parking facility is located south of Barangaroo Station. It connects the parking facility exit and local road of Napoleon Street with the regional roads of Hickson Road, which runs along the western waterfront of Barangaroo, and Sussex Street running through the Sydney CBD.

Figure 5-36 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-36 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU06

Table 5-33 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.347	12.7	50.4	LOS A
		East	0.420	26.5	43.5	LOS B
	Weekday AM	North	0.360	19.2	49.4	LOS B
		West	0.243	47.8	1.9	LOS D
		Total	0.420	18.9	50.4	LOS B
Hickson Road/		South	0.492	17.7	67.7	LOS B
Napoleon		East	0.464	31.8	40.3	LOS C
Sussex	Weekday PM	North	0.597	28.4	86.8	LOS B
Street		West	0.583	42.9	20.3	LOS D
(Signal)		Total	0.597	26.0	86.8	LOS B
Week		South	0.320	11.0	44.7	LOS A
		East	0.471	26.4	46.4	LOS B
	Weekend	North	0.423	16.5	58.3	LOS B
		West	0.195	44.3	1.5	LOS D
		Total	0.471	17.1	58.3	LOS B

Table 5-33 Block 4 – Intersection performance summary of BGU06

Overall, the intersection of Hickson Road, Napoleon Street and Sussex Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.7 BGU07 – Margaret Street/Kent Street/Napoleon Street

The signalised intersection, composed of Margaret Street, Kent Street and Napoleon Street, is located south-east of Barangaroo Station. It connects the local roads of Napoleon Street and Margaret Street in the Sydney CBD with Kent Street, a major local road that runs through the Sydney CBD. A dedicated cycleway runs along the east side of Kent Street.

Figure 5-37 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-37 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU07

Fable 5-34 presents a	performance summary	of this intersection.
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Table 5-34 Block 4 – Intersection performance summary of BGU07

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.674	25.0	124.2	LOS B
		East	0.744	31.8	65.3	LOS C
	Weekday AM	North	0.432	27.4	43.8	LOS B
	7 (17)	North-west	0.665	24.3	52.7	LOS B
		Total	0.744	26.8	124.2	LOS B
Margaret	Weekday	South	0.400	22.7	72.1	LOS B
Street/ Kent		East	0.435	17.7	37.0	LOS B
Napoleon		North	0.313	25.0	42.0	LOS B
Street		North-west	0.455	13.0	38.6	LOS A
(Signal)		Total	0.455	20.4	72.1	LOS B
		South	0.329	16.7	44.5	LOS B
Weel	Weekend	East	0.416	17.8	38.4	LOS B
		North	0.347	22.7	34.9	LOS B
		North-west	0.415	14.5	32.7	LOS A
		Total	0.416	17.8	44.5	LOS B

Overall, the intersection of Margaret Street, Kent Street and Napoleon Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Margaret Street (east approach) extends back to Clarence Street during the AM peak hour. Similarly, the 95th percentile queue on Kent Street (south approach) extends back to the pedestrian mid-block crossing near Margaret Street during the AM and PM peak hours.

5.4.8 BGU08 – Margaret Street/Clarence Street

The signalised intersection, composed of Margaret Street and Clarence Street, is located south-east of Barangaroo Station. It connects the local road of Margaret Street with Clarence Street, a major local road that runs through the Sydney CBD.

Figure 5-38 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-38 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU08

Table 5-35 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.706	22.7	83.5	LOS B
	Weekday	East	0.505	21.4	55.4	LOS B
	AM	West	0.862	48.5	63.1	LOS D
		Total	0.862	25.8	83.5	LOS B
Margaret	Weekdav	South	0.532	22.5	95.0	LOS B
Street/ Clarence		East	0.278	12.6	29.6	LOS A
Street	PM	West	0.627	40.9	55.3	LOS C
(Signal)		Total	0.627	22.3	95.0	LOS B
		South	0.226	16.5	27.5	LOS B
		East	0.191	9.5	15.0	LOS A
	vveekend	West	0.572	29.5	44.0	LOS C
		Total	0.572	17.3	44.0	LOS B

Table 5-35 Block 4 – Intersection performance summary of BGU08

Overall, the intersection of Margaret Street and Clarence Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Margaret Street (west approach) extends back to Kent

Street during all peak hours. Similarly, the 95th percentile queue on Margaret Street (east approach) extends back to York Street during the AM peak hour.

It was also noted that Block 4 pedestrian volumes at Margaret Street and Clarence Street significantly decreased during the weekday AM peak hour compared to pre-opening conditions. The drop in pedestrian volume may have been caused by pedestrians utilising Sydney Metro to travel to/from Barangaroo Station instead of connecting to Wynyard Station.

5.4.9 BGU09 – Margaret Street/York Street

The signalised intersection, composed of Margaret Street and York Street, is located south-east of Barangaroo Station. It connects the local road of Margaret Street with York Street, a major local road that runs through the Sydney CBD.

Figure 5-39 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-39 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU09

Table 5-36 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		East	0.194	22.8	22.3	LOS B
	Weekday	North	0.407	13.7	56.4	LOS A
	AM	West	0.170	35.6	18.9	LOS C
Margaret		Total	0.407	15.8	56.4	LOS B
Street/York		East	0.186	18.3	28.7	LOS B
Street	Weekday	North	0.375	20.7	63.0	LOS B
(Signal)	PM	West	0.268	31.8	37.0	LOS C
		Total	0.375	21.3	63.0	LOS B
		East	0.092	18.0	9.7	LOS B
	vveekend	North	0.293	13.7	38.9	LOS A

Table 5-36 Block 4 – Intersection performance summary of BGU09

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		West	0.289	29.4	32.7	LOS C
		Total	0.293	15.9	38.9	LOS B

Overall, the intersection of Margaret Street and York Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Margaret Street and York Street significantly decreased during the weekday AM peak hour compared to pre-opening conditions. The drop in pedestrian volume may have been caused by pedestrians utilising Sydney Metro to travel to/from Barangaroo Station instead of connecting to Wynyard Station.

5.4.10 BGU10 – Pedestrian Mid-block Crossing at Sussex Street under Exchange Place

The signalised pedestrian mid-block crossing at Sussex Street, under Exchange Place, is located south of Barangaroo Station. It offers a signalised pedestrian crossing over Sussex Street, a regional road that runs through the Sydney CBD.

Figure 5-40 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-40 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU10

Table 5-37 presents a performance summary of this intersection.

Table 5-37 Block 4 – Intersection performance summary of BGU10

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Pedestrian Mid-block Weekday	South	0.191	6.8	23.9	LOS A	
	Weekday AM	North	0.202	6.8	25.1	LOS A
Sussex		Total	0.202	6.8	25.1	LOS A

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Street under		South	0.249	8.1	30.8	LOS A
Exchange Place	Exchange Weekday	North	0.254	8.1	31.4	LOS A
(=)))	I IVI	Total	0.254	8.1	31.4	LOS A
(Pedestrian only – Signal) Weekend		South	0.217	7.9	28.1	LOS A
	Weekend	North	0.206	7.8	26.3	LOS A
		Total	0.217	7.8	28.1	LOS A

Overall, the pedestrian mid-block crossing at Sussex Street under Exchange Place performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.11 BGU11 – Pedestrian Mid-block Crossing at Kent Street near Margaret Street

The signalised pedestrian mid-block crossing at Kent Street, near Margaret Street, is located south of Barangaroo Station. It offers a signalised pedestrian crossing over Kent Street near Margaret Street, a major local road that runs through the Sydney CBD. A dedicated cycleway runs along the east side of Kent Street.

Figure 5-41 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-41 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU11

Table 5-38 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.445	10.7	39.7	LOS A
Pedestrian Mid-block	Weekday AM	North	0.255	9.6	20.7	LOS A
Crossing at	Zivi	Total	0.445	10.4	39.7	LOS A
Kent Street	Weekday PM Weekend	South	0.352	10.2	30.0	LOS A
Margaret		North	0.229	9.4	18.6	LOS A
Street		Total	0.352	9.9	30.0	LOS A
(Pedestrian only – Signal)		South	0.265	10.0	21.8	LOS A
		North	0.192	9.5	15.2	LOS A
Cignal		Total	0.265	9.8	21.8	LOS A

Table 5-38 Block 4 – Intersection performance summary of BGU11

Overall, the pedestrian mid-block crossing at Kent Street, near Margaret Street, performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at the Mid-block crossing at Kent Street near Margaret Street significantly increased during the weekday PM peak hour compared to pre-opening conditions. Exact reasons for the increase in pedestrian volume are unknown, however it may be unrelated to the Sydney Metro opening given the intersection is located well away from Barangaroo Station.

5.4.12 BGU12 – Sussex Street/Erskine Street

The signalised intersection, composed of Sussex Street and Erskine Street, is located south of Barangaroo Station. It connects the regional road of Sussex Street running through the Sydney CBD with the local road of Erskine Street.

Figure 5-42 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-42 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU12

Table 5-39 pres	ents a perform	ance summary	of this intersed	ction.		
Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.360	29.7	48.4	LOS C
		East	0.528	8.8	60.1	LOS A
	Weekday AM	North	0.294	22.7	45.4	LOS B
	AIVI	West	0.563	17.6	70.7	LOS B
		Total	0.563	18.1	70.7	LOS B
Succov		South	0.424	31.4	55.0	LOS C
Street/		East	0.457	10.2	59.8	LOS A
Erskine	Weekday DM	North	0.383	24.5	59.7	LOS B
Sileet	L IAI	West	0.493	15.5	59.3	LOS B
(Signal)		Total	0.493	19.6	59.8	LOS B
		South	0.494	31.2	69.6	LOS C
	Weekend	East	0.481	9.4	42.3	LOS A
		North	0.242	22.5	36.8	LOS B
		West	0.531	17.1	73.4	LOS B

Overall, the intersection of Sussex Street and Erskine Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Erskine Street (west approach) extends back to Shelley Street during all peak hours.

0.531

19.3

73.4

LOS B

Total

5.4.13 BGU13 – Kent Street/Erskine Street

The signalised intersection, composed of Kent Street and Erskine Street, is located south of Barangaroo Station. It connects the local road of Erskine Street with Kent Street, a major local road that runs through the Sydney CBD. A dedicated cycleway runs along the east side of Kent Street.

Figure 5-43 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-43 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU13

Table 5-40 presents a	performance summary	of this intersection.
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Table 5-40 Block 4 – Intersection performance summary of BGU13

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.278	13.6	47.2	LOS A
		East	0.452	36.0	46.3	LOS C
	Weekday AM	North	0.940	45.1	76.7	LOS D
	7 (1)1	West	0.422	32.7	40.3	LOS C
		Total	0.940	26.7	76.7	LOS B
	Weekday PM	South	0.362	25.1	49.7	LOS B
Kent Street/ Erskine		East	0.220	24.1	31.0	LOS B
Street		North	0.601	28.2	52.0	LOS B
(Signal)		West	0.253	26.7	38.8	LOS B
(eignai)		Total	0.601	25.9	52.0	LOS B
		South	0.184	16.4	29.3	LOS B
		East	0.479	48.4	40.5	LOS D
	Weekend	North	0.762	42.9	48.9	LOS D
		West	0.556	43.1	55.3	LOS D
		Total	0.762	33.8	55.3	LOS C

Overall, the intersection of Kent Street and Erskine Street performs satisfactorily at LOS C or better during all peak hours. The 95th percentile queue on Erskine Street (east approach) extends to Clarence Street during the AM and weekend peak hours.

5.4.14 BGU14 – Sussex Street/King Street

The signalised intersection, composed of Sussex Street and King Street, is located south of Barangaroo Station. It connects the King Street Western Distributor (A1) off-ramp with the regional road of Sussex Street, running through the Sydney CBD. A dedicated cycleway runs along the north side of King Street.

Figure 5-44 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-44 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU14

Table 5-41 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North	0.658	26.1	120.7	LOS B
	Weekday AM	South-west	0.802	27.3	193.4	LOS B
		Total	0.802	27.2	193.4	LOS B
Sussex Street/King	Weekday PM	North	0.653	18.6	144.4	LOS B
Street		South-west	0.621	25.8	110.0	LOS B
(Signal)		Total	0.653	24.1	144.4	LOS B
	Weekend	North	0.442	18.3	81.9	LOS B
		South-west	0.761	25.5	162.1	LOS B
		Total	0.761	23.8	162.1	LOS B

Table 5-41 Block 4 – Intersection performance summary of BGU14

Overall, the intersection of Sussex Street and King Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.15 BGU15 – Kent Street/King Street

The signalised intersection, composed of Kent Street and King Street, is located south of Barangaroo Station. It connects the local road of King Street with Kent Street, a major local road that runs through

the Sydney CBD. A dedicated cycleway runs along the east side of Kent Street and north side of King Street.

Figure 5-45 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-45 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU15

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.623	40.2	66.1	LOS C
	Weekday AM	West	0.553	6.0	37.8	LOS A
		Total	0.623	17.5	66.1	LOS B
Kent Street/	Weekday	South	0.442	32.4	64.2	LOS C
King Street		West	0.427	5.6	32.8	LOS A
(Signal)	1 111	Total	0.442	20.2	64.2	LOS B
	Weekend	South	0.424	38.2	45.3	LOS C
		West	0.418	8.2	62.6	LOS A
		Total	0.424	17.8	62.6	LOS B

Table 5-42 presents a performance summary of this intersection.

 Table 5-42 Block 4 – Intersection performance summary of BGU15

Overall, the intersection of Kent Street and King Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.16 BGU16 – New Pedestrian Mid-block Crossing at Hickson Road (north of metro station)

The unsignalised pedestrian mid-block crossing at Hickson Road (north of the metro station) is located directly east of Barangaroo Station.

Figure 5-46 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-46 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU16

Table 5-43 presents a	performance summary	of this intersection.
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.178	2.1	5.7	LOS A
New Pedestrian	Weekday AM	North	0.102	2.1	3	LOS A
		Total	0.178	2.1	5.7	LOS A
Crossing at	Weekday PM	South	0.241	2.1	7.9	LOS A
Hickson Road (porth		North	0.162	2.1	4.9	LOS A
of metro		Total	0.241	2.1	7.9	LOS A
station) (Pedestrian only - Zebra)	Weekend	South	0.328	2.1	12.1	LOS A
		North	0.181	2.1	5.5	LOS A
		Total	0.328	2.1	12.1	LOS A

 Table 5-43 Block 4 – Intersection performance summary of BGU16

Overall, the new pedestrian mid-block crossing at Hickson Road (north of metro station) performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.17 BGU17 – New Pedestrian Mid-block Crossing at Hickson Road (south of metro station)

The unsignalised pedestrian mid-block crossing at Hickson Road (south of the metro station) is located directly east of Barangaroo Station.

Figure 5-47 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-47 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU17

Table 5-44 presents a performance summary of this intersection.

Table 5-44 Block 4 – Intersection	performance summary of BGU17
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.183	2.2	5.9	LOS A
New	Weekday AM	North	0.105	2.2	3.1	LOS A
Pedestrian		Total	0.183	2.2	5.9	LOS A
Crossing at	Weekday PM	South	0.249	2.2	8.2	LOS A
Hickson Road (south		North	0.168	2.2	5.1	LOS A
of metro station) (Pedestrian only - Zebra)		Total	0.249	2.2	8.2	LOS A
	Weekend	South	0.343	2.3	12.6	LOS A
		North	0.189	2.3	5.8	LOS A
		Total	0.343	2.3	12.6	LOS A

Overall, the new pedestrian mid-block crossing at Hickson Road (south of metro station) performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.4.18 BGU18 – Shelley Street/Erskine Street

The signalised intersection, composed of Shelley Street and Erskine Street, is located south of Barangaroo Station. It connects the local roads of Erskine Street and Shelley Street in the Sydney CBD near the King Street Wharf.

Figure 5-48 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-48 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of BGU18

Fable 5-45 presents a	performance summary	of this intersection.
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Table 5-45 Block 4 – Intersection performance summary of BGU18

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.554	12.2	67.0	LOS A
		East	0.547	39.8	47.3	LOS C
	Weekday AM	North	0.091	10.1	6.8	LOS A
	7 (17)	West	0.457	36.7	37.1	LOS C
		Total	0.554	22.9	67.0	LOS B
Shallov	Weekday PM	South	0.382	12.6	39.0	LOS A
Street/		East	0.316	36.3	28.0	LOS C
Erskine Street		North	0.231	11.3	24.9	LOS A
Olicer		West	0.316	33.7	28.3	LOS C
(Signal)		Total	0.382	20.5	39.0	LOS B
		South	0.437	12.6	29.9	LOS A
	Weekend	East	0.637	45.2	70.3	LOS D
		North	0.106	11.7	15.3	LOS A
		West	0.300	31.1	35.3	LOS C
		Total	0.637	27.9	70.3	LOS B

Overall, the intersection of Shelley Street and Erskine Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Erskine Street (east approach) extends back to Sussex Street during the weekday AM and weekend peak hours.

5.4.19 Comparison with previous study blocks

Figure 5-49 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are slightly higher during all peak hours compared to the pre-opening conditions.



Figure 5-49 Study block comparison – Barangaroo Station peak hourly traffic volume across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-50 and Figure 5-51. All intersections in the Barangaroo Station study area perform at LOS C or better during Block 4, which is generally similar to previous study blocks.



Figure 5-50 Study block comparison – Barangaroo Station intersection performance summary (BGU01-BGU06 & BGU16-BGU17)

	BGU07 - MARGARET ST / KENT ST / NAPOLEON ST	BGU08 - MARGARET ST / CLARENCE ST
Towns Ala Hickson polymer Structure at 2	AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
Windmill Street over 3	asol asol asol asol asol asol asol asol	a sou a sou
Argyle Place		
Argyle Street	BLOCK 1 BLOCK 2 BLOCK 4 BLOCK 1 BLOCK 2 BLOCK 2 BLOCK 2 BLOCK 2 BLOCK 2 BLOCK 3 BLOCK 3 BLOCK 4	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4 BLOCK 2 BLOCK 4 BLOCK 2 BLOCK 2 BLOCK 4
Tano I and	BGU09 - MARGARET ST / YORK ST	BGU10 - PEDESTRIAN MID-BLOCK CROSSING AT SUSSEX ST
High High	AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
^{1a} a ₁ 5 ⁴ 06 ^{1a} a ₁ 5 ⁴ 06 ^{1a} a ₁ 5 ⁴ 06 ^{1a} a ₁ 5 ^{1a} a ₁ ^{1a} a ₁ 5 ^{1a} a ₁ 5	B SOJ B SOJ	LOSA LOSA LOSA LOSA LOSA LOSA LOSA LOSA
nkins Stree Goucester S Han Beorge Stree	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4 BLOCK 1 BLOCK 3 BLOCK 1 BLOCK 4 BLOCK 4 BLOCK 4	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4 BLOCK 1 BLOCK 4 BLOCK 1 BLOCK 1 BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 3 BLOCK 3 BLOCK 3
P Ken gi nalley Street	BGU11 - PEDESTRIAN MID-BLOCK CROSSING AT KENT ST	BGU12 - SUSSEX ST / ERSKINE ST
Grosvenor Street	AM PEAK PM PEAK WE PEAK	AM PEAK PM PEAK WE PEAK
ange drove		
anger oo Avenue BGU07	LOSA LOSA LOSA LOSA LOSA LOSA LOSA LOSA	a sol a sol
angaroo Avenue BGU10 BGU11 BGU	BLOCK1 LOSA BLOCK2 LOSA BLOCK3 LOSA BLOCK3 LOSA BLOCK3 LOSA BLOCK3 LOSA BLOCK4 LOSA BLOCK4 LOSA BLOCK4 LOSA BLOCK4 LOSA BLOCK4 LOSA BLOCK4 LOSA	BLOCK1 LOSB BLOCK2 LOSB BLOCK3 LOSB BLOCK4 LOSB BLOCK3 LOSB BLOCK4 LOSB BLOCK4 LOSB BLOCK3 LOSB BLOCK4 LOSB BLOCK3 LOSB BLOCK4 LOSB BLOCK3 LOSB BLOCK4 LOSB BLOCK4 LOSB BLOCK4 LOSB BLOCK4 LOSB BLOCK4 LOSB
angaroo Avenue BGU10 BGU11 BGU11 BGU12 BGU12 BGU12 BGU12 BGU12 BGU12 BGU13 BGU12 BGU13 BGU12 BGU14 BGU15 BGU	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 2 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A BLOCK 2 LOS A BLOCK 4 LOS A	BIO 000 K - 1000 K - 1
angaroo Avenue BGU10 BGU10 BGU11 BGU12 BGU13 BGU12 BGU13 BGU14 BGU	Noola Noola Noola Read 100 K 100 K Read <t< th=""><th>BB0118 - SHELLEY ST / ERSKINE ST BB0118 - SHELLEY ST / ERSKINE ST BB018 - SHELLEY ST / ERSKINE ST</th></t<>	BB0118 - SHELLEY ST / ERSKINE ST BB0118 - SHELLEY ST / ERSKINE ST BB018 - SHELLEY ST / ERSKINE ST
Andge Street BGU09 BGU11 BGU11 BGU11 BGU12 BGU12 BGU13 BGU13 BGU13 BGU13 BGU13 BGU14 BGU14 BGU15 BG	Iose Iose Iose Iose <t< th=""><th>B COMPARENT B COMPARENT COMPARENT B COMPARENT COMPARENT COMPARENT C</th></t<>	B COMPARENT B COMPARENT COMPARENT B COMPARENT COMPARENT COMPARENT C
Legend Pedestrian only - Signal	I I <th>B B B B B B B B B B B B B B C</th>	B B B B B B B B B B B B B B C
Legend Pedestrian only - Signal Signal	0K1 008 008 008 0K2 008 008 008 0K3 008 008 008 0K4 008 008 008	CK1 IOSB ICK2 IOSB IOSB IOSB IOSB IOSB IOSB IOSB IOSB IOSC IOSB IOSB

95

Figure 5-51 Study block comparison – Barangaroo Station intersection performance summary (BGU07-BGU13, BGU18)

Sydney Metro City and Southwest - Traffic and Interchange Monitoring Block 4 Report – Sydney Metro C&SW - Traffic and Interchange Monitoring



Figure 5-52 Study block comparison – Barangaroo Station intersection performance summary (BGU14-BGU15)

5.5 Martin Place Station

Martin Place Station is a new underground station and the fifth stop on the City & Southwest Line (towards Sydenham). It is located to west of the existing Martin Place Station (Sydney Trains) in Martin Place.

Martin Place Station has various accesses from Hunter Street, Castlereagh Street, Elizabeth Street and Martin Place. The accesses are separated into Martin Place North and Martin Place South. The four entrances are listed below:

- Martin Place North Entry (Hunter Street/Castlereagh Street); at the south-east corner of the intersection of Hunter Street, Castlereagh Street and Bligh Street
- Martin Place North Entry (Hunter Street/Elizabeth Street); at the south-west corner of the intersection of Hunter Street, Elizabeth Street and Chifley Square
- Martin Place South Entry (Castlereagh Street); along the eastern side of Castlereagh Street, north
 of King Street
- Martin Place South Entry (Elizabeth Street); along the western side of Elizabeth Street, north of King Street.

Bus services are available within approximately 150 metres of Martin Place Station, located at Elizabeth Street and Castlereagh Street. Pedestrian connections between the existing Martin Place Station platforms and metro station platforms are provided. The existing taxi ranks close to the station were retained. New bicycle parking racks are provided on Castlereagh Street and Elizabeth Street near the Martin Place North accesses.

The Martin Place Station study area consists of six intersections. Table 5-46 presents the peak hours utilised for modelling the intersections. Table 5-47 provides a summary of the intersection LOS while Figure 5-53 visualises a geospatial summary of the intersection LOS within the Martin Place Station study area.

Network Intersection		Weekday AM peak hour		Weekday I hoເ	PM peak ır	Weekend peak hour	
ID	ID	Day	Start time	Day	Start time	Day	Start time
	MPL01		8:30am	Wednesday	5:45pm	Saturday	6:15pm
	MPL02	Madaaaday					
MPL-N1	MPL03	wednesday					
	MPL04						
-	MPL05	Monday	8:15am	Wednesday	6:30pm	Saturday	5:30pm
-	MPL06	Wednesday	8:30am	Wednesday	5:15pm	Saturday	5:30pm

Table 5-46 Block 4 – Martin Place Station peak hours modelled

Table 5-47 Block 4 – Martin Place Station intersection performance summary

Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
MPL01	Hunter Street/Castlereagh Street/ Bligh Street (Signal)	LOS B	LOS B	LOS B	
MPL02	Hunter Street/Elizabeth Street/ Chifley Square (Signal)	LOS B	LOS C	LOS C	
MPL03	Bent Street/Bligh Street (Signal)	LOS A	LOS A	LOS A	
MPL04	Bent Street/Phillip Street (Signal)	LOS B	LOS B	LOS B	

Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
MPL05	Pedestrian Mid-block Crossing at Castlereagh Street (Signal)	LOS B	LOS A	LOS A	
MPL06	Pedestrian Mid-block Crossing at Elizabeth Street (Signal)	LOS A	LOS A	LOS A	

Overall, the intersection performance in the Martin Place Station study area during the peak hours is satisfactory, operating at LOS C or better.



Figure 5-53 Block 4 – Martin Place Station intersection performance summary

5.5.1 MPL01 – Hunter Street/Castlereagh Street/Bligh Street

The signalised intersection, composed of Hunter Street, Castlereagh Street and Bligh Street, is located directly north-west of Martin Place North. It connects the local roads of Bligh Street and Hunter Street in the Sydney CBD with Castlereagh Street, a major local road running through the Sydney CBD.

Figure 5-54 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

19.2

21.0

40.4

8.3

19.4

37.9

53.9

20.0

19.1

53.9

Figure 5-54 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of MPL01

Table 5-48 Block 4	4 – Intersection pe	erformance summ	ary of MPL01		
Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)
	Weekday AM	East	0.293	11.4	38.6
Hunter Street/ Castlereagh Street/ Bligh Street		North	0.638	54.5	33.4
		North-west	0.271	11.1	34.3
		Total	0.638	20.1	38.6
	Weekday PM	East	0.256	12.1	31.5
		North	0.704	55.7	37.2
		North-west	0.330	10.2	37.9

Total

East

North

North-west

Total

Weekend

Table 5-48 presents a performance summary of this intersection.

PLO
F

(Signal)

Overall, the intersection of Hunter Street, Castlereagh Street and Bligh Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Hunter Street (east approach) extends back to Elizabeth Street during the weekend peak hour.

0.704

0.447

0.599

0.173

0.599

LOS

LOS A LOS D LOS A LOS B LOS A LOS D LOS A

LOS B

LOS B

LOS C

LOS A

LOS B

It was also noted that Block 4 pedestrian volumes at Hunter Street and Castlereagh Street significantly increased across all peak hours compared to pre-opening conditions. Given that MPL01 is located in close proximity of Martin Place station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.5.2 MPL02 – Hunter Street/Elizabeth Street/Chifley Square

The signalised intersection, composed of Hunter Street, Elizabeth Street and Chifley Square, is located directly north-east of Martin Place North. It connects the local roads of Chifley Square and Hunter Street in the Sydney CBD with Elizabeth Street, a major local road linking the Sydney CBD and Waterloo.

Figure 5-55 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-55 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of MPL02

Table 5-49 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 [™] percentile queue (metres)	LOS
	Weekday AM	South	0.850	21.2	126.5	LOS B
		East	0.484	32.2	45.4	LOS C
		North-east	0.381	14.9	26.9	LOS B
Hunter Street/ Elizabeth Street/ Chifley Square (Signal)		West	0.537	32.0	65.3	LOS C
		Total	0.850	24.8	126.5	LOS B
	Weekday PM	South	0.947	33.9	209.6	LOS C
		East	0.419	32.8	38.1	LOS C
		North-east	0.351	28.5	52.9	LOS B
		West	0.531	32.8	65.3	LOS C
		Total	0.947	32.8	209.6	LOS C
	Weekend	South	0.903	41.0	199.9	LOS C
		East	0.652	38.9	39.3	LOS C

 Table 5-49 Block 4 – Intersection performance summary of MPL02

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North-east	0.288	17.5	25.6	LOS B
		West	0.354	24.4	50.1	LOS B
		Total	0.903	34.5	199.9	LOS C

Overall, the intersection of Hunter Street, Elizabeth Street and Chifley Square performs satisfactorily at LOS C or better during all peak hours. The 95th percentile queue on Elizabeth Street (south approach) extends back to the mid-block crossing on Elizabeth Street (MPL06) during all peak hours. Similarly, the 95th percentile queue on Hunter Street (west approach) extends back to Bligh Street during all peak hours.

It was also noted that Block 4 pedestrian volumes at Hunter Street, Elizabeth Street and Chiefly Square significantly increased during the weekday AM peak hour compared to pre-opening conditions. Given that MPL02 is located in close proximity of Martin Place station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.5.3 MPL03 – Bent Street/Bligh Street

The signalised intersection, composed of Bent Street and Bligh Street, is located north of Martin Place North. It connects the local roads of Bent Street and Bligh Street in the Sydney CBD, providing access to the major local road of Castlereagh Street further south.

Figure 5-56 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-56 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of MPL03

Table 5-50 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday AM	South-east	0.285	4.1	39.1	LOS A
Bent Street/ Bligh Street (Signal)		North-west	0.101	3.7	11.0	LOS A
		Total	0.285	4.0	39.1	LOS A
	Weekday PM	South-east	0.246	3.6	24.3	LOS A
		North-west	0.092	3.5	10.0	LOS A
		Total	0.246	3.6	24.3	LOS A
	Weekend	South-east	0.386	5.5	31.4	LOS A
		North-west	0.086	3.8	9.0	LOS A
		Total	0.386	5.1	31.4	LOS A

Table 5-50 Block 4 – Intersection performance summary of MPL03

Overall, the intersection of Bent Street and Bligh Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue on Bent Street (south-east approach) extends back to Phillip Street during the weekday AM and weekend peak hours.

It was also noted that Block 4 pedestrian volumes at Bent Street and Bligh Street significantly increased during the weekday AM peak hour compared to pre-opening conditions. Given that MPL03 is located in close proximity of Martin Place station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.5.4 MPL04 – Bent Street/Phillip Street

The signalised intersection, composed of Bent Street and Phillip Street, is located north of Martin Place North. It connects the local roads of Bent Street and Phillip Street in the Sydney CBD, providing access to the major local road of Elizabeth Street further south.

Figure 5-57 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-57 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of MPL04

Table 5-51 presents a performance summary of this intersection.
Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.596	23.5	85.4	LOS B
		North	0.212	20.0	33.3	LOS B
	Weekday ΔM	North-west	0.142	19.7	16.8	LOS B
		South-west	0.416	18.2	63.0	LOS B
Bent Street/ Phillip Street (Signal)		Total	0.596	20.5	85.4	LOS B
	Weekday PM	South-east	0.667	36.8	65.4	LOS C
		North	0.189	11.0	29.2	LOS A
		North-west	0.329	35.3	22.6	LOS C
		South-west	0.400	10.7	61.8	LOS A
		Total	0.667	18.8	65.4	LOS B
		South-east	0.390	26.3	46.8	LOS B
		North	0.158	13.5	21.8	LOS A
	Weekend	North-west	0.167	21.3	15.9	LOS B
		South-west	0.293	13.6	45.6	LOS A
		Total	0.390	17.7	46.8	LOS B

Table 5-51 Block 4 – Intersection performance summary of MPL04

Overall, the intersection of Bent Street and Phillip Street performs satisfactorily at LOS B during all peak hours. The 95th percentile queue lengths on Bent Street (south-east approach) extends back to Macquarie Street during the AM peak hour.

It was also noted that Block 4 pedestrian volumes at Hunter Street, Elizabeth Street and Chiefly Square significantly decreased during the weekday PM peak hour compared to pre-opening conditions. Exact reasons for the drop in pedestrian volumes are unknown.

5.5.5 MPL05 – Pedestrian Mid-block Crossing at Castlereagh Street

The signalised pedestrian mid-block crossing at Castlereagh Street is located directly north-west of Martin Place South. It offers a signalised pedestrian crossing over Castlereagh Street, a major local road that runs through the Sydney CBD.

During Block 4, the configuration of the intersection was restructured as two approach/departure lanes (previously three approach/departure lanes in pre-opening conditions). Additionally, the eastern kerbside approach lane of Castlereagh Street was altered to be a partial bus and general traffic lane.

Figure 5-58 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-58 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of MPL05

Table 5-52 presents a performance summary of this intersection.

Fable 5-52 Block 4 – Intersection	performance summary of MPL05
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Pedestrian Mid-blockWeekday AMCrossing at Castlereagh StreetWeekday PM	Weekday	North	0.429	15.5	29.5	LOS B
	AM	Total	0.429	15.5	29.5	LOS B
	Weekday PM	North	0.281	12.3	21.2	LOS A
		Total	0.281	12.3	21.2	LOS A
(Pedestrian only – Weel Signal)		North	0.180	12.6	12.6	LOS A
	Weekend	Total	0.180	12.6	12.6	LOS A

Overall, the pedestrian mid-block crossing at Castlereagh Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at the mid-block crossing at Castlereagh Street significantly increased during the weekday AM peak hour compared to pre-opening conditions. Given that MPL05 is located in close proximity of Martin Place station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.5.6 MPL06 – Pedestrian Mid-block Crossing at Elizabeth Street

The signalised pedestrian mid-block crossing at Elizabeth Street is located directly north-east of Martin Place South. It offers a signalised pedestrian crossing over Elizabeth Street, a major local road linking the Sydney CBD and Waterloo.

Figure 5-59 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Figure 5-59 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of MPL06

Fable 5-53 presents a	performance summary	of this intersection.
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.424	8.8	77.9	LOS A
Pedestrian Mid-block Crossing at Elizabeth Street (Pedestrian only – Signal)	Weekday AM	North	0.338	7.6	58.3	LOS A
		Total	0.424	9.7	77.9	LOS A
	Weekday PM	South	0.431	8.9	80.3	LOS A
		North	0.323	7.5	55.2	LOS A
		Total	0.431	9.9	80.3	LOS A
		South	0.374	8.4	66.2	LOS A
	Weekend	North	0.191	6.8	29.4	LOS A
		Total	0.374	9.5	66.2	LOS A

 Table 5-53 Block 4 – Intersection performance summary of MPL06

Overall, the pedestrian mid-block crossing at Elizabeth Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.5.7 Comparison with previous study blocks

Figure 5-60 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are relatively similar during the AM and PM peak hours, and slightly higher during the weekend peak hour compared to pre-opening conditions.



Figure 5-60 Study block comparison – Martin Place Station peak hourly traffic volumes across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-61. All intersections in the Martin Place Station study area perform at a LOS C or better during Block 4, which is generally similar to previous study blocks.

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Figure 5-61 Study block comparison – Martin Place Station intersection performance summary

5.6 Gadigal Station

Gadigal Station (previously Pitt Street Station) is a new underground station and the sixth stop on the City & Southwest Line (towards Sydenham). It is located at the junction of Sydney's southern CBD and the midtown retail precinct.

Gadigal Station has accesses from both Park Street and Bathurst Street. The two entrances are listed below:

- Park Street Entry; along Park Street between Pitt Street and Castlereagh Street
- Bathurst Street Entry; along Bathurst Street between Pitt Street and Castlereagh Street.

The over station developments at Gadigal Station were still under construction during Block 4, including some of the public domain works.

Several bus routes operate within the vicinity of the new Gadigal Station. Bus services are available within approximately 100 metres of Gadigal Station, located at Elizabeth Street and Park Street. The CBD and South-East Light Rail also operates nearby along George Street.

New bicycle parking racks are available on Park Street and Bathurst Street, near the station entrances.

The Gadigal Station study area consists of four intersections. Table 5-54 presents the peak hours utilised for modelling the intersections. Table 5-55 provides a summary of the intersection LOS while Figure 5-62 visualises a geospatial summary of the intersection LOS within the Gadigal Station study area.

Table 5-54 Block 4 – Gadigal Station peak hours modelled

Network	Intersection	Weekda <u>y</u> h	eekday AM peak Weekday PM peak hour hour		PM peak Weekend peak hour		
ID ID		Day	Start time	Day	Start time	Day	Start time
	PIT01			Thursday	5:00pm	Saturday	1:45pm
PIT-N1	PIT02	T I In	9:00am				
	PIT03	Thursday					
	PIT04						

Table 5-55 Block 4 – Gadigal Station intersection performance summary

Intersection		LOS		
ID	ID Intersection		Weekday PM Peak	Weekend Peak
PIT01	Pitt Street/Bathurst Street (Signal)	LOS B	LOS B	LOS A
PIT02	Castlereagh Street/Bathurst Street (Signal)	LOS A	LOS B	LOS A
PIT03	Park Street/Castlereagh Street (Signal)	LOS B	LOS C	LOS B
PIT04	Park Street/Pitt Street (Signal)	LOS B	LOS B	LOS A

Overall, the intersection performance in the Gadigal Station study area during the peak hours is satisfactory, operating at LOS C or better.



Figure 5-62 Block 4 – Gadigal Station intersection performance summary

5.6.1 PIT01 – Pitt Street/Bathurst Street

The signalised intersection, composed of Pitt Street and Bathurst Street, is located directly west of the Bathurst Street access to Gadigal Station. It connects the major local road of Pitt Street and major regional road of Bathurst Street running through the inner Sydney CBD.

During Block 4, the available storage on the right turn kerbside lane on Pitt Street (south approach) was reduced during the AM and PM peak hours due to the a construction work zone. Additionally, Pitt Street (south approach) was closed during the weekend peak hour to facilitate construction works.

Figure 5-63 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-63 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of PIT01

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.618	31.0	27.9	LOS C
Pitt Street/ Bathurst Street (Signal)	Weekday AM	West	0.437	10.7	40.6	LOS A
		Total	0.618	15.4	40.6	LOS B
	Weekday PM	South	0.427	31.5	39.1	LOS C
		West	0.341	10.4	57.1	LOS A
		Total	0.427	15.7	57.1	LOS B
		South	0.001	15.8	0.0	LOS B
	Weekend	West	0.361	9.9	33.0	LOS A
		Total	0.361	10.0	33.0	LOS A

Table 5-56 presents a performance summary of this intersection.

Table 5-56 Block 4 – Intersection	performance summary	of PIT01

Overall, the intersection of Pitt Street and Bathurst Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Pitt Street and Bathurst Street significantly increased during the weekday AM and weekend peak hours compared to pre-opening conditions. Given that PIT01 is located along the frontage of Gadigal station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.6.2 PIT02 – Castlereagh Street/Bathurst Street

The signalised intersection, composed of Castlereagh Street and Bathurst Street, is located directly east of the Bathurst Street access to Gadigal Station. It connects the major local road of Castlereagh Street and major regional road of Bathurst Street running through the inner Sydney CBD.

During Block 4, the western kerb side lane on Castlereagh Street (north approach) was closed off due to construction works.

Figure 5-64 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-64 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of PIT02

Tuble o of block + Intersection performance summary of the	Table 5-57	Block 4 -	Intersection	performance	summary of PIT02
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North	0.389	17.1	40.3	LOS B
Castlereagh Street/ Bathurst Street (Signal)	Weekday AM	West	0.324	4.9	25.9	LOS A
	7 (17)	Total	0.389	7.3	40.3	LOS A
	Weekday PM	North	0.260	17.6	40.1	LOS B
		West	0.397	19.3	66.5	LOS B
		Total	0.397	19.0	66.5	LOS B
		North	0.233	13.0	16.6	LOS A
	Weekend	West	0.306	3.0	9.4	LOS A
		Total	0.306	4.4	16.6	LOS A

Overall, the intersection of Castlereagh Street and Bathurst Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Castlereagh Street and Bathurst Street significantly decreased during the weekday AM peak hour compared to pre-opening conditions. Exact reasons for the decrease in pedestrian volumes are unknown, however there were significant construction works occurring on the footpaths at PIT02 which may have deterred pedestrians from crossing at this intersection and walking to adjacent intersections.

5.6.3 PIT03 – Park Street/Castlereagh Street

The signalised intersection, composed of Park Street and Castlereagh Street, is located directly east of the Park Street access to Gadigal Station. It connects the major regional road of Park Street and major local road of Castlereagh Street running through the inner Sydney CBD.

During Block 4, the northern kerbside lane on Park Street (west approach) was occupied by a work zone during the AM peak hour. The kerbside departure lane on Park Street (west approach) was also occupied by a work zone during the AM and weekend peak hour. Additionally, the western kerbside lane on Castlereagh Street (southern leg) was closed during all peak hours due to construction works.

Figure 5-65 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.

Park St (W)



Source: Nearmap (September 2024)

Figure 5-65 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of PIT03

Table 5-58 presents a performance summary of this intersection.

Tahlo 5-58	R Block 4 -	Intersection	nerformance	summary	of PITO3
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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		East	0.491	18.6	77.4	LOS B
Park	Weekday AM	North	0.644	50.7	59.7	LOS D
Street/ Castlereagh		West	0.307	15.4	43.6	LOS B
Street		Total	0.644	27.4	77.4	LOS B
(Signal)	Weekday	East	0.308	16.8	52.8	LOS B
	PM	North	0.753	51.1	90.3	LOS D

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		West	0.213	16.6	27.1	LOS B
		Total	0.753	32.6	90.3	LOS C
		East	0.448	9.6	75.0	LOS A
		North	0.560	47.7	49.7	LOS D
	vveekend	West	0.090	10.1	10.2	LOS A
			0.560	21.8	75.0	LOS B

Overall, the intersection of Park Street and Castlereagh Street performs satisfactorily at LOS C or better during all peak hours. The 95th percentile queue on Park Street (east approach) extends back to Elizabeth Street during all peak hours.

5.6.4 PIT04 – Park Street/Pitt Street

The signalised intersection, composed of Park Street and Pitt Street, is located directly west of the Park Street access to Gadigal Station. It connects the major regional road of Park Street and major local road of Pitt Street running through the inner Sydney CBD.

During Block 4, the kerbside approach and departure lanes of Park Street (east approach) was occupied by a work zone towards Castlereagh Street during the AM and PM peak hours, whereas the weekend peak hour was only affected by the work zone on the kerbside departure lane of Park Street (east approach).

Figure 5-66 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-66 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of PIT04

Table 5-59 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.458	20.6	32.2	LOS B
	Weekday	East	0.796	18.5	81.1	LOS B
	AM	West	0.194	10.7	14.5	LOS A
		Total	0.796	18.8	18.8 81.1	
	Weekday	South	0.417	19.7	27.6	LOS B
Park Street/ Pitt Street		East	0.657	13.0	59.0	LOS A
(0) 1)	PM	West	0.238	10.9	18.3	LOS A
(Signal)		Total	0.657	15.6	59.0	LOS B
		South	0.262	17.3	17.1	LOS B
		East	0.732	12.8	76.8	LOS A
	Weekend	West	0.111	8.9	8.4	LOS A
		Total	0.732	13.8	76.8	LOS A

Table 5-59 Block 4 – Intersection performance summary of PIT04

Overall, the intersection of Park Street and Pitt Street performs satisfactorily at LOS B or better during all peak hours. The 95th percentile queue on Park Street (east approach) extends back to Castlereagh Street during the AM and weekend peak hour.

It was also noted that Block 4 pedestrian volumes at Park Street and Pitt Street significantly increased during the weekday PM and weekend peak hours compared to pre-opening conditions. Given that PIT04 is located along the frontage of Gadigal station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.6.5 Comparison with previous study blocks

Figure 5-67 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, traffic volumes are relatively similar during the AM and PM peak hours, and lower during the weekend peak hour compared to pre-opening conditions.



Figure 5-67 Study block comparison – Gadigal Station peak hourly traffic volumes across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-68. All intersections in the Gadigal Station study area perform at LOS C or better during Block 4, which is generally similar to previous study blocks.

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			PIT01 - I	PITT S	T / BATH	URST	ST					PIT02 -	CAST	LEREA	GH ST	/ BATH	JRST S	г		
	A	M PEAK		F	M PEAK			WE P	EAK		AM P	EAK		F	M PE	٨K	1	WE F	PEAK	
York	LOSA	LOS B LOS B	LOSB	LOSB	LOSB	LOSB	VSOT	LOS A	LOSA LOSA	LOSA	LOSA	LOSA	5	LOSA	LOSA	LOSB	V SOT	LOSA	LOSA	LOSA
Street Stlereagh S	BLOCK 1	BLOCK 3	BLOCK 4	BLOCK 1	BLOCK 2 BLOCK 3	BLOCK 4	BLOCK 1	BLOCK 2	BLOCK 3 BLOCK 4	BLOCK 1	BLOCK 2	BLOCK 4		BLOCK 1	BLOCK 2 BLOCK 3	BLOCK 4	BLOCK 1	BLOCK 2	BLOCK 3	BLOCK 4
Ca Ca		PIT	03 - PAF	RK ST	CASTL	EREAG	H ST						PIT04	4 - PAR	K ST /	PITT ST				
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Park Street			_					_		_	_								_ <u> </u>	<u>}</u>
PITO3 treet treet	BLOCK 1	BLOCK 3	BLOCK 4	BLOCK 1	BLOCK 2 BLOCK 3	BLOCK 4	BLOCK 1	BLOCK 2	BLOCK 3 BLOCK 4	BLOCK 1	BLOCK 2	BLOCK 3		BLOCK 1	BLOCK 2 BLOCK 2	BLOCK 4	BLOCK 1	BLOCK 2	BLOCK 3 PLOCK 3	
PITO1 PITO2 Cross City Tunnel Bathurst Street Wilmot Street																				
Legend \$ Signal Station Access Street																				

Figure 5-68 Study block comparison – Gadigal Station intersection performance summary

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5.7 Central Station

Central Station is an existing station and the seventh stop on the City & Southwest Line (towards Sydenham). It is located at the southern end of the Sydney CBD, directly south of Belmore Park between Pitt Street and Elizabeth Street.

The Sydney Metro lines were built under the existing platforms 13, 14 and 15 in Central Station. In addition to the existing seven entrances, a new eastern entrance was recently opened on Chalmers Street providing access to both train and metro platforms.

Bus services are available within approximately 100 metres of Central Station, located at Eddy Avenue, Pitt Street, Lee Street and Elizabeth Street. Dedicated cycle lanes are currently provided along Elizabeth Street and Eddy Avenue near Central Station.

The Central Station study area consists of five intersections. During Block 4, one intersection (CEN04) which will be a new pedestrian mid-block crossing had not yet been constructed. Table 5-60 presents the peak hours utilised for modelling the intersections. Table 5-61 provides a summary of the intersection LOS while Figure 5-69 visualises a geospatial summary of the intersection LOS within the Central Station study area.

Table	5-60	Block 4	4 – 1	Central	Station	peak	hours	modelled
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Network Intersection		Weekday A hou	M peak r	Weekday hoเ	PM peak Jr	Weekend peak hour		
ID	ID	Day	Start time	Day	Start time	Day	Start time	
	CEN01		0.15 am		F:4Fnm	Coturdou	10.00	
CEIN-INT	CEN02	wednesday	0.15am	weunesuay	5. ropin	Saluruay	12.00pm	
	CEN03		0.45	Thursday	5 -00	Ostundau	40.00	
CEN-INZ	CEN05	vvednesday	8:15am	Thursday	5:30pm	Saturday	12:00pm	
CEN04 Under construction.								

Table 5-61 Block 4 – Central Station intersection performance summary

Intersection			LOS	
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak
CEN01	Elizabeth Street/Eddy Avenue (Signal)	LOS B	LOS B	LOS B
CEN02	Elizabeth Street/Foveaux Street (Signal)	LOS B	LOS C	LOS B
CEN03	Elizabeth Street/Cooper Street (Priority – Give Way)	LOS A	LOS A	LOS A
CEN04	New Pedestrian Mid-block Crossing at Randle Lane (Pedestrian only – Signal)		Not operational	
CEN05	Elizabeth Street/Randle Street (Signal)	LOS A	LOS A	LOS A

Overall, the intersection performance in the Central Station study area during the peak hours is satisfactory, operating at LOS C or better.



Figure 5-69 Block 4 – Central Station intersection performance summary

5.7.1 CEN01 – Elizabeth Street/Eddy Avenue

The signalised intersection, composed of Elizabeth Street and Eddy Avenue, is located north-east of Central Station. It connects the regional roads of Eddy Avenue, running through the Sydney CBD, and Elizabeth Street, linking the Sydney CBD and Waterloo. The traffic signals at this intersection are coordinated with the intersection of Elizabeth Street and Foveaux Street (CEN02).

Figure 5-70 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-70 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CEN01

Table 5-62 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekdav	South	0.728	12.3	57.1	LOS A
		North	0.733	36.3	155.1	LOS C
	AM	North-west	0.832	30.0	78.1	LOS C
		Total	0.832	22.5	155.1	LOS B
Elizabeth	Weekday	South	0.618	10.4	57.1	LOS A
Street/ Eddy		North	0.864	51.2	198.2	LOS D
Avenue	PM	North-west	0.627	22.6	60.6	LOS B
(Signal)		Total	0.864	26.6	198.2	LOS B
		South	0.405	6.2	44.4	LOS A
		North	0.458	31.7	53.9	LOS C
	vveekend	North-west	0.615	26.6	48.4	LOS B
		Total	0.615	18.0	53.9	LOS B

Table 5-62 Block 4 – Intersection performance summary of CEN01

Overall, the intersection of Elizabeth Street and Eddy Avenue performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Elizabeth Street (north approach) extends back to Albion

Street during the weekday AM and PM peak hours. Similarly, the 95th percentile queue on Eddy Avenue (north-west approach) extends back to the pedestrian mid-block crossing on Eddy Avenue during all peak hours and the queues on Elizabeth Street (south approach) extends back to the intersection of Elizabeth Street/Foveaux Street (CEN02) during all peak hours.

5.7.2 CEN02 – Elizabeth Street/Foveaux Street

The signalised intersection, composed of Elizabeth Street and Foveaux Street, is located east of Central Station. It connects the regional roads of Foveaux Street, running through Surry Hills, and Elizabeth Street, linking the Sydney CBD and Waterloo. The traffic signals at this intersection are co-ordinated with the intersection of Elizabeth Street and Eddy Avenue (CEN01).

Figure 5-71 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Figure 5-71 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CEN02

Table 5-63 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday AM	South	0.636	27.9	150.5	LOS B
Flizabeth		South-east	0.628	27.8	80.2	LOS B
		North	0.385	10.4	57.1	LOS A
		Total	0.636	23.2	150.5	LOS B
Street/	Weekdav	South	0.554	32.1	110.8	LOS C
Foveaux Street		South-east	0.890	45.0	160.7	LOS D
Olicer	PM	North	0.500	10.8	57.1	LOS A
(Signal)		Total	0.890	30.7	160.7	LOS C
		South	0.498	25.3	103.5	LOS B
	Weekend	South-east	0.310	22.7	50.2	LOS B
		North	0.307	9.2	47.3	LOS A

Table 5-63 Block 4 – Intersection performance summary of CEN02

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		Total	0.498	19.9	103.5	LOS B

Overall, the intersection of Elizabeth Street and Foveaux Street performs satisfactorily at LOS C or better during all peak hours. The 95th percentile queue on Elizabeth Street (north approach) extends back to Eddy Avenue during all peak hours. Similarly, the 95th percentile queue on Elizabeth Street (south approach) extends back to Randle Street during the AM peak hour and the queues on Foveaux Street extend back to Commonwealth Street during the PM peak hour.

5.7.3 CEN03 – Elizabeth Street/Cooper Street

The priority intersection, composed of Elizabeth Street and Cooper Street, is located east of Central Station. It connects the local road of Cooper Street with the regional road of Elizabeth Street, linking the Sydney CBD to Waterloo.

Figure 5-72 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Figure 5-72 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CEN03

Table 5-64 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
El al al		South-east	0.091	6.9	2.6	LOS A
Elizabeth Street/	Weekday AM	North	0.224	3.6	5.3	LOS A
Cooper		Total	0.091	6.9	2.6	LOS A
Street		South-east	0.135	8.4	3.7	LOS A
(Priority – Give Way)	Weekday PM	North	0.298	3.5	4.0	LOS A
	PIN	Total	0.135	8.4	3.7	LOS A

Table 5-64 Block 4 – Intersection performance summary of CEN03

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.068	5.9	1.9	LOS A
	Weekend	North	0.180	2.8	2.5	LOS A
		Total	0.068	5.9	1.9	LOS A

Overall, the intersection of Elizabeth Street and Cooper Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.7.4 CEN04 – New Pedestrian Mid-block Crossing at Randle Lane

The proposed pedestrian mid-block crossing at Randle Lane would be located directly east of Central Station. During Block 4, the mid-block crossing was yet to be constructed and therefore was not operational. As such, it was not assessed as part of the Block 4 study.

5.7.5 CEN05 – Elizabeth Street/Randle Street

The signalised intersection, composed of Elizabeth Street and Randle Street, is located east of Central Station. It connects the local road of Randle Street with the regional road of Elizabeth Street, linking the Sydney CBD to Waterloo.

During Block 4, the kerbside bus lane on Randle Street (south-western approach) was closed off due to construction works.

Figure 5-73 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-73 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of CEN05

Table 5-65 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95™ percentile queue (metres)	LOS
		North	0.258	2.4	34.4	LOS A
	Weekday AM	South-west	0.404	7.1	51.8	LOS A
Elizabeth	7 (10)	Total	0.404	5.1	51.8	LOS A
Street/		North	0.346	2.9	52.9	LOS A
Randle Street	Weekday PM	South-west	0.313	6.2	35.5	LOS A
(Signal)	I IVI	Total	0.346	4.3	52.9	LOS A
		North	0.220	2.2	26.9	LOS A
	Weekend	South-west	0.310	6.1	32.7	LOS A
		Total	0.310	4.3	32.7	LOS A

Table 5-65 Block 4 – Intersection performance summary of CEN05

Overall, the intersection of Elizabeth Street and Randle Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.7.6 Comparison with previous study blocks

Figure 5-74 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are relatively similar in the AM and PM peak hours, and lower in the weekend peak hour compared to pre-opening conditions.



Figure 5-74 Study block comparison - Central Station peak hourly traffic volumes across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-75. All intersections in the Central Station study area perform at LOS C or better during Block 4, which is generally similar to previous study blocks.

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Figure 5-75 Study block comparison – Central Station intersection performance summary

5.8 Waterloo Station

Waterloo Station is a new underground station and the eighth stop on the City & Southwest Line (towards Sydenham). It is located in the north-western quadrant of Waterloo, bounded by Botany Road, Cope Street, Raglan Street and Wellington Street.

Waterloo Station has accesses on both Raglan Street and Cope Street. The two entrances are listed below:

- Raglan Street Entry; at the south-west corner of the intersection of Raglan Street and Cope Street
- Cope Street Entry; along the western side of Cope Street, north of Wellington Street.

Bus services are available within approximately 150 metres of Waterloo Station, located along Botany Road. The existing bus stops will be retained for northbound routes, and the existing bus stops for southbound routes will be relocated to the mid-block on Botany Road between Raglan Street and Wellington Street. A new on-road marked cycle link will be provided along Wellington Street.

The Waterloo Station study area consists of six intersections. Table 5-66 presents the peak hours utilised for modelling the intersections.

Table 5-67 provides a summary of the intersection LOS while Figure 5-76 visualises a geospatial summary of the intersection LOS within the Waterloo Station study area.

Network	Intersection	Weekday / hoເ	Weekday AM peak hour		PM peak Ir	Weekend peak hour	
ID	ID	Day	Start time	Day	Start time	Day	Start time
	WLO01		ay 7:45am		4:45pm	Saturday	12:30pm
	WLO02	Thursday		Thursday			
	WLO03						
	WLO04						
	WLO05						
	WLO06						

Table 5-66 Block 4 – Waterloo Station peak hours modelled

Table 5-67 Block 4 – Waterloo Station intersection performance summary

Intersection			LOS	
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak
WLO01	Botany Road/Raglan Street/ Henderson Road (Signal)	LOS B	LOS C	LOS C
WLO02	Raglan Street/Cope Street (Signal)	LOS A	LOS A	LOS A
WLO03	Botany Road/Wellington Street/ Buckland Street (Signal)	LOS A	LOS A	LOS A
WLO04	Cope Street/Wellington Street (Stop)	LOS A	LOS A	LOS A
WLO05	Wyndham Street/Henderson Road (Signal)	LOS B	LOS C	LOS C
WLO06	New Pedestrian Mid-block Crossing at Cope Street (Pedestrian only – Zebra)	LOS A	LOS A	LOS A

Overall, the intersection performance in the Waterloo Station study area during the peak hours is satisfactory, operating at LOS C or better.



Figure 5-76 Block 4 – Waterloo Station intersection performance summary

5.8.1 WLO01 – Botany Road/Raglan Street/Henderson Road

The signalised intersection, composed of Botany Road, Raglan Street and Henderson Road, is located directly north-west of Waterloo Station. It connects the local road of Raglan Street in Waterloo with the State roads of Botany Road, linking Waterloo and Matraville, and Henderson Road, linking Waterloo and Eveleigh.

Figure 5-77 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-77 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of WLO01

Table 5-68 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.689	30.9	133.5	LOS C
		East	0.432	51.2	42.9	LOS D
	Weekday AM	North	0.681	22.6	93.7	LOS B
	7.101	West	0.622	16.0	28.0	LOS B
Botany		Total	0.689	26.8	133.5	LOS B
Road/		South	0.916	72.7	169.9	LOS F
Ragian Street/		East	0.803	62.7	67.0	LOS E
Henderson	Weekday PM	North	0.849	26.3	145.3	LOS B
Road	E IVI	West	0.904	23.7	68.1	LOS B
(Signal)		Total	0.916	40.4	169.9	LOS C
		South	0.556	45.6	103.5	LOS D
		East	0.508	55.1	44.4	LOS D
	vveekend	North	0.792	28.0	126.5	LOS B
		West	0.708	18.3	29.4	LOS B

Table 5-68 Block 4 – Intersection	performance summary	of WI	001
Table 3-00 block 4 - Intel Section	periormance summary		.001

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		Total	0.792	33.3	126.5	LOS C

Overall, the intersection of Botany Road, Raglan Street and Henderson Road performs satisfactorily at LOS C or better during all peak hours. The 95th percentile queue on Raglan Street (east approach) extends back to Cope Street during the weekday PM peak hour. Similarly, the 95th percentile queue on Henderson Road (west approach) extend back to the intersection of Wyndham Street/Henderson Street (WLO05) during the weekday PM peak hour.

It was also noted that Block 4 pedestrian volumes at Botany Road, Raglan Street and Henderson Road significantly increased across all peak hours compared to pre-opening conditions. Given that WLO01 is located along the frontage of Waterloo station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.8.2 WLO02 – Raglan Street/Cope Street

The signalised intersection, composed of Raglan Street and Cope Street, is located directly north-east of Waterloo Station. It connects the local roads of Raglan Street and Cope Street in Waterloo. During Block 4, the intersection was redesigned from a roundabout intersection to a signalised intersection to cater for the opening of the Waterloo metro station.

Figure 5-78 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Figure 5-78 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of WLO02

Table 5-69 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
Raglan	Weekday	South	0.141	14.0	6.0	LOS A
Street/Cope Street		East	0.185	9.6	10.3	LOS A
(Signal)		North	0.302	19.5	5.6	LOS B

Table 5-69 Block 4 – Intersection performance summary of WLO02

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Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		West	0.334	10.7	20.6	LOS A
		Total	0.334	11.5	20.6	LOS A
		South	0.114	17.4	5.7	LOS B
	Weekday	East	0.218	10.8	15.2	LOS A
		North	0.252	22.5	8.5	LOS B
	I IVI	West	0.269	11.6	21.5	LOS A
		Total	0.269	12.9	21.5	LOS A
		South	0.077	13.1	3.3	LOS A
		East	0.168	9.2	9.6	LOS A
	Weekend	North	0.195	18.3	4.7	LOS B
		West	0.324	10.5	20.0	LOS A
		Total	0.324	11.0	20.0	LOS A

Overall, the intersection of Raglan Street and Cope Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Raglan Street and Cope Street significantly increased across all peak hours compared to pre-opening conditions. Given that WLO02 is located along the frontage of Waterloo station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.8.3 WLO03 – Botany Road/Wellington Street/Buckland Street

The signalised intersection, composed of Botany Road, Wellington Street and Buckland Street, is located directly south-west of Waterloo Station. It connects the local roads of Wellington Street in Waterloo and Buckland Street, linking Waterloo and Alexandria, with the State road of Botany Road, linking Waterloo and Matraville.

Figure 5-79 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-79 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of WLO03

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.361	5.2	66.4	LOS A
		East	0.222	58.8	13.6	LOS E
	Weekday AM	North	0.314	3.7	47.6	LOS A
	7 (1)1	West	0.340	52.8	30.8	LOS D
		Total	0.361	8.6	66.4	LOS A
Botany Road/	Weekday PM	South	0.363	6.1	71.0	LOS A
Wellington		East	0.319	59.1	25.1	LOS E
Street/ Buckland		North	0.365	2.5	41.7	LOS A
Street		West	0.306	52.6	30.3	LOS D
(Signal)		Total	0.365	8.9	71.0	LOS A
(eignai)		South	0.428	7.3	74.7	LOS A
N		East	0.214	59.4	17.6	LOS E
	Weekend	North	0.561	8.3	82.0	LOS A
		West	0.341	53.6	33.8	LOS D
		Total	0.561	13.6	82.0	LOS A

 Table 5-70 presents a performance summary of this intersection.

 Table 5-70 Block 4 – Intersection performance summary of WLO03

Overall, the intersection of Botany Road, Wellington Street and Buckland Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Botany Road, Wellington Street and Buckland Street significantly increased during the weekend peak hour compared to pre-opening conditions. Given that WLO03 is located along the frontage of Waterloo station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.8.4 WLO04 – Cope Street/Wellington Street

The priority intersection, composed of Cope Street and Wellington Street, is located directly south-east of Waterloo Station. It connects the local roads of Cope Street, linking Waterloo and Redfern, and Wellington Street in Waterloo. During Block 4, the intersection was redesigned from a roundabout intersection to a priority intersection to cater for the opening of the Waterloo metro station.

Figure 5-80 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.

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Source: Nearmap (September 2024)

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Figure 5-80 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of WLO03

Table 5-71 pi	resents a	performance	summary	of this	intersection.
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Cope St (S)

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.038	8.5	1.0	LOS A
		East	0.043	5.8	1.4	LOS A
	Weekday AM	North	0.011	7.8	0.3	LOS A
	7 (17)	West	0.074	4.8	1.5	LOS A
		Total	0.038	8.5	1.0	LOS A
	Weekday PM	South	0.040	8.6	1.1	LOS A
Cope Street/ Wellington		East	0.052	5.5	1.6	LOS A
Street		North	0.011	7.9	0.3	LOS A
(Stop)		West	0.094	4.7	1.6	LOS A
(0.0p)		Total	0.040	8.6	1.1	LOS A
	Weekend	South	0.028	8.1	0.7	LOS A
		East	0.040	5.2	1.3	LOS A
		North	0.013	7.4	0.3	LOS A
		West	0.102	4.7	2.9	LOS A
		Total	0.028	8.1	0.7	LOS A

Overall, the intersection of Cope Street and Wellington Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.8.5 WLO05 - Wyndham Street/Henderson Road

The signalised intersection, composed of Wyndham Street and Henderson Road, is located west of Waterloo Station. It connects Henderson Road, linking Waterloo and Eveleigh, and Wyndham Street in Alexandria.

Figure 5-81 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-81 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of WLO05

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.633	53.1	74.1	LOS D
	Weekday	East	0.618	7.9	51.5	LOS A
	AM	West	0.742	60.1	94.1	LOS E
		Total	0.742	27.6	94.1	LOS B
Wyndham	Weekday PM	South	0.677	59.0	73.0	LOS E
Street/ Henderson		East	0.402	14.6	79.7	LOS B
Road		West	0.651	60.3	100.4	LOS E
(Signal)		Total	0.677	31.2	100.4	LOS C
	Weekend	South	0.584	53.4	62.9	LOS D
		East	0.331	13.1	67.7	LOS A
		West	0.667	61.4	94.2	LOS E
		Total	0.667	30.8	94.2	LOS C

Table 5-72 presents a performance summary of this intersection.

		South	0.633	53.1	74.1	L
	Weekday AM	East	0.618	7.9	51.5	L
		West	0.742	60.1	94.1	L
		Total	0.742	27.6	94.1	L
Wyndham Street/ Henderson Road (Signal)	Weekday PM	South	0.677	59.0	73.0	L
		East	0.402	14.6	79.7	L
		West	0.651	60.3	100.4	L
		Total	0.677	31.2	100.4	L
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Table 5-72 Block 4 – Intersection performance summary of WLO05

Overall, the intersection of Wyndham Street and Henderson Road performs satisfactorily at LOS C or better during all peak hours. The 95th percentile queue on Henderson Road (east approach) extends

back to Botany Road during the weekday PM and weekend peak hours. Similarly, the 95th percentile queue on Henderson Road (west approach) extends back to Garden Street during all peak hours.

It was also noted that Block 4 pedestrian volumes at Wyndham Street and Henderson Road significantly increased across all peak hours compared to pre-opening conditions. Given that WLO05 is located along the frontage of Waterloo station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.8.6 WLO06 – New Pedestrian Mid-block Crossing at Cope Street

The new unsignalised pedestrian mid-block crossing at Cope Street is located directly east of Waterloo Station. This crossing was under construction during Block 1 to 3, however was operational during Block 4.

Figure 5-82 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-82 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of WLO06

Table 5-73 presents a performance summary of this intersection.

Table 5-73 Block 4 – Intersection performance summary of WLO06

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South	0.030	2.2	0.8	LOS A
Now	Weekday AM	North	0.007	2.2	0.2	LOS A
Pedestrian		Total	0.030	2.2	0.8	LOS A
Mid-block Crossing at	Weekday PM	South	0.022	2.1	0.5	LOS A
(Pedestrian only – Zebra)		North	0.005	2.1	0.1	LOS A
		Total	0.022	2.1	0.5	LOS A
	Weekend	South	0.013	2.1	0.3	LOS A
		North	0.006	2.1	0.2	LOS A
		Total	0.013	2.1	0.3	LOS A

Overall, the pedestrian mid-block crossing at Cope Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.8.7 Comparison with previous study blocks

Figure 5-83 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are slightly lower during the AM and PM peak hours, and generally similar in the weekend peak hour compared to pre-opening conditions.



Figure 5-83 Study block comparison – Waterloo Station peak hourly traffic volumes across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-84. All intersections in the Waterloo Station study area perform at LOS C or better during Block 4, which is generally similar to previous study blocks.

Botany Road/Raglan Street (WLO01) had a notable change in LOS, whereby the intersection improved from a LOS D to a LOS C in the PM peak hour compared to Block 3. This change in LOS for WLO01 was due to lower traffic volumes at this intersection in Block 4 during the PM peak hour.

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Figure 5-84 Study block comparison – Waterloo Station intersection performance summary

5.9 Sydenham Station

Sydenham Station is an existing station and the ninth stop on the City & Southwest Line (towards Sydenham). It is located in the north-western area of Sydenham, bounded by Railway Parade, Gleeson Avenue, and Burrows Avenue in Sydenham.

Platforms 1 and 2 of the existing Sydenham Station have been upgraded and extended to facilitate metro functionality. In addition to the existing entrance at Gleeson Avenue, two new entrances have been constructed – one in the north and the other in the south. The northern entry opens onto a plaza near the corner of Railway Parade, and the southern entry provides access onto a plaza on Burrows Avenue near Hogan Avenue.

Bus services are provided within approximately 100 metres of Sydenham Station, located along Burrows Avenue and Railway Parade.

The Sydenham Station study area consists of six intersections. Table 5-74 presents the peak hours utilised for modelling the intersections. Table 5-75 provides a summary of the intersection LOS while Figure 5-85 visualises a geospatial summary of the intersection LOS within the Sydenham Station study area.

Network	Intersection	Weekday AM peak hour		Weekday PM peak hour		Weekend peak hour	
ID	ID	Day	Start time	Day	Start time	Day	Start time
	SYD01	Tuesday	8:00am	Wednesday	4:30pm	Saturday	10:45am
SYD-N1	SYD02	Tuesday					
-	SYD03	Monday	11:30am	Wednesday	5:00pm	Saturday	12:15pm
-	SYD04	Thursday	8:00am	Thursday	3:30pm	Saturday	10:45am
-	SYD05	Tuesday	8:00am	Friday	4:45pm	Saturday	1:30pm
-	SYD06	Tuesday	8:00am	Wednesday	4:30pm	Saturday	11:15am

Table 5-74 Block 4 – Sydenham Station peak hours modelled

Table 5-75 Block 4 – Sydenhan	n Station intersection	performance summary
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Intersection		LOS			
ID	Intersection	Weekday AM Peak	Weekday PM Peak	Weekend Peak	
SYD01	Railway Parade/Gleeson Avenue (Signal)	LOS A	LOS A	LOS A	
SYD02	Burrows Avenue/Gleeson Avenue (Signal)	LOS B	LOS B	LOS B	
SYD03	Burrows Avenue/George Street (Priority – Give Way)	LOS A	LOS A	LOS A	
SYD04	Railway Parade/Sydenham Road (Signal)	LOS A	LOS A	LOS A	
SYD05	Marrickville Road/Buckley Street (Priority – Give Way)	LOS A	LOS A	LOS A	
SYD06	Sydenham Road/Buckley Street (Priority – Give Way)	LOS A	LOS A	LOS A	

Overall, the intersection performance in the Sydenham Station study area during the peak hours is satisfactory, operating at LOS B or better.



Figure 5-85 Block 4 – Sydenham Station intersection performance summary

5.9.1 SYD01 – Railway Parade/Gleeson Avenue

The signalised intersection, composed of Railway Parade and Gleeson Avenue, is located directly west of Sydenham Station. It connects the State roads of Railway Parade and Gleeson Avenue in Sydenham.

Figure 5-86 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-86 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of SYD01

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.211	4.5	0.0	LOS A
Deikueu	Weekday AM	North-east	0.505	14.0	54.6	LOS A
		Total	0.505	11.1	54.6	LOS A
Parade/	Weekday PM	South-east	0.245	4.6	0.0	LOS A
Gleeson		North-east	0.442	12.9	47.0	LOS A
(Signal)		Total	0.442	9.9	47.0	LOS A
	Weekend	South-east	0.201	4.5	0.0	LOS A
		North-east	0.434	9.8	66.5	LOS A
		Total	0.434	8.2	66.5	LOS A

Table 5-76 presents a performance summary of this intersection.

Table 5-76 Block 4 – Intersection performance summary of SYD01

Overall, the intersection of Railway Parade and Gleeson Avenue performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 vehicle volumes at Railway Parade and Gleeson Avenue significantly decreased across all peak hours compared to pre-opening conditions. Exact reasons for the reduction in vehicle demand is unknown, however the reduction in traffic volumes at SYD01 is consistent with that suggested by SCATS detector data.
5.9.2 SYD02 – Burrows Avenue/Gleeson Avenue

The signalised intersection, composed of Burrows Avenue and Gleeson Avenue, is located directly south of Sydenham Station. It connects the local road of Burrows Avenue with the State road of Gleeson Avenue in Sydenham.

Figure 5-87 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.



Source: Nearmap (September 2024)

Figure 5-87 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of SYD02

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.341	15.3	74.4	LOS B
		North-east	0.371	53.1	32.8	LOS D
	Weekday AM	North-west	0.533	5.7	57.5	LOS A
		South-west	0.148	52.5	10.6	LOS D
		Total	0.533	15.9	74.4	LOS B
Burrows		South-east	0.430	17.5	94.7	LOS B
Avenue/		North-east	0.629	58.6	61.8	LOS E
Gleeson	Weekday PM	North-west	0.520	5.6	49.8	LOS A
/ Wondo	I IVI	South-west	0.100	48.5	8.2	LOS D
(Signal)		Total	0.629	20.1	94.7	LOS B
		South-east	0.347	12.8	79.2	LOS A
		North-east	0.464	56.6	39.4	LOS E
	Weekend	North-west	0.531	5.7	64.8	LOS A
		South-west	0.181	54.0	11.6	LOS D
		Total	0.531	14.6	79.2	LOS B

Table 5-77 presents a performance summary of this intersection.

Overall, the intersection of Burrows Avenue and Gleeson Avenue performs satisfactorily at LOS B during all peak hours. The 95th percentile queue on Gleeson Avenue (north-west approach) extends back to Railway Parade during the weekday AM and weekend peak hours.

It was also noted that Block 4 pedestrian volumes at Burrows Avenue and Gleeson Avenue significantly increased during the weekday PM and weekend peak hours compared to pre-opening conditions. Given that SYD02 is located along the frontage of Sydenham station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.9.3 SYD03 – Burrows Avenue/George Street

The priority intersection, composed of Burrows Avenue and George Street, is located directly east of Sydenham Station. It connects the local roads of Burrows Avenue and George Street in Sydenham.

Figure 5-88 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-88 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of SYD03

Table 5-78 presents a performance summary of this intersection.

Table 5-78 Block 4 – Intersection performance summary of SYD03

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.024	9.4	0.5	LOS A
	Weekday	North-east	0.176	4.1	6.4	LOS A
Burrows	AM	South-west	0.200	5.2	6.5	LOS A
Avenue/		Total	0.024	9.4	0.5	LOS A
Street		South-east	0.028	10.3	0.6	LOS A
(Priority –	Weekday	North-east	0.306	4.3	12.4	LOS A
Give Way)	PM	South-west	0.157	5.8	4.7	LOS A
		Total	0.028	10.3	0.6	LOS A
	Weekend	South-east	0.036	10.0	0.8	LOS A

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North-east	0.205	4.2	7.5	LOS A
		South-west	0.232	5.4	7.5	LOS A
		Total	0.036	10.0	0.8	LOS A

Overall, the intersection of Burrows Avenue and George Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

It was also noted that Block 4 pedestrian volumes at Burrows Avenue and George Street significantly increased during the weekend peak hour compared to pre-opening conditions. Given that SYD03 is located along the frontage of Sydenham station, the rise in pedestrian activity is likely associated to commuters accessing Sydney Metro during Block 4.

5.9.4 SYD04 – Railway Parade/Sydenham Road

The signalised intersection, composed of Railway Parade and Sydenham Road, is located directly north of Sydenham Station. It connects the State roads of Railway Parade and Sydenham Road in Sydenham.

Figure 5-89 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-89 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of SYD04

Table 5-79 presents a performance summary of this intersection.

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North-west	0.450	6.4	68.0	LOS A

 Table 5-79 Block 4 – Intersection performance summary of SYD04

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday	South-west	0.022	30.0	1.7	LOS C
	AM	Total	0.450	6.6	68.0	LOS A
Railway		North-west	0.420	5.6	65.2	LOS A
Parade/ Sydenham	Weekday PM	South-west	0.057	31.8	4.6	LOS C
Road	1 101	Total	0.420	6.0	65.2	LOS A
(Signal)		North-west	0.446	5.7	71.7	LOS A
	Weekend	South-west	0.093	36.0	7.9	LOS C
		Total	0.446	6.4	71.7	LOS A

Overall, the intersection of Railway Parade and Sydenham Road performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.9.5 SYD05 – Marrickville Road/Buckley Street

The priority intersection, composed of Marrickville Road and Buckley Street, is located west of Sydenham Station. It connects the State roads of Buckley Street in Sydenham and Marrickville Road, linking Sydenham and Dulwich Hill.

Figure 5-90 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-90 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of SYD05

Table 5-80 presents a performance summary of this intersection.

Table 5-80 Block 4 – Intersection performance summary of SYD05

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		South-east	0.739	8.7	35.6	LOS A

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday	North-west	0.887	12.4	65.6	LOS A
	AM	Total	0.887	12.4	65.6	LOS A
Marrickville Road/		South-east	0.706	7.7	33.4	LOS A
Buckley	Weekday PM	North-west	0.692	7.8	22.1	LOS A
Street	I IVI	Total	0.692	7.8	22.1	LOS A
(Priority – Give Way)		South-east	0.341	6.2	12.9	LOS A
	Weekend	North-west	0.298	6.1	10.0	LOS A
		Total	0.341	6.2	12.9	LOS A

Overall, the intersection of Marrickville Road and Buckley Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.9.6 SYD06 – Sydenham Road/Buckley Street

The priority intersection, composed of Sydenham Road and Buckley Street, is located north of Sydenham Station. It connects the State roads of Buckley Street in Sydenham and Sydenham Road, linking Sydenham and Marrickville.

Figure 5-91 illustrates both the general intersection layout as modelled in SIDRA Intersection and the layout as per aerial imagery.





Source: Nearmap (September 2024)

Figure 5-91 Block 4 – AM peak model SIDRA Intersection layout (left) and Nearmap aerial imagery (right) of SYD06

Table 5-81 presents a performance summary of this intersection.

Table 5-81 Block 4 – Intersection performance summary of SYD06

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
		North-west	0.418	0.1	0.0	LOS A

Intersection	Peak	Approach	Degree of saturation	Average delay (seconds)	95 th percentile queue (metres)	LOS
	Weekday	South-west	0.261	5.8	0.0	LOS A
<u> </u>	AM	Total	0.261	5.8	0.0	LOS A
Sydenham Road/		North-west	0.396	0.1	0.0	LOS A
Buckley	Weekday PM	South-west	0.196	5.8	0.0	LOS A
Street	I IVI	Total	0.196	5.8	0.0	LOS A
(Priority – Give Way)		North-west	0.388	0.1	0.0	LOS A
	Weekend	South-west	0.245	5.8	0.0	LOS A
		Total	0.245	5.8	0.0	LOS A

Overall, the intersection of Sydenham Road and Buckley Street performs satisfactorily at LOS A during all peak hours. The 95th percentile queue lengths are accommodated within the approach distances for all approaches.

5.9.7 Comparison with previous study blocks

Figure 5-92 provides a comparison of the total peak hourly traffic volumes recorded across all intersections for Block 4 against previous study blocks. As shown, Block 4 traffic volumes are generally similar to pre-opening conditions during all peak hours.



Figure 5-92 Study block comparison – Sydenham Station peak hourly traffic volumes across all intersections

A comparison of the intersection LOS for Block 4 against previous study blocks is shown in Figure 5-93. All intersections in the Sydenham site perform at a LOS B or better during Block 4, which is generally similar to previous study blocks.

54	SYD01 - F	AILWAY PDE / GLEESO	NAVE	SYD02 - B	URROWS AVE / GLEESO	ON AVE
Caloffan estrel	AM PEAK	PM PEAK	WE PEAK	AM PEAK	PM PEAK	WE PEAK
and the state of the set of the set	LOSA LOSA LOSA LOSA	A SOL LOSA LOSA LOSA	LOSA LOSA LOSA	a sol	B SOL	A SOL LOS B LOS B LOS B
SHAT SYDOE SHILOW STU	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4					
	SYD03 - E	BURROWS AVE / GEORG	GE ST	SYD04 - R	RAILWAY PDE / SYDENHA	AM RD
	AM PEAK	PM PEAK	WE PEAK	AM PEAK	PM PEAK	WE PEAK
SYD04	8 8 8 8 8 8 8 8 8 8 8 8 8	4 4 4 4 9 9 9 9 9	8 8 8 8 8 8 8 8 8 8 8	Z Z Z Z	4 4 4 4 4 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
	10° 10° 10°	L08 L08	L01 L01	0 0 0 0	LOS 101	Lo. Lo.
software the second	BLOCK 1 BLOCK 2 BLOCK 3 BLOCK 4					
8	SYD05 - MA	RRICKVILLE RD / BUCK	LEY ST	SYD06 - 5	SYDENHAM RD / BUCKL	EY ST
Astrony parts M	AM PEAK	PM PEAK	WE PEAK	AM PEAK	PM PEAK	WE PEAK
SYDD1						
SYD01 BYD03 V Reality	LOS A LOS A LOS A	LOSA LOSA LOSA	LOSA LOSA LOSA	LOSA LOSA LOSA LOSA	LOSA LOSA LOSA	V SOJ
SYD01 BYD03 V SYD05 Automatic Automa	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A	BLOCK 1 LOS A BLOCK 2 LOS A BLOCK 3 LOS A BLOCK 4 LOS A

Figure 5-93 Study block comparison – Sydenham Station intersection performance summary

6.0 Transport interchange monitoring

This section details analysis of the interchange traffic survey data at kerbside facilities nearby station interchanges.

6.1 Chatswood Station

In the Chatswood Station study area, a total of five taxi and kiss and ride facilities were assessed during Block 4. These included three kiss and ride facilities and two taxi facilities. Refer to Section 3.3 for detailed information about their locations and the number of bays.

6.1.1 Kiss and ride

Table 6-1 presents a summary of the peak hour vehicle demands for the kiss and ride facilities, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- CWDK1 Railway Street:
 - The highest demand recorded at CWDK1 occurred during the weekend peak hour, when there were 26 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to two minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- CWDK2 69 Albert Avenue:
 - The highest demand recorded at CWDK2 occurred during the weekday AM peak hour, when there were 75 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to three minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- CWDK3 Endeavour Street:
 - The highest demand recorded at CWDK3 occurred during the weekday AM peak hour, when there were 35 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to three minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-1 Block 4 – Chatswood Station interchange assessment peak hour summary (kiss and ride)

П	Peak hour								
19	Summary	Weekday AM	Weekday PM	Weekend					
	Peak hour	Thursday 7am-8am	Friday 6pm-7pm	Saturday 6pm-7pm					
CWDK1	Vehicles (vehicle per hour)	23	22	26					
Street)	Average dwell time (minutes)	1	1	2					
	Boarding/alighting passenger (excluding driver)	31	24	35					

חו	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Monday 7am-8am	Wednesday 4pm-5pm	Sunday 5pm-6pm	
CWDK2	Vehicles (vehicle per hour)	75	45	31	
(Albert Avenue)	Average dwell time (minutes)	1	2	3	
	Boarding/alighting passenger (excluding driver)	114	29	45	
CWDK3 (Endeavour Street)	Peak hour	Wednesday 7am-8am	Friday 6pm-7pm	Saturday 4pm-5pm	
	Vehicles (vehicle per hour)	35	31	27	
	Average dwell time (minutes)	1	1	3	
	Boarding/alighting passenger (excluding driver)	45	31	23	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-1 to Figure 6-3 present the daily demand profile for the three kiss and ride facilities at Chatswood Station.



Figure 6-1 Block 4 – Daily demand profile of CWDK1



Figure 6-2 Block 4 – Daily demand profile of CWDK2



Figure 6-3 Block 4 – Daily demand profile of CWDK3

6.1.2 Taxi

Table 6-2 presents a summary of the peak hour vehicle demands for the taxi facilities, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- CWDT1 Victoria Avenue:
 - The highest demand recorded at CWDT1 occurred during the weekend peak hour, when there were 29 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from six to 13 minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- CWDT2 Endeavour Street:
 - The highest demand recorded at CWDT2 occurred during the weekend peak hour, where there were 17 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to three minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-2 Block 4 – Chatswood Station interchange assessment peak hour summary (taxi)

ID	Peak hour				
10	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Tuesday 9am-10am	Friday 11pm-12am	Saturday 6pm-7pm	
CWDT1 (Victoria Avenue)	Vehicles (vehicle per hour)	24	26	29	
	Average dwell time (minutes)	13	6	9	
	Boarding/alighting passenger (excluding driver)	19	34	31	
CWDT2 (Endeavour Street)	Peak hour	Tuesday 9am-10am	Tuesday 4pm-5pm	Saturday 7pm-8pm	
	Vehicles (vehicle per hour)	15	15	17	
	Average dwell time (minutes)	1	2	3	

Boarding/alighting passenger (excluding driver)	16	17	24	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-4 and Figure 6-5 present the daily demand profile for the two taxi facilities at Chatswood Station.



Figure 6-4 Block 4 – Daily hourly demand profile of CWDT1



Figure 6-5 Block 4 – Daily demand profile of CWDT2

6.1.3 Comparison with previous study blocks

Figure 6-6 provides a comparison of the total peak hourly vehicle demand recorded across the interchange facilities for Block 4 against pre-opening conditions. Key findings were as follows:

- CWDK1 Railway Street vehicle demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDK2 69 Albert Avenue vehicle demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDK3 Endeavour Street vehicle demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDT1 Victoria Avenue vehicle demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDT2 Endeavour Street vehicle demands are generally similar during all peak hours compared to pre-opening conditions in Block 2 and 3, however Block 1 demands were much lower.

Similarly, Figure 6-7 provides a comparison of the total peak hourly boarding and alighting demand recorded across the interchange facilities for Block 4 against pre-opening conditions. Key findings were as follows:

- CWDK1 Railway Street boarding and alighting demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDK2 69 Albert Avenue boarding and alighting demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDK3 Endeavour Street boarding and alighting demands are generally similar during the weekday AM and PM peak hours, and lower during the weekend peak hour compared to preopening conditions.
- CWDT1 Victoria Avenue boarding and alighting demands are generally similar during all peak hours compared to pre-opening conditions.
- CWDT2 Endeavour Street boarding and alighting demands are generally similar during all peak hours compared to pre-opening conditions in Blocks 2 and 3, however Block 1 demands were much lower.



Figure 6-6 Study block comparison – Chatswood Station interchange vehicle demand summary

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Figure 6-7 Study block comparison – Chatswood Station interchange boarding and alighting demand summary

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6.2 Crows Nest Station

In the Crows Nest Station study area, a total of four taxi and kiss and ride facilities were assessed during Block 4. These included one taxi facility and three kiss and ride facilities. Refer to Section 3.3 for detailed information about their locations and the number of bays.

6.2.1 Kiss and ride

Table 6-3 presents a summary of the peak hour vehicle demands for the kiss and ride facilities, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- CSTK1 Oxley Street (east):
 - The highest demand recorded at CSTK1 occurred during the weekend peak hour, when there were eight vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- CSTK2 Oxley Street (west):
 - The highest demand recorded at CSTK2 occurred during the weekday AM peak hour, when there were 13 vehicles per hour
 - The average dwell time during the weekday and weekend peak hours ranged from one to eight minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- CSTK3 Clarke Street:
 - The highest demand recorded at CSTK3 occurred during the weekday AM and weekend peak hours, when there were five vehicles per hour
 - The average dwell time during the weekday and weekend peak hours ranged from one to two minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane.
 Current number of bays are generally sufficient to cater the peak demand and queues.
 However, during Block 4, kiss and ride demand was observed to also occur in the existing P10 minutes parking zone on the opposite side of the road from CSTK3 Clarke Street.

Table 6-3 Block 4 – Crows Nest Station interchange assessment peak hour summary (kiss and ride)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Thursday 7am-8am	Tuesday 4pm-5pm	Saturday 5pm-6pm	
CSTK1 (Oxley Street – East)	Vehicles (vehicle per hour)	7	5	8	
	Average dwell time (minutes)	1	1	1	
	Boarding/alighting passenger (excluding driver)	6	6	11	

ID	Peak hour					
	Summary	Weekday AM	Weekday PM	Weekend		
	Peak hour	Thursday 8am-9am	Thursday 5pm-6pm	Sunday 2pm-3pm		
CSTK2 (Oxley	Vehicles (vehicle per hour)	13	10	9		
Street – West)	Average dwell time (minutes)	1	8	5		
	Boarding/alighting passenger (excluding driver)	16	16	17		
CSTK3 (Clarke Street)	Peak hour	Wednesday 7am-8am	Friday 5pm-6pm	Saturday 10am-11am		
	Vehicles (vehicle per hour)	5	4	5		
	Average dwell time (minutes)	2	1	1		
	Boarding/alighting passenger (excluding driver)	4	4	4		

Note: Average dwell times were rounded to the nearest minute.

Figure 6-8 to Figure 6-10 presents the daily demand profile for the three kiss and ride facilities at Crows Nest Station.



Figure 6-8 Block 4 – Daily demand profile of CSTK1



Figure 6-9 Block 4 – Daily demand profile of CSTK2



Figure 6-10 Block 4 – Daily demand profile of CSTK3

6.2.2 Taxi

Table 6-4 presents a summary of the peak hour vehicle demands for the taxi facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- CSTT1 Clarke Street:
 - The highest demand recorded at CSTT1 occurred during the weekday PM peak hour, when there were 11 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to four minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues. Note that most vehicles accessing this bay were general vehicles.

Table 6-4 Block 4 – Crows Nest Station interchange assessment peak hour summary (taxi)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
CSTT1 (Clarke Street)	Peak hour	Tuesday 10am-11am	Wednesday 7pm-8pm	Saturday 7pm-8pm	
	Vehicles (vehicle per hour)	6	11	7	
	Average dwell time (minutes)	3	4	1	
	Boarding/alighting passenger (excluding driver)	5	8	9	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-11 presents the daily demand profile for the taxi facility at Crows Nest Station.





6.2.3 Interchange demand summary

Figure 6-12 and Figure 6-13 provide a summary of the total peak hourly vehicle demand and boarding and alighting demand, respectively, recorded across the interchange facilities for Block 4.



Figure 6-12 Block 4 – Crows Nest Station interchange vehicle demand summary



Figure 6-13 Block 4 – Crows Nest Station interchange boarding and alighting demand summary

6.3 Victoria Cross Station

In the Victoria Cross Station study area, a total of five taxi, kiss and ride, bus, and accessible parking facilities were assessed during Block 4. These included one taxi facility, one kiss and ride facility, two bus facilities, and one accessible parking facility. Refer to Section 3.3 for detailed information about their locations and the number of bays.

6.3.1 Bus

Table 6-5 presents a summary of the peak hour demands for the bus facilities, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- VICB1 Miller Street (east):
 - The highest demand recorded at VICB1 occurred during the weekday AM peak hour, when there were 30 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to two minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- VICB2 Miller Street (west):
 - The highest demand recorded at VICB2 occurred during the weekday PM peak hour, where there were 28 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-5 Block 4 – Victoria Cross Station interchange assessment peak hour summary (bus)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Tuesday 8am-9am	Monday 5pm-6pm	Saturday 4pm-5pm	
VICB1 (Miller	Vehicles (vehicle per hour)	30	14	12	
Street - East)	Average dwell time (minutes)	1	2	1	
	Boarding/alighting passenger (excluding driver)	105	31	20	
VICB2 (Miller Street - West)	Peak hour	Tuesday 8am-9am	Monday 3pm-4pm	Saturday 1pm-2pm	
	Vehicles (vehicle per hour)	11	28	8	
	Average dwell time (minutes)	1	1	1	
	Boarding/alighting passenger (excluding driver)	20	26	4	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-14 and Figure 6-15 present the daily demand profile for the two bus facilities at Victoria Cross Station.



Figure 6-14 Block 4 – Daily demand profile of VICB1



Figure 6-15 Block 4 – Daily demand profile of VICB2

6.3.2 Kiss and ride

Table 6-6 presents a summary of the peak hour vehicle demands for the kiss and ride facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- VICK1 McLaren Street:
 - The highest demand recorded at VICK1 occurred during the weekday AM peak hour, where there were 25 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to four minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-6 Block 4 – Victoria Cross Station interchange assessment peak hour summary (kiss and ride)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
VICK1 (McLaren Street)	Peak hour	Tuesday 8am-9am	Thursday 5pm-6pm	Saturday 8am-9am	
	Vehicles (vehicle per hour)	25	14	8	
	Average dwell time (minutes)	1	3	4	

ID	Peak hour			
	Summary	Weekday AM	Weekday PM	Weekend
	Boarding/alighting passenger (excluding driver)	21	13	8

Note: Average dwell times were rounded to the nearest minute.

Figure 6-16 presents the daily demand profile for the kiss and ride facility at Victoria Cross Station.



Figure 6-16 Block 4 – Daily demand profile of VICK1

6.3.3 Taxi

Table 6-7 presents a summary of the peak hour demands for the taxi facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- VICT1 McLaren Street:
 - The highest demand recorded at VICT1 occurred during the weekday AM peak hour, when there were eight vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to eight minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane.
 Current number of bays are generally sufficient to cater the peak demand and queues. Note that most vehicles accessing this bay were general vehicles.

Table 6-7 Block 4 – Victoria Cross Station interchange assessment peak hour summary (ta	xi)
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ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
VICT1 (McLaren Street)	Peak hour	Friday 8am-9am	Wednesday 6pm-7pm	Sunday 12pm-1pm	
	Vehicles (vehicle per hour)	8	4	3	
	Average dwell time (minutes)	8	2	1	
	Boarding/alighting passenger (excluding driver)	11	5	0	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-17 presents the daily demand profile for the taxi facility at Victoria Cross Station.



Figure 6-17 Block 4 – Daily demand profile of VICT1

6.3.4 Accessible parking

Table 6-8 presents a summary of the peak hour demands for the accessible parking facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- VICA1 McLaren Street:
 - The highest demand recorded at VICA1 occurred during the weekday AM peak hour, when there were 11 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to 78 minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-8 Block 4 – Victoria Cross Station interchange assessment peak hour summary (accessible parking)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Wednesday 8am-9am	Wednesday 3pm-4pm	Saturday 10am-11am	
VICA1 (McLaren Street)	Vehicles (vehicle per hour)	11	3	2	
	Average dwell time (minutes)	1	1	78	
	Boarding/alighting passenger (excluding driver)	9	5	8	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-18 presents the daily demand profile for the accessible parking at Victoria Cross Station.



Figure 6-18 Block 4 – Daily demand profile of VICA1

6.3.5 Interchange demand summary

Figure 6-19 and Figure 6-20 provide a summary of the total peak hourly vehicle demand and boarding and alighting demand, respectively, recorded across the interchange facilities for Block 4.



Figure 6-19 Block 4 – Victoria Cross Station interchange vehicle demand summary



Figure 6-20 Block 4 – Victoria Cross Station interchange boarding and alighting demand summary

6.4 Barangaroo Station

In the Barangaroo Station study area, a total of four taxi, kiss and ride, and bus facilities were assessed during Block 4. These included one taxi facility, one kiss and ride facility and two bus facilities. Refer to Section 3.3 for detailed information about their locations and the number of bays.

6.4.1 Bus

Table 6-9 presents a summary of the peak hour demands for the bus facilities, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- BGUB1 Hickson Road (east):
 - The highest demand recorded at BGUB1 occurred during the weekday PM peak hour, when there were nine vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- BGUB2 Hickson Road (west):
 - The highest demand recorded at BGUB2 occurred during the weekday PM peak hour, when there were nine vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

 Table 6-9 Block 4 – Barangaroo Station interchange assessment peak hour summary (bus)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Tuesday 8am-9am	Wednesday 5pm-6pm	Sunday 4pm-5pm	
BGUB1 (Hickson	Vehicles (vehicle per hour)	6	9	6	
Road - East)	Average dwell time (minutes)	1	1	1	
	Boarding/alighting passenger (excluding driver)	8	32	25	
BGUB2 (Hickson Road - West)	Peak hour	Thursday 7am-8am	Wednesday 6pm-7pm	Saturday 11am-12pm	
	Vehicles (vehicle per hour)	5	9	7	
	Average dwell time (minutes)	1	1	1	
	Boarding/alighting passenger (excluding driver)	5	12	8	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-21 and Figure 6-22 present the daily demand profiles for the two bus facilities at Barangaroo Station.



Figure 6-21 Block 4 – Daily demand profile of BGUB1





6.4.2 Kiss and ride

Table 6-10 presents a summary of the peak hour demands for the kiss and ride facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- BGUK1 Hickson Road:
 - The highest demand recorded at BGUK1 occurred during the weekday AM and PM peak hours, when there were three vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-10 Block 4 – Barangaroo Station interchange assessment peak hour summary (kiss and ride)

п	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
BGUK1 (Hickson Road)	Peak hour	Monday 7am-8am	Friday 4pm-5pm	Sunday 11am-12pm	
	Vehicles (vehicle per hour)	3	3	2	
	Average dwell time (minutes)	1	1	1	

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Boarding/alighting passenger (excluding driver)	5	7	1	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-23 provides the daily demand profile for the kiss and ride facility at Barangaroo Station.



Figure 6-23 Block 4 – Daily demand profile of BGUK1

6.4.3 Taxi

Table 6-11 presents a summary of the peak hour demands for the taxi facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- BGUT1 Hickson Road:
 - The highest demand recorded at BGUT1 was one vehicle per hour which occurred during all peak hours.
 - The average dwell time during the weekday and weekend peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-11 Block 4 – Barangaroo St	ation interchange assessment	peak hour summary (taxi)
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ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
BGUT1 (Hickson Road)	Peak hour	Monday 9am-10am	Monday 12pm-1pm	Saturday 12pm-1pm	
	Vehicles (vehicle per hour)	1	1	1	
	Average dwell time (minutes)	1	1	1	
	Boarding/alighting passenger (excluding driver)	0	2	0	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-24 presents the daily demand profile for the taxi facility at Barangaroo Station.



Figure 6-24 Block 4 – Daily demand profile of BGUT1

6.4.4 Interchange demand summary

Figure 6-25 and Figure 6-26 provide a summary of the total peak hourly vehicle demand and boarding and alighting demand, respectively, recorded across the interchange facilities for Block 4.



Figure 6-25 Block 4 – Barangaroo Station interchange vehicle demand summary



Figure 6-26 Block 4 – Barangaroo Station interchange boarding and alighting demand summary

6.5 Waterloo Station

In the Waterloo Station study area, a total of four taxi, kiss and ride, bus, and accessible parking facilities were assessed during Block 4. These included one taxi facility, one kiss and ride facility, one bus facility, and one accessible parking facility. Refer to Section 3.3 for detailed information about their locations and the number of bays.

6.5.1 Bus

Table 6-12 presents a summary of the peak hour demands for the bus facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- WLOB1 Raglan Street:
 - The highest demand recorded at WLOB1 occurred during the weekday PM peak hour, when there were 11 vehicles per hour.
 - The average dwell time during all the peak hours was up to one minute.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-12 Block 4 – Waterloo Station interchange assessment peak hour summary (bus)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
WLOB1 (Raglan Street)	Peak hour	Monday 8am-9am	Monday 3pm-4pm	Saturday 8am-9am	
	Vehicles (vehicle per hour)	10	11	9	
	Average dwell time (minutes)	1	1	1	
	Boarding/alighting passenger (excluding driver)	87	31	25	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-27 presents the daily demand profile for the bus facility at Waterloo Station.



Figure 6-27 Block 4 – Daily demand profile of WLOB1

6.5.2 Kiss and ride

Table 6-13 presents a summary of the peak hour demands for the kiss and ride facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- WLOK1 Cope Street:
 - The highest demand recorded at WLOK1 occurred during the weekday PM peak hour, when there were 19 vehicles per hour
 - The average dwell time during the weekday and weekend peak hours ranged from one to five minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-13 Block 4 – Waterloo Station interchange assessment peak hour summary (kiss and ride)

ID	Peak hour			
	Summary	Weekday AM	Weekday PM	Weekend
WLOK1 (Cope Street)	Peak hour	Thursday 8am-9am	Friday 5pm-6pm	Sunday 5pm-6pm
	Vehicles (vehicle per hour)	16	19	11
	Average dwell time (minutes)	1	2	5
	Boarding/alighting passenger (excluding driver)	16	18	10

Note: Average dwell times were rounded to the nearest minute.

Figure 6-28 presents the daily demand profile for the kiss and ride facility at Waterloo Station.



Figure 6-28 Block 4 – Daily demand profile of WLOK1

6.5.3 Taxi

Table 6-14 presents a summary of the peak hour demands for the taxi facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- WLOT1 Cope Street:
 - The highest demand recorded at WLOT1 occurred during the weekday AM peak hour, when there were five vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to three minutes.

- Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-14 Block 4 – Waterloo Station interchange assessment peak hour summary (taxi)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
WLOT1 (Cope Street)	Peak hour	Peak hour Wednesday Monda 8am-9am 3pm-4p		Saturday 11pm-12am	
	Vehicles (vehicle per hour)	5	4	4	
	Average dwell time (minutes)	2	1	3	
	Boarding/alighting passenger (excluding driver)	6	5	7	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-29 presents the daily demand profile for the taxi facility at Waterloo Station.



Figure 6-29 Block 4 – Daily demand profile of WLOT1

6.5.4 Accessible parking

Table 6-15 presents a summary of the peak hour demands for the accessible parking facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- WLOA1 Cope Street:
 - The highest demand recorded at WLOA1 occurred during the weekday PM peak hour, when there were three vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to four minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-15 Block 4 – Waterloo Station interchange assessment peak hour summary (accessible parking)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
WLOA1 (Cope Street)	Peak hour	Monday 6am-7am	Thursday 6pm-7pm	Saturday 10am-11am	
	Vehicles (vehicle per hour)	1	3	1	
חו	Peak hour				
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	Summary	Weekday AM	Weekday PM	Weekend	
	Average dwell time (minutes)	1	4	1	
	Boarding/alighting passenger (excluding driver)	2	5	0	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-30 presents the daily demand profile for the accessible parking at Waterloo Station.



Figure 6-30 Block 4 – Daily demand profile of WLOA1

6.5.5 Interchange demand summary

Figure 6-31 and Figure 6-32 provide a summary of the total peak hourly vehicle demand and boarding and alighting demand, respectively, recorded across the interchange facilities for Block 4. It is noted that some drivers occasionally parked in the interchange facilities without picking up or dropping off passengers, resulting in vehicle demand sometimes being higher than the boarding and alighting demand.

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Figure 6-31 Block 4 – Waterloo Station interchange vehicle demand summary

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Figure 6-32 Block 4 – Waterloo Station interchange boarding and alighting demand summary

6.6 Sydenham Station

In the Sydenham Station study area, a total of five taxi, bus stop, kiss and ride and accessible parking facilities were assessed during Block 4. These included one bus facility, two kiss and ride facilities, one taxi facility and one accessible parking area. Refer to Section 3.3 for detailed information about their locations and the number of bays.

6.6.1 Bus

Table 6-16 presents a summary of the peak hour demands for the bus facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- SYDB1 Railway Parade:
 - The highest demand recorded at SYDB1 occurred during the weekday AM peak hour, when there were 16 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from two to three minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-16 Block 4	I – Sydenham Station interchange assessment peak hour summary (bus)

ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Tuesday 7am-8am	Tuesday 5pm-6pm	Saturday 1pm-2pm	
SYDB1 (Pailway	Vehicles (vehicle per hour)	16	15	8	
Parade)	Average dwell time (minutes)	2	3	2	
	Boarding/alighting passenger (excluding driver)	137	64	34	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-33 presents the daily demand profile for the bus facility at Sydenham Station.



Figure 6-33 Block 4 – Daily demand profile of SYDB1

6.6.2 Kiss and ride

Table 6-17 presents a summary of the peak hour demands for the kiss and ride facilities, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- SYDK1 Burrows Avenue:
 - The highest demand recorded at SYDK1 occurred during the weekday PM peak hour, when there were 31 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from two to three minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.
- SYDK2 Sydenham Road:
 - The highest demand recorded at SYDK2 occurred during the weekday AM peak hour, when there were 45 vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to two minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-17 Block 4 – Sydenham S	station interchange assessment peak	hour summary (kiss and ride)
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ID	Peak hour				
	Summary	Weekday AM	Weekday PM	Weekend	
	Peak hour	Thursday 7am-8am	Thursday 6pm-7pm	Saturday 10am-11am	
SYDK1	Vehicles (vehicle per hour)	22	31	17	
(Burrows Avenue)	Average dwell time (minutes)	2	2	3	
	Boarding/alighting passenger (excluding driver)	29	33	18	
	Peak hour	Tuesday 8am-9am	Thursday 5pm-6pm	Saturday 5pm-6pm	
SYDK2	Vehicles (vehicle per hour)	45	22	20	
(Sydenham Road)	Average dwell time (minutes)	1	2	1	
	Boarding/alighting passenger (excluding driver)	46	20	24	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-34 and Figure 6-35 provide daily demand profile for the kiss and ride facilities at Sydenham Station.







Figure 6-35 Block 4 – Daily demand profile of SYDK2

6.6.3 Taxi

Table 6-18 presents a summary of the peak hour demands for the taxi facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- SYDT1 Burrows Avenue:
 - The highest demand recorded at SYDT1 occurred during the weekday PM peak hour, when there were six vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from one to two minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues. Note that most vehicles accessing the bay were general vehicles.

Table 6-18 Block 4 – Sydenham Station interchange assessment peak hour summary (taxi)

ID	Peak hour				
10	Summary	Weekday AM	Weekday PM	Weekend	
SVDT1	Peak hour	Friday 6am-7am	Thursday 9pm-10pm	Sunday 1pm-2pm	
(Burrows	Vehicles (vehicle per hour)	4	6	5	
Avenue)	Average dwell time (minutes)	1	1	2	

ID	Peak hour				
10	Summary	Weekday AM	Weekday PM	Weekend	
	Boarding/alighting passenger (excluding driver)	1	5	12	

Note: Average dwell times were rounded to the nearest minute.

Figure 6-36 presents the daily demand profile for the taxi facility at Sydenham Station.



Figure 6-36 Block 4 – Daily demand profile of SYDT1

6.6.4 Accessible parking

Table 6-19 presents a summary of the peak hour demands for the accessible parking facility, as well as the average dwell time and total number of boarding/alighting passengers during the identified peak hours.

Based on the interchange survey data, the following were the key observations:

- SYDA1 Bolton Street:
 - The highest demand recorded at SYDA1 occurred during the weekday AM peak hour, when there were three vehicles per hour.
 - The average dwell time during the weekday and weekend peak hours ranged from four to 51 minutes.
 - Generally, no queues were observed to extend out of the bay and block the travel lane. Current number of bays are generally sufficient to cater the peak demand and queues.

Table 6-19 Block 4 – Sydenham Station interchange assessment peak hour summary (accessible parking)

חו	Peak hour					
	Summary	Weekday AM	Weekday PM	Weekend		
	Peak hour	Tuesday 10am-11am	Thursday 12pm-1pm	Sunday 8am-9am		
SYDA1	Vehicles (vehicle per hour)	3	2	2		
(Bolton Street)	Average dwell time (minutes)	4	29	51		
	Boarding/alighting passenger (excluding driver)	7	4	8		

Note: Average dwell times were rounded to the nearest minute.

Figure 6-37 presents the daily demand profile for the accessible parking at Sydenham Station.





6.6.5 Comparison with previous study blocks

Figure 6-38 provides a comparison of the peak hourly vehicle demand recorded across the interchange facilities for Block 4 against pre-opening conditions. Key findings are as follows:

- SYDB1 Railway Parade vehicle demands are generally similar during all peak hours compared to pre-opening conditions.
- SYDK1 Burrows Avenue vehicle demands are generally similar in the weekday AM and weekend peak hours, and slightly higher in the weekday PM peak hour compared to pre-opening conditions.
- SYDK2 Sydenham Road vehicle demands are substantially higher during all peak hours compared to pre-opening conditions. This is likely due to the opening of the new Sydney Metro station access on Sydenham Road near this facility.
- SYDT1 Burrows Avenue vehicle demands are generally similar during all peak hours compared to pre-opening conditions in Block 2, however slightly higher than Block 1 and 3.
- SYDA1 Bolton Street vehicle demands are generally similar during all peak hours compared to preopening conditions.

Similarly, Figure 6-39 provides a comparison of the total peak hourly boarding and alighting demand recorded across the interchange facilities for Block 4 against pre-opening conditions. Key findings were as follows:

- SYDB1 Railway Parade boarding and alighting demands are:
 - slightly higher in the weekday AM peak hour compared to pre-opening conditions in Blocks 1, 2 and 3
 - similar in the weekday PM peak hour compared to pre-opening conditions in Block 1 and 3, however lower than Block 2 demands.
 - similar during the weekend peak hour compared to pre-opening conditions in Blocks 1, 2 and
 3.
- SYDK1 Burrows Avenue boarding and alighting demands are generally similar during all peak hours compared to pre-opening conditions.
- SYDK2 Sydenham Road boarding and alighting demands are substantially higher during all peak hours compared to pre-opening conditions. This is likely due to the opening of the new Sydney Metro station access on Sydenham Road near this facility.
- SYDT1 Burrows Avenue boarding and alighting demands are:
 - similar during the weekday AM peak hour compared to pre-opening conditions in Blocks 1 and 3, however lower than Block 2 demands
 - similar during the weekday PM peak hour compared to pre-opening conditions in Blocks 1, 2 and 3

- similar during the weekend peak hour compared to pre-opening conditions in Block 2, however higher than Block 1 and 3 demands.
- SYDA1 Bolton Street boarding and alighting demands are generally similar during all peak hours compared to pre-opening conditions.

It is noted that some drivers occasionally parked in the interchange facilities without picking up or dropping off passengers, resulting in vehicle demand sometimes being higher than the boarding and alighting demand.

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Figure 6-38 Block 4 – Study block comparison – Sydenham Station interchange vehicle demand summary

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Figure 6-39 Block 4 – Study block comparison – Sydenham Station interchange boarding and alighting demand summary

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7.0 Summary

AECOM has been commissioned by Sydney Metro to undertake traffic and interchange monitoring for the Sydney Metro City & Southwest, covering the stretch between Chatswood Station and Sydenham Station (the Project).

The primary objective of the traffic and interchange monitoring assessment is to evaluate the potential impacts of metro operations at the nine stations along the Sydney Metro City & Southwest (Chatswood to Sydenham) on the surrounding intersections and interchange facilities.

To meet the CoA requirements and align with the project program for Sydney Metro City & Southwest (Chatswood to Sydenham), the traffic and interchange monitoring program will be conducted in six study blocks. The monitoring period will span 12 months before the commencement of CSSI operations (pre-opening) and another 12 months after the commencement (post-opening).

This report relates to Block 4 which represents the first study block following the commencement of operations of the Sydney Metro City & Southwest Line (Chatswood to Sydenham).

The overall scope of works for the Block 4 study covers the following:

- **Traffic monitoring:** Intersection surveys were conducted for a one-week period in September 2024. The surveys included classified intersection count survey and vehicular queue length survey.
- **Transport interchange monitoring:** Surveys were conducted at all stations with operating interchange facilities, namely Chatswood Station, Crows Nest Station, Victoria Cross Station, Barangaroo Station, Waterloo Station and Sydenham Station. Interchange operation surveys were conducted at these six stations continuously for a one-week period in September 2024.
- Site visit and observations: Site visits were undertaken in conjunction with the traffic and interchange operation monitoring for at least one weekday AM peak period, one weekday PM peak period, and one weekend peak period at each station.
- Intersection assessment: To evaluate the intersection operation during Block 4, isolated and network traffic modelling assessments were performed using SIDRA Intersection modelling software.
- **Traffic and interchange monitoring report:** The key findings of the Block 4 study were presented to Sydney Metro and key stakeholders in January 2025. This report provides a summary of the details regarding the Block 4 traffic and interchange operation assessment.

Key findings of the Block 4 study are:

- Intersection monitoring: Based on site observation and SIDRA Intersection modelling results, intersection operation and performance of key intersections at each station are summarised as follows.
 - Chatswood Dive Site:
 - The intersection of Mowbray Road and Hampden Road (CWD01) performs at LOS B during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions.
 - Crows Nest Station:
 - All intersections within the Crows Nest Station study area perform at LOS C or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions. Pacific Highway/Falcon Street/Shirley Road (CST04) had a notable change in LOS, whereby the intersection improved from a LOS D to a C in the weekday AM peak hour compared to Block 3. This change in LOS for CST04 was due to lower traffic volumes at this intersection in Block 4.

- Victoria Cross Station:
 - All intersections within the Victoria Cross Station study area operate at LOS D or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions. Miller Street/Berry Street (VIC02) had a notable change in LOS, whereby the intersection reduced from a LOS C to a D in the weekday AM peak hour compared to Block 3. It should be noted however that this intersection was on the border of LOS C and D during Block 3. This change in LOS for VIC02 was due to higher traffic volumes at this intersection in Block 4.
- Barangaroo Station:
 - All intersections within the Barangaroo Station study area operate at LOS C or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions.
- Martin Place Station:
 - All intersections within the Martin Place Station study area operate at LOS C or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions.
- Gadigal Station:
 - All intersections within the Gadigal Station study area operate at LOS C or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions.
- Central Station:
 - All intersections within the Central Station study area operate at LOS C or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions.
- Waterloo Station:
 - All intersections within the Waterloo Station study area operate at LOS C or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions. Botany Road/Raglan Street (WLO01) had a notable change in LOS, whereby the intersection improved from a LOS D to a LOS C in the weekday PM peak hour compared to Block 3. This change in LOS for WLO01 was due to lower traffic volumes at this intersection in Block 4.
- Sydenham Station:
 - All intersections within the Sydenham Station study area operate at LOS B or better during all peak hours.
 - Block 4 intersection performance is generally similar to pre-opening conditions.
- Transport interchange monitoring: The interchange operation surveys focused on analysing taxi, bus stop, kiss and ride, and accessible parking facilities at Chatswood Station, Crows Nest Station, Victoria Cross Station, Barangaroo Station, Waterloo Station and Sydenham Station. The key findings are summarised as follows:
 - Chatswood Station:
 - The capacities of the interchange facilities were generally sufficient to cater for the existing demand, with no queues extending outside the bays.
 - Block 4 vehicle demands were generally similar to pre-opening conditions for all interchange facilities across all peak hours.

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- Crows Nest Station:
 - The capacities of the interchange facilities were generally sufficient to cater for the existing demand, with no queues extending outside the bays.
- Victoria Cross Station
 - The capacities of the interchange facilities were generally sufficient to cater for the existing demand, with no queues extending outside the bays.
- Barangaroo Station
 - The capacities of the interchange facilities were generally sufficient to cater for the existing demand, with no queues extending outside the bays.
- Waterloo Station
 - The capacities of the interchange facilities were generally sufficient to cater for the existing demand, with no queues extending outside the bays.
- Sydenham Station:
 - The capacities of the interchange facilities were generally sufficient to cater for the existing demand, with no queues extending outside the bays.
 - Block 4 vehicle demands were generally similar to pre-opening conditions for all interchange facilities, with the exception of Sydenham Road (SYDK2) which had demands that were substantially higher than pre-opening conditions across all peak hours. This increase is likely due to the opening of the new Sydney Metro station access on Sydenham Road near this facility.

In summary, the results from Block 4 traffic monitoring demonstrate generally satisfactory intersection performance, consistently achieving LOS D or better across all stations. The assessment of interchange facilities at Chatswood, Crows Nest, Victoria Cross, Barangaroo, Waterloo and Sydenham stations generally indicates sufficient provision to meet the demand observed during Block 4.

Appendix A

Stakeholder meeting minutes

Appendix A Stakeholder meeting minutes



Minutes of Meeting

Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring

Subject	Block 4 Presentation	Page	1
Venue	MS-Teams	Time	3:00pm - 4:00pm
Participants	Nita Hutapea (NH), Sydney Metro Jennifer Adams (JA), Inner West Council Minas Kassiou (MK), Inner West Council Michael Huy (MH), Inner West Council George Tsaprounis (GT), Inner West Council Arvind Narwal (AN), Inner West Council Anoop Sridhar (AS), AECOM Mack Brinums (MB), AECOM Jimmy Wan (JW), AECOM Mark Yeung (MY), AECOM		
Apologies	Garry Hitchcox (GH), Sydney Metro		
File/Ref No.	SM-C&SW-MM-IWC-004	Date	29-Jan-2025
Distribution	As above		

No	Item	Action	Date
1.	 Project Overview and Results NH gave an overview/background on the project. AS gave an overview of scope of works, approach and results of Block 4 monitoring. 	-	-
2.	 GT queried the number of bays at SYDK2 and whether the capacity was sufficient. MB noted there were two formal bays, however there were a number of instances where vehicles would drop-off across the driveways on either side of the bay, effectively accommodating four cars at one time. Block 4 observations indicated that the bay was sufficient with accommodating the demand without impacting through traffic. However, it was close to capacity, with video footage indicating occasions where a vehicle would leave the bay as another vehicle would arrive. AS noted Block 4 surveys occurred shortly after the opening of Sydney Metro. This bay would continue to be monitored in Block 5 and 6 to see if there is any change in demand as people become more aware of the available interchange facilities. 	Note	-

ΑΞϹΟΜ

No	ltem	Action	Date
3.	 GT queried whether there was any data on turnstile numbers at Sydenham Station. AS noted tap-on/tap-off data was available, however it was difficult to aggregate it to the exact dates the Block 4 surveys were completed. MB noted that the Opal tap-on/tap-off data is collated by month. Sydney Metro commenced operations on 19 August 2024, therefore the August Opal data would not have shown the full increase in trips as a result of Sydney Metro given Sydney Metro was not operating for half the month. Similarly, the Bankstown line was closed in September, so September data would have been skewed whereby half the data would have had the Bankstown line closed. GT requested some further analysis on Opal data in future presentations. For Council's reference, Train and Metro Station Entry and Exit data can be downloaded here: https://opendata.transport.nsw.gov.au/data/datas et/train-station-entries-and-exits-data 	AECOM to provide some further analysis on Opal data in future presentations	Block 5
4.	 MH queried whether there was a breakdown of data available for the number of taxis vs general vehicles using SYDT1. AS noted a breakdown of vehicles was not available, as the survey company just records when a vehicle enters or leaves a bay. Given the extensive amount of data being collected (24 hours for 7 days), it is difficult to go into further detail to difference between taxis and general vehicles. It was agreed that AECOM would manually review the video footage for the identified AM, PM and WE peak hours and advise the split of taxis vs general vehicles. MH requested further detail on use of SYDT1 for future presentations. 	AECOM to review Block 4 video footage for the identified AM, PM and WE peak hours and advise the split of taxis vs general vehicles AECOM to provide further commentary on breakdown of vehicles during peak hours at SYDT1	7/02/2025 Block 5

Enclosures:

- Block 4 Presentation Inner West Council
- Traffic survey data for Inner West Council sites



Minutes of Meeting

Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring

Subject	Block 4 Presentation	Page	1
Venue	MS-Teams	Time	10:00am - 11:00am
Participants	Nita Hutapea (NH), Sydney Metro Jerimia Tukadra (JT), North Sydney Council Anoop Sridhar (AS), AECOM Mack Brinums (MB), AECOM Jimmy Wan (JW), AECOM Mark Yeung (MY), AECOM		
Apologies	Garry Hitchcox (GH), Sydney Metro		
File/Ref No.	SM-C&SW-MM-NSC-001	Date	22-Jan-2025
Distribution	As above		

No	Item	Action	Date
1.	 Introduction Attendees introduced themselves, highlighting their roles and organisations. 		
2.	 Project Overview and Results NH gave an overview/background on the project. AS gave an overview of scope of works and the approach for Block 4 monitoring. 		
3.	 JT made note that all information was very helpful, particularly taxi demand information. MB clarified that the taxi bay demand includes all vehicles that stopped in the bay, regardless of whether they were a taxi or not. At CSTT1 in particular, observations indicated the majority of this demand was associated with general vehicles. 		
4.	 JT noted that Council has struggled to get taxis to use taxi zone on McLaren Street. NH noted that Sydney Metro had contacted the Taxi Council to try and increase utilisation of bay, but also noted Block 4 surveys were undertaken shortly after opening of Metro so it may take some time for people to become familiar with all the facilities. 	Note	

Enclosures:

- Block 4 Presentation North Sydney Council
- Traffic survey data for the North Sydney Council sites



Minutes of Meeting

Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring

Subject	Block 4 Presentation	Page	1
Venue	MS-Teams	Time	2:00pm - 3:00pm
Participants	Nita Hutapea (NH), Sydney Metro Chris Slenders (CS), TfNSW Anoop Sridhar (AS), AECOM Mack Brinums (MB), AECOM Jimmy Wan (JW), AECOM Mark Yeung (MY), AECOM		
Apologies	Garry Hitchcox (GH), Sydney Metro Khaled Dib (KD), TfNSW Zakaria Ahmad (ZA), TfNSW		
File/Ref No.	SM-C&SW-MM-TfNSW-004	Date	23-Jan-2025
Distribution	As above		

No	Item	Action	Date
1.	 Project Overview and Results MB gave an overview of Block 4 monitoring results. 	-	-
2.	 CS noted it was interesting to see a reduction in pedestrian volumes at PIT02 given TfNSW have done separate surveys which indicated pedestrian volumes at this intersection had increased, although acknowledged surveys likely were completed at different times. MB noted that there was quite a lot of construction work occurring at this intersection which reduced pedestrian path and crossing widths during Block 4, so pedestrians may have preferred to cross at a different intersection. Construction works associated with Castlereagh Street cycleway have now finished since the Block 4 surveys were completed. 	-	-

Enclosures:

- Block 4 Presentation
- Block 4 Survey Data

Appendix B

SIDRA Intersection modelling assumptions

Appendix B SIDRA Intersection modelling assumptions

Technical Assumptions and Outputs Memo

1.0 Traffic and Interchange monitoring data outputs

The following outputs are proposed to be provided for the traffic and interchange monitoring:

- Weekly profile graph for individual intersections for 24hr period.
- Summary of daily total traffic volumes per intersection/interchange in a tabular format.
- Weekly profile graph for each station (total of all intersections) for 24hr period.
- Summary of daily total traffic volumes for each station (total of all intersections) in a tabular format.
- Graph of total traffic flows of intersection for typical peak periods during weekdays (06:00-10:00 am and 03:00-07:00 pm) and weekends (10:00am 02:00pm).
- Turning movements for identified peak hours during weekdays AM and PM peaks and weekend peaks in a network flow diagram in excel spreadsheets.
- Pedestrian volumes for identified peak hours during weekdays AM and PM peaks and weekend peaks in a network diagram in excel spreadsheets.
- Vehicle counts for 7-day weekly profile, typical peak periods, identified peaks for interchanges to include:
 - o Vehicle counts for each bay
 - o Vehicle occupancy (passenger only, driver excluded)
 - Vehicle dwell time for each bay
 - Vehicle queue length (outside the bay)

2.0 SIDRA modelling related assumptions

Table 1 outlines technical assumptions that will be applied for SIDRA modelling analysis.

 Table 1
 SIDRA Modelling Assumptions

SI No.	Parameter	Assumption
1.	SIDRA Software Version	SIDRA 9.1
2.	Lane Configuration - Grade	A default 0% grade will be applied to all lanes / TCS plans where applicable.
3.	Lane Width	A default 3.3m lane width will be applied to all lanes.
4.	Approach / Exit Cruise Speed	Based on posted speed limit. A default speed of 50km/h will be adopted where posted speed limit is not enforced.
5.	Roundabout Entry Radius & Entry Angle	A default entry radius of 20m and a default entry angle of 30 degrees will be adopted for all roundabouts.
6.	Critical Gap & Follow-up Headway	The default 'Program' input will be adopted for all movements.
7.	Gap Acceptance	The default 'SIDRA Standard' gap acceptance capacity model will be adopted for all vehicle types.

SI No.	Parameter	Assumption
		Reference will also be made to relevant standards/requirements in Austroads (RMS Modelling Guidelines), where applicable.
8.	Vehicle Movement Start Loss & End Gain	Based on SCATS data provided and survey footages / site observations
9.	Pedestrian Walking Speed (Average)	1.2 m/s
10.	Pedestrian Crossing Distance	Based on intersection geometry/Program (TCS plan where available / Nearmap aerial imageries)
11.	Peak Flow Period	30 minutes
12.	Peak Flow Factor	95%
13.	Phasing Arrangements	Based on SCATS data provided
14.	Phase Time and Frequency	Based on SCATS data provided
15.	Yellow Time & All-Red Time	Based on SCATS data provided
16.	Site Cycle/phase Time	User-Given Phase Time (Based on Phase time & frequency)
17.	Maximum Number of Iterations for Network Analysis	A default 30 iterations will be adopted. Increases of the maximum number of iterations may be applied depending on the Diagnostics Status.
18.	Network Cycle Time	User-Given Cycle Time (Based on User-Given Phase Time for all signals within the network)
19.	Network Signal Coordination	Coordinated Sites / User offsets / CCGs will be defined based on SCATS data provided. Signal offsets included in the SIDRA models provided by Sydney Metro will be adopted where relevant SCATS data are not available.
20.	Queue in Outputs (Site & Network)	95th Percentile
21.	PCU factor	LV: 1.0, HV & Bus: 2.0, Bicycles: 0.3
22.	Site level of service method	Delay (RTA NSW)
23.	Extra Bunching (Site Analysis)	Based on RMS Traffic Modelling Guidelines
24.	Movement Classes	Based on each intersection geometry (LV, HV, Buses, Bicycles)
25.	All other parameters	Default SIDRA settings

The following additional assumptions will be adopted for SIDRA modelling based on the discussion with Sydney Metro on 04 Apr 2023.

Table 2 Additional SI	ORA Modelling	Assumptions
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SI No.	Items	Assumption
1.	Network peak hours	For each station, peak hours will be identified for individual intersections and proposed networks (highlighted in green cells in Figure 1). By reviewing these individual and network peak hours, a station-wide peak hour will be nominated/adopted for each peak period. Peak period dates will be identified for each station

https://aecom.sharepoint.com/sites/SydneyMetroCSW/Shared Documents/General/200_Project_Control/210_Project_Plan_Risk/Appendix 2 -Technical Assumptions and Outputs Memo (v2).docx Revision 0 – 24-Mar-2023

Prepared for - Sydney Metro - ABN: 12 354 063 515

SI No.	Items	Assumption
		for AM, PM and weekend. For eg.SYD AM Peak - Tuesday; SYD PM Peak - Thursday; WLO AM Peak - Wednesday
2.	Cyclist movements	For SIDRA modelling, cyclist movements will only be included if there is a dedicated cycling phase.
3.	Intersection approach/lane closure	Due to construction activities, some approaches/lanes were observed temporarily (partially) closed on site. These temporary closures will be reflected in the models unless it only occurs for a short period of time (for e.g. 10 to 15mins). Notes will be made to approach/lane closure observed on-site, and approach/lane excluded in SIDRA modelling.
4.	CST06 intersection geometry	Hume St North (one-way exit) will not be included in Block 1 modelling. Notes will be made to the left turn movements observed from Clarke St northwest to Hume St north.
5.	BGU05 intersection geometry	Clarence St northbound on-ramp lane to SHB will not be included in the modelling.
6.	CEN03/CEN05 intersection geometry	Elizabeth St/Randle St intersection has been included as CEN05, and will be modelled as network model with CEN03.



Figure 1 Adopted Network Volume for Network Peak Hour Identification

The following SIDRA outputs would be provided for each intersection.

- Degree of saturation (DoS)
- Average delay (sec)
- 95th percentile queue (m)
- Level of service (LoS)

A sample format of the output table is shown in Table 3.

Table 3 Example SIDRA output format

Intersection	Peak	Leg	Degree of saturation (DoS)	Average delay (sec)	95 th percentile queue (m)	Level of service (LoS)
		South				
		East				
	AM	North				
		West				
		Total				
Road1 /	РМ	South				
Road2		East				
(Signal /		North				
Roundabout /		West				
Priority)		Total				
		South				
		East				
	Weekend	North				
		West				
		Total				

Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring SIDRA Network Model Coverage

							SIDRA Network Model (AECOM	
S.ID	Intersection ID	Station Name	Intersection Name	Intersection Control	Intersection Geometry	Intersection Geometry	Revised)	Coordination
					Layout	Code	Pre-opening	
01	CWD01	Chatswood Station	Mowbray Rd / Hampden Rd	Signal	3-leg Intersection	2_4_6	-	-
02	CWD02	Chatswood Station	Pedestrian Bridge Crossing along Mowbray	Pedestrian only - Bridge Crossing	Bridge Crossing	2_6	-	-
03	CST01	Crows Nest Station	Pacific Hwy / Albany St	Signal	3-leg Intersection	3_4_8	CST-N1	Offset_CST-N1
04	CST02	Crows Nest Station	Pacific Hwy / Oxley St	Signal	4-leg Intersection	2468	CST-N1	Offset CST-N1
05	CST03	Crows Nest Station	Pacific Hwy / Hume St	Signal	4-leg Intersection	2 4 6 8	CST-N1	Offset CST-N1
06	CST04	Crows Nest Station	Pacific Hwy / Falcon St / Shirley Rd	Signal	5-leg Intersection	1 3 4 6 8	CST-N1	Offset CST-N1
07	CST05	Crows Nest Station	Clarke St / Ovley St	Priority - Give Way	3-leg Intersection	1 4 6	CST-N1	
08	CSTOR	Crows Nest Station	Clarke St / Hume St	Priority Give Way	A leg Intersection	1 4 6 8	CST N1	
00	CST00	Crows Nest Station	Clarke St / Willoughby Bd	Priority Cive Way	2 log Intersection	1 5 7	031-111	-
09	00700			Filolity - Give way	3-leg intersection		-	-
10	CS108	Crows Nest Station	Albany St / Willoughby Rd	Signal	4-leg Intersection	1_3_5_7		-
11	CST09	Crows Nest Station	Albany St / Oxley St	Roundabout	4-leg Intersection	1_3_5_7	CST-N1	-
12	CST10	Crows Nest Station	Albany St / Clarke Ln	Priority - Give Way	3-leg Intersection	3_4_7	CST-N1	-
13	CST11	Crows Nest Station	Oxley St / Clarke Ln	Priority - Stop	4-leg Intersection	2_4_6_8	CST-N1	-
14	CST12	Crows Nest Station	Hume St / Clarke Ln	Priority - Stop	3-leg Intersection	2_4_6	CST-N1	-
15	CST13	Crows Nest Station	Pacific Hwy / Alexander St	Signal	4-leg Intersection	1_3_4_8	CST-N1	Offset_CST-N1
16	CST14	Crows Nest Station	Falcon St / Alexander St	Signal	4-leg Intersection	1_3_5_7	CST-N1	Offset_CST-N1
17	VIC01	Victoria Cross Station	Pacific Hwy / Berry St	Signal	4-leg Intersection	3468	VIC-N1	Offset VIC-N1
18	VIC02	Victoria Cross Station	Miller St / Berry St	Signal	4-leg Intersection	1 3 5 7	VIC-N1	Offset VIC-N1
19	VIC03	Victoria Cross Station	Miller St / Mcl aren St	Signal	4-leg Intersection	1 3 5 7	VIC-N1	
20	VIC04	Victoria Cross Station	Pacific Hwy / Miller St	Signal	5-leg Intersection	1 4 5 7 8	VIC-N1	Offset VIC-N1
20	BCU01	Barangaroo Station	Hickson Pd / Towns Pl	Priority Give Way	3 leg Intersection	368	BCU N1	Oliset_ViC-NT
21	DOUDD	Darangaroo Station	Delecto Del (Teuro Di	Developent		3_0_0	DGU-N1	-
22	BGUUZ	Barangaroo Station	Daigety Rd / Towns Pi	Roundabout	3-leg intersection	4_5_7	BGU-N1	-
23	BGU03	Barangaroo Station	Kent St / Argyle St	Priority - Give Way	4-leg Intersection	1_3_5_7	-	-
24	BGU04	Barangaroo Station	Pedestrian Mid-block Crossing at Kent St near Gas Ln	Pedestrian only - Signal	Pedestrian Mid-block Crossing	1_5	BGU-N2	Offset_BGU-N2
25	BGU05	Barangaroo Station	Kent St / Sydney Harbour Bridge (SHB) On-ramp	Signal	4-leg Intersection	1_2_3_5	BGU-N2	Offset_BGU-N2
26	BGU06	Barangaroo Station	Hickson Rd / Napoleon St / Sussex St	Signal	4-leg Intersection	1_3_5_7	BGU-N3	-
27	BGU07	Barangaroo Station	Margaret St / Kent St / Napoleon St	Signal	4-leg Intersection	1 3 5 8	BGU-N2	Offset BGU-N2
28	BGU08	Barangaroo Station	Margaret St / Clarence St	Signal	4-leg Intersection	1 3 5 7	BGU-N2	Offset BGU-N2
29	BGU09	Barangaroo Station	Margaret St / York St	Signal	A-leg Intersection	1 3 5 7	BGU-N2	
30	BGU10	Barangaroo Station	Pedestrian Mid block Crossing at Sussex St under Exchange Pl	Pedestrian only Signal	Pedestrian Mid block Crossing	1 5	BOU N3	-
21	BGU10	Barangaroo Station	Pedestrian Mid-block Crossing at Kept St peer Margaret St	Pedestrian only Signal	Pedestrian Mid-block Crossing	1_5	BG0-N3 BCU N3	-
31	BOUIT	Barangaroo Station		Fedestrian only - Signal	Fedestrian wid-block crossing		BGU-N3	
32	BGU12	Barangaroo Station	Sussex St / Erskine St	Signal	4-leg Intersection	1_3_5_7	BGU-N3	Offset_BGU-N3
33	BGU13	Barangaroo Station	Kent St / Erskine St	Signal	4-leg Intersection	1_3_5_7	BGU-N3	Offset_BGU-N3
34	BGU14	Barangaroo Station	Sussex St / King St	Signal	4-leg Intersection	1_3_5_6	BGU-N4	Offset_BGU-N4
35	BGU15	Barangaroo Station	Kent St / King St	Signal	4-leg Intersection	1_3_5_7	BGU-N4	Offset_BGU-N4
36	BGU16	Barangaroo Station	New Pedestrian Mid-block Crossing at Hickson Rd (north of Metro Station)	Pedestrian only - Zebra	Pedestrian Mid-block Crossing	1_5	-	-
37	BGU17	Barangaroo Station	New Pedestrian Mid-block Crossing at Hickson Rd (south of Metro Station)	Pedestrian only - Zebra	Pedestrian Mid-block Crossing	1 5	-	-
38	BGU18	Barangaroo Station	Shelley St / Frskine St	Signal	4-leg Intersection	1357	BGU-N3	-
39	MPL 01	Martin Place Station	Hunter St / Castlereagh St / Bligh St	Signal	4-leg Intersection	1358	MPL-N1	Offset MPL-N1
40	MDL 02	Martin Place Station	Hunter St / Elizabeth St / Chifley Square	Signal		2 2 5 7		Offact MPL N1
40	IVIFL02		Hunter St / Elizabeth St / Chiney Square	Signal	4-leg intersection	2_3_5_7		OliseLMPL-NT
41	MPL03	Martin Place Station	Bent St / Bligh St	Signal	3-leg Intersection	4_6_8	MPL-N1	Offset_MPL-N1
42	MPL04	Martin Place Station	Bent St / Phillip St	Signal	4-leg Intersection	1_4_6_8	MPL-N1	Offset_MPL-N1
43	MPL05	Martin Place Station	Pedestrian Mid-block Crossing at Castlereagh St	Pedestrian only - Signal	Pedestrian Mid-block Crossing	1_5	-	-
44	MPL06	Martin Place Station	Pedestrian Mid-block Crossing at Elizabeth St	Pedestrian only - Signal	Pedestrian Mid-block Crossing	1_5	-	-
45	PIT01	Pitt Street Station	Pitt St / Bathurst St	Signal	4-leg Intersection	1_3_5_7	PIT-N1	-
46	PIT02	Pitt Street Station	Castlereagh St / Bathurst St	Signal	4-leg Intersection	1_3_5_7	PIT-N1	-
47	PIT03	Pitt Street Station	Park St / Castlereagh St	Signal	4-leg Intersection	1 3 5 7	PIT-N1	-
48	PIT04	Pitt Street Station	Park St / Pitt St	Signal	4-leg Intersection	1 3 5 7	PIT-N1	-
49	CEN01	Central Station	Flizabeth St / Eddy Ave	Signal	3-leg Intersection	1.5.8	CEN-N1	Offset CEN-N1
#0 E0	CENIO	Central Station	Elizabeth St / Equation St	Signal	3 leg Intersection	1 4 5	CEN N4	Offeet CEN NI
50	OENU2		Elizabeth Ot / Fovedux St	Drive Ma		1_4_0		Unser_CEN-NT
51	CEN03	Central Station	Elizabeth St / Cooper St	Priority - Give way	3-leg Intersection	1_4_5	CEN-N2	-
52	CEN04	Central Station	New Pedestrian Mid-block Crossing at Randle Ln	Pedestrian only - Signal	Pedestrian Mid-block Crossing	2_6	-	-
53	CEN05	Central Station	Elizabeth St / Randle St	Signal	3-leg Intersection	1_5_6	CEN-N2	-
54	WLO01	Waterloo Station	Botany Rd / Raglan St / Henderson Rd	Signal	4-leg Intersection	1_3_5_7	WLO-N1	Offset_WLO-N1
55	WLO02	Waterloo Station	Raglan St / Cope St	Signal	4-leg Intersection	1_3_5_7	WLO-N1	-
56	WLO03	Waterloo Station	Botany Rd / Wellington St / Buckland St	Signal	4-leg Intersection	1 3 5 7	WLO-N1	Offset_WLO-N1
57	WLO04	Waterloo Station	Cope St / Wellington St	Priority - Stop	4-leg Intersection	1 3 5 7	WLO-N1	-
58	WL 005	Waterloo Station	Wyndham St / Henderson Rd	Signal	4-leg Intersection	1357	WLO-N1	Offset WLO-N1
50	WL 006	Waterloo Station	New Pedestrian Mid block Crossing at Cope St	Pedestrian only Zebra	Pedestrian Mid block Crossing	1.5	TLO-NI	ONSC_MEDINI
09	EVD04	Sudanham Ctation	Pailway Dda / Classen Ave	Cignol	2 log Intersection	246	SVD N4	-
60	STDUT	Sydennam Station	naiway rue / Gleeson Ave	Signal	3-leg intersection	2-4-0	STU-N1	-
61	SYD02	Sydenham Station	Burrows Ave / Gieeson Ave	Signal	4-leg intersection	2_4_0_8	SYD-N1	-
62	SYD03	Sydenham Station	Burrows Ave / George St	Priority - Give Way	3-leg Intersection	2_4_6	-	-
63	SYD04	Sydenham Station	Railway Pde / Sydenham Rd	Signal	3-leg Intersection	5_6_8	-	-
64	SYD05	Sydenham Station	Marrickville Rd / Buckley St	Priority - Give Way	3-leg Intersection	2_4_8	-	-
65	SYD06	Sydenham Station	Sydenham Rd / Buckley St	Priority - Give Way	3-leg Intersection	4_6_8	-	-

Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring Intersection Geometry

Source: Nearmap accessed XX XX XXXX

Include NearMaps layout (already prepared for each site) and include a markup showing the approach distances, short lane lengths, parking zone, no stopping zone etc.



Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring

Site Name:	
Site ID:	
Type:	
Scenario:	
Links to:	
Links to: SIDRA File	

atus			
Open	Attention Required for modeller / reviewer		
Progress	Working in progress		
Closed	Closed		
N/A	Not Applicable/Not Required		
odeller:			

AM Po

PM Poak

rranic volume input	
SCATS Data	TCS Plan

item	Model Element	Notes (For modeller)	Modeller		Reviewer		Verifier		Modeller		Reviewer		Verifier		Modeller		Reviewer	
1	General		Status	Notes														
1.1	SIDRA Setup	New South Wales	Open															
1.2	Intersection Type	For priority intersections, check for 'give way' or 'stop'	Open		Open	1												
2	Intersection																	
2.1	Site Name	To be consistent with the Intersection Master List	Open		Open	(
2.2	Site Title	Include TCS numbers in the model. if applicable	Open															
2.3	Approach Names	Include as per Nearmap, compare with Intersection Master List	Open		Open	<i>i</i>												
2.4	Leg Geometry	Two-way, one-way etc.	Open		Open	<i>i</i>												
2.5	Approach/Exit Distance	Check and update as per NearMaps (distance till the next intersection if more than 500m)	Open															
		For isolated intersections, include as per Traffic modelling guidelines. For sites in the network, ensure Program																(III - IIII - III - IIII - IIIII - IIII - IIII - IIII - IIIII - IIII - IIII - IIII - IIII - IIIII - IIII - IIII - IIII - IIII - IIIII - IIIII - IIII - IIII - IIIII - IIIII - IIII - IIIII - IIIII - IIIII - IIIII - IIIII - IIIII - IIIIII
2.6	Extra Bunching	option is selected for 'network internal' approaches (user input should still be included for 'network external'	Open		Open	(III - IIII - III - IIII - IIIII - IIII - IIIII - IIII - IIIII - IIII - IIIII - IIII - IIII - IIII - IIIII - IIIII - IIII - IIII - IIIII - IIIII - IIII - IIII - IIII - IIIII - IIII - IIII - IIII - IIIII - IIIII - IIII - IIII - IIIII - IIIIII												
	-	approaches, where applicable).																
3	Movement Definitions																	
3.1	Vehicle Types	Confirm inclusion of Ruses Biownes if relevant (for easier volume input select Bus and biownes for all intersections)	Open		Open		Onen		Open		Open		Onen		Open		Open	
0.1	Venice Types	comministration of Babba, Boyees, in recent (for cashe volume input, select bas and boyees for an intersection)					opon		Open				open				Open	
3.2	OD Movements	Switch off banned movements as her site observations, compare with Intersection Master list for banned movements	Onen		Onen		Open		Open		Onen		Open		Open		Open	
		owner en damed motentents as per site observations, compare war merecetter matter not for damed motentents.																
4	Lane Geometry								_									
4.1	Lane Configuration / Length	Check the full length of lane and 'short lane' based on Nearmap - refer Intersection Geometry tab (round to 5m)	Open															
4.0	Less Tree	Under and and an ender for effectione					0		0				0				0	
4.2	Lane Type	High angle or Low angle for slip lanes	Open															
4.5	Current and Number		Open		Open		Open		Open		Open		Open		Open		Open	
4.4	Grade	A defende 000 versie will be explicit to all leaves (TCC eleverythere explicitly	Open		Open	1												
4.5	Lana Dissiplinos	A detault une grade will de applied to all lanes. / I CS plans where applicable.	Open		Open	1												
4.0	Lane Capacity Adjustment	Options in specific movement classes have barried invertients (or eq. right turn only of DUSes)	Open															
5	Lane Movements	Justinications based on site observations required it these factors are adjusted	Open															
5	Lane movements	As per site observations or support ideas. From approach lane to exit lane (e.g. hus lane on approach side should																
5.1	Lane Movement Proportion	As per site observations of survey videos. I for approach are to exit rare (e.g. bus rare on approach side should direct to bus rare on out side)	Open															
6	Roundabout (if applicable)																	
6.1	Number of Lanes		N/A		N/A		N/A		N/A		N/A		N/A		N/A		N/A	
6.2	Circulating Width		N/A		N/A		N/A		N/A		N/A		N/A		N/A		N/A	1
6.3	Island Diameter		N/A		N/A		N/A		N/A		N/A		N/A		N/A		N/A	1
		Include ped crossing for all rounadbouts (with / without zebra crossing); if there's no zebra crossing, make a note in																1
6.4	Ped Crossing at Roundabout	the checklist - 'No zebra crossing, priority settings (ped or veh) to be further revied with survey footages to calibrate	N/A		N/A	1												
		the model.'																1
7	Pedestrians																	
7.1	Crossing Location / Type	Full crossing / staged crossing / slip lane crossing (signalised or zebra)	Open															
7.2	Pedestrian Volume	Update as per surveys	Open															
7.3	Peak Flow Factor	95%	Open															
7.4	Crossing Distance	Based on intersection geometry (round to 0.5m)	Open															
7.5	Walking Speed (Average)	1.2 m/s (as recommended in RMS Modelling Guide)	Open		Open	(III - IIII - III - IIII - IIIII - IIII - IIIII - IIII - IIIII - IIII - IIIII - IIII - IIII - IIII - IIIII - IIIII - IIII - IIII - IIIII - IIII - IIII - IIII - IIII - IIII - IIIII - IIII - IIII - IIII - IIIII - IIIIII												
7.6	Pedestrian Timing Data	Adopt the SCATS walk time as minimum walk time, minimum clearance as default 5 sec, Clearance 1 & 2 as per	Onen		Onen		Open		Open		Onen		Open		Open		Open	
		SCATS data							-									
1.7	waik Time Extension	Remain as 'unticked' (can adjust based on survey videos, where applicable)	Open															
8	volumes								-									
8.1	Vehicle Volumes	Check individual intersections; For network model, check midblock flows (ensure inpit volumes are set to 'Separate')	Open															
8.2	Peak Flow Period	20 minutos	Open		Onen		Open		Open		Onen		Onen		Open		Open	
83	Peak Flow Factor	06%	Open															
9	Priorities		Open		Open		open		Open		Open		Open		Open		Open-	
9.1	Priorities	Ensure priority settings updated for turn movements at signals with opposed ped movements	Open		Onen		Open		Onen		Onen		Open		Open		Open	
10	Gap Acceptance		e pen						0,0011								0,000	
10.1	Opposing Peds (Extra Loss)	Justifications required if these factors are adjusted	Open															
11	Vehicle Movement Data																	
11.1	Approach / Exit Cruise Speed	Based on posted speed limits or agreed assumptions (if no posted speed limits)	Open															
11.2	Start Loss / End Gain	Justifications required if these factors are adjusted	Open															
11.3	Early Cut-Off / Late Start	Justifications required if these factors are adjusted	Open															
12	Phasing & Timing (if applicable)																	
12.1	Phasing Arrangements	As per SCATS. TCS Plan	Open															
12.2	Red Arrow Drop Off		Open		Open		Open		Open		Open		Open		Open		Open	
12.3	Phase Time / Frequency	User-give phase times. Frequency as per SCATS/Site observations	Open															
12.4	Yellow Time	As per SCATS (if SCATS data indicates .5, round up and leave a note in the checklist)	Open															
12.5	All-Red Time	As per SCATS (if SCATS data indicates .5, round up and leave a note in the checklist)	Open															
13	Parameter Settings																	
13.1	Site LoS Method	Delay (RTA NSW); Site Level of Service Target LoS C	Open															
13.2	Queue in Output	95th Percentile	Open															
13.3	Inco lactor	[LV: 1.0, HV & BUS: 2.0, Bicycles: 0.3	Open		Open -		Open											

Weekend Peak

Sydney Metro City & Southwest - Traffic and Interchange Operation Monitoring SIDRA Network Model Coverage

Site Name: CHW Network 1 Site D: Network 1 Site D: NA Scenario: TBC Links to: Clease 1 SIDRA File NA Traffic Volume Input Reviewer: SCATS Data Verifier:																				
ltem	Model Element	Notes	Mod	eller		AM Peak Reviewer	1	Verifier		Modeller		PM Peak Reviewer		Verifier		Modeller	S	Sat Peak Reviewer		Verifier
1	Network Data		Status	Notes	Status	Notes	Status	Notes	Status	Notes	Status	Notes	Status	Notes	Status	Notes	Status	Notes	Status	Notes
1.1	Queue in Output	95th Percentile	Open		Open		Open		Open		Open		Open		Open		Open		Open	
1.2	Maximum Number of Iterations	30; unless notes are given in Diagnostics	Open		Open		Open		Open		Open		Open		Open		Open		Open	
2	CCGs																			
2.1	CCG Set Up	If applicable	Open		Open		Open		Open		Open		Open		Open		Open		Open	
3	Network Timing																			
3.1	Coordinated Site Selection	If applicable	Open		Open		Open		Open		Open		Open		Open		Open		Open	
3.2	User Offset	If applicable	Open		Open		Open		Open		Open		Open		Open		Open		Open	
3.3	Route Defination	If applicable	Open		Open		Open		Open		Open		Open		Open		Open		Open	
3.4	Network Cycle Time	If applicable	Open		Open		Open		Open		Open		Open		Open		Open		Open	

Appendix C

Network flow diagrams

Appendix C Network flow diagrams









Block 4

79				
7 ↓ 68	196 ݷ	490 3	Albany Street	
50 1		519		
8				

Alexander Street 377 1 360 To CST07 136 **1** 712 → 7 → 332 28 ⊥ → 804 Falcon Stree t 11
 ← 676
 ⊊ 27 Ť ↑ ↑ ↑
1 230 64 **←** -21 677 714 295 359


Block 4

Time Period

69				
12	100	433		
↓ 45		3	Albany Street	
62		424		
7				

Alexander Street 394 **1** 302 To CST07 208 **1** 730 → 20 → 270 32 805 Falcon Stree
 ←
 ↑
 ⊢

 10
 178
 43
 ← 21 721 755 231 306



Block 4

Vehicle Type Time Perio

29 115 368 ↓ ↓ 3 55 3 80 456 Albany Street	86			
↓ ↓ Albany Street 55 456	29	115	368	
88 456	1 55	4	3	Albany Street
430	88		456	











































Appendix D

Movement summary outputs

Appendix D Movement summary outputs

Site: CWD01 [CWD01 Mowbray Rd / Hampden Rd (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 3037

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 150 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [Veh. veh	ack Of eue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Hampder	n Rd (Sl	E)											
21	L2	All MCs	215	3.9	215	3.9	0.516	58.6	LOS E	13.7	99.3	0.92	0.82	0.92	18.2
Appro	ach		215	3.9	215	3.9	0.516	58.6	LOS E	13.7	99.3	0.92	0.82	0.92	18.2
NorthE	East: I	Mowbray	Rd (NE)											
24	L2	All MCs	93	0.0	93	0.0	0.459	22.0	LOS B	20.3	146.4	0.60	0.58	0.60	32.1
25	T1	All MCs	935	4.5	935	4.5	*0.459	17.4	LOS B	20.3	147.5	0.60	0.55	0.60	25.9
Appro	ach		1027	4.1	1027	4.1	0.459	17.8	LOS B	20.3	147.5	0.60	0.56	0.60	26.7
North\	Vest:	Dive Site	Access	s (NV	/)										
27	L2	All MCs	1	0.0	1	0.0	0.001	3.5	LOS A	0.0	0.1	0.14	0.43	0.14	34.9
29	R2	All MCs	1	0.0	1	0.0	*0.009	74.6	LOS F	0.1	0.5	0.96	0.59	0.96	6.3
Appro	ach		2	0.0	2	0.0	0.009	39.0	LOS C	0.1	0.5	0.55	0.51	0.55	11.5
South	West:	Mowbray	/ Rd (S	W)											
31	T1	All MCs	931	3.6	931	3.6	0.320	5.3	LOS A	10.0	71.8	0.33	0.29	0.33	39.1
32	R2	All MCs	389	2.7	389	2.7	*0.478	19.1	LOS B	18.6	133.1	0.74	0.81	0.74	30.9
Appro	ach		1320	3.3	1320	3.3	0.478	9.4	LOS A	18.6	133.1	0.45	0.45	0.45	35.1
All Vel	nicles		2564	3.7	2564	3.7	0.516	16.9	LOS B	20.3	147.5	0.55	0.52	0.55	28.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	lovemei	nt Perfo	ormanc	e							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of A Service	VERAGE E QUEL	BACK OF JE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist. S	Aver. Speed
	ped/h	ped/h	sec		ped	m		Trate	sec	m	m/sec
SouthEast: Hampden Rd (SE)											
P5 Full	5	5	68.2	LOS F	0.0	0.0	0.95	0.95	234.8	200.0	0.85
NorthEast: Mo	wbray Ro	l (NE)									
P6 Full	28	29	68.2	LOS F	0.1	0.1	0.95	0.95	234.9	200.0	0.85
NorthWest: Div	ve Site Ac	ccess (N	W)								
P7 Full	1	1	70.1	LOS F	0.0	0.0	0.97	0.97	236.8	200.0	0.84
All Pedestrians	34	36	68.3	LOS F	0.1	0.1	0.95	0.95	235.0	200.0	0.85

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Site: CWD01 [CWD01 Mowbray Rd / Hampden Rd (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 3037

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 150 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [Veh. veh	ack Of eue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Hampder	n Rd (Sl	E)											
21	L2	All MCs	201	0.5	201	0.5	0.419	54.0	LOS D	12.2	85.5	0.88	0.80	0.88	19.1
Appro	ach		201	0.5	201	0.5	0.419	54.0	LOS D	12.2	85.5	0.88	0.80	0.88	19.1
NorthE	ast: I	Mowbray	Rd (NE)											
24	L2	All MCs	9	0.0	9	0.0	0.506	24.8	LOS B	24.1	170.0	0.65	0.59	0.65	30.9
25	T1	All MCs	1111	1.0	1111	1.0	*0.506	20.2	LOS B	24.1	170.1	0.65	0.59	0.65	24.2
Appro	ach		1120	1.0	1120	1.0	0.506	20.3	LOS B	24.1	170.1	0.65	0.59	0.65	24.3
North	Vest:	Dive Site	Access	s (NV	/)										
27	L2	All MCs	1	0.0	1	0.0	0.001	3.2	LOS A	0.0	0.1	0.13	0.43	0.13	35.3
29	R2	All MCs	1	0.0	1	0.0	*0.009	74.6	LOS F	0.1	0.5	0.96	0.59	0.96	6.3
Appro	ach		2	0.0	2	0.0	0.009	38.9	LOS C	0.1	0.5	0.54	0.51	0.54	11.5
South	Nest:	Mowbray	Rd (S	W)											
31	T1	All MCs	836	0.8	836	0.8	0.282	5.4	LOS A	8.8	62.2	0.32	0.29	0.32	38.9
32	R2	All MCs	314	0.7	314	0.7	*0.447	22.2	LOS B	16.6	116.8	0.80	0.83	0.80	29.3
Appro	ach		1149	0.7	1149	0.7	0.447	10.0	LOS A	16.6	116.8	0.45	0.43	0.45	34.4
All Vel	nicles		2473	0.9	2473	0.9	0.506	18.3	LOS B	24.1	170.1	0.58	0.53	0.58	27.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	loveme	nt Perf	ormanc	e								
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of A	AVERAGE QUE	BACK OF UE Diet 1	Prop. Que	Eff. Stop	Travel Time	Travel Dist. 3	Aver. Speed	
	ped/h	ped/h	sec		ped	m		Hate	sec	m	m/sec	
SouthEast: Hampden Rd (SE)												
P5 Full	12	13	68.2	LOS F	0.1	0.1	0.95	0.95	234.9	200.0	0.85	
NorthEast: Mo	wbray Ro	d (NE)										
P6 Full	49	52	68.3	LOS F	0.2	0.2	0.96	0.96	235.0	200.0	0.85	
NorthWest: Div	ve Site A	ccess (N	IW)									
P7 Full	1	1	70.1	LOS F	0.0	0.0	0.97	0.97	236.8	200.0	0.84	
All Pedestrians	62	65	68.3	LOS F	0.2	0.2	0.96	0.96	235.0	200.0	0.85	

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Site: CWD01 [CWD01 Mowbray Rd / Hampden Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 3037

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 145 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [Veh. veh	ack Of eue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Hampder	n Rd (Sl	E)											
21	L2	All MCs	289	1.1	289	1.1	0.532	50.2	LOS D	17.0	120.3	0.89	0.82	0.89	20.0
Appro	ach		289	1.1	289	1.1	0.532	50.2	LOS D	17.0	120.3	0.89	0.82	0.89	20.0
North	East: I	Mowbray	Rd (NE)											
24	L2	All MCs	31	0.0	31	0.0	0.548	27.1	LOS B	25.7	182.3	0.70	0.64	0.70	29.7
25	T1	All MCs	1112	1.8	1112	1.8	*0.548	22.6	LOS B	25.7	182.7	0.70	0.64	0.70	22.8
Appro	ach		1142	1.8	1142	1.8	0.548	22.7	LOS B	25.7	182.7	0.70	0.64	0.70	23.1
North\	Nest:	Dive Site	Access	s (NV	/)										
27	L2	All MCs	1	0.0	1	0.0	0.001	3.5	LOS A	0.0	0.1	0.14	0.44	0.14	34.9
29	R2	All MCs	1	0.0	1	0.0	*0.010	73.3	LOS F	0.1	0.5	0.96	0.59	0.96	6.4
Appro	ach		2	0.0	2	0.0	0.010	38.4	LOS C	0.1	0.5	0.55	0.51	0.55	11.6
South	West:	Mowbray	/ Rd (S	W)											
31	T1	All MCs	1096	0.5	1096	0.5	0.356	4.8	LOS A	10.8	76.2	0.31	0.28	0.31	40.4
32	R2	All MCs	372	0.8	372	0.8	*0.501	27.6	LOS B	18.4	129.6	0.80	0.89	0.80	27.2
Appro	ach		1467	0.6	1467	0.6	0.501	10.6	LOS A	18.4	129.6	0.44	0.44	0.44	33.7
All Vel	hicles		2901	1.1	2901	1.1	0.548	19.3	LOS B	25.7	182.7	0.59	0.55	0.59	26.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	lovemei	nt Perfe	ormanc	e								
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of A Service	VERAGE E QUEL	BACK OF JE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed	
	ped/h	ped/h	sec		ped	m		Nato	sec	m	m/sec	
SouthEast: Hampden Rd (SE)												
P5 Full	8	8	65.7	LOS F	0.0	0.0	0.95	0.95	232.4	200.0	0.86	
NorthEast: Mowbray Rd (NE)												
P6 Full	18	19	65.7	LOS F	0.1	0.1	0.95	0.95	232.4	200.0	0.86	
NorthWest: Div	ve Site Ac	cess (N	W)									
P7 Full	1	1	67.6	LOS F	0.0	0.0	0.97	0.97	234.3	200.0	0.85	
All Pedestrians	27	28	65.8	LOS F	0.1	0.1	0.95	0.95	232.4	200.0	0.86	

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Site: CST01 [CST01 Pacific Hwy / Albany St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 768

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 135 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Derr Fl [Total veh/h	nand lows HV] %	Ar Fl [Total] veh/h	rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
SouthEast: Pacific Hwy (SE)															
22	T1	All MCs	1296	5.0	1296	5.0	0.501	1.8	LOS A	5.6	40.9	0.15	0.13	0.15	54.6
23b	R3	All MCs	157	2.7	157	2.7	*0.892	83.2	LOS F	11.3	81.2	1.00	0.93	1.21	6.1
Appro	ach		1453	4.8	1453	4.8	0.892	10.6	LOS A	11.3	81.2	0.24	0.22	0.26	37.1
East:	East: Albany St (E)														
4b	L3	All MCs	29	3.6	29	3.6	*0.678	67.5	LOS E	6.8	49.0	0.98	0.84	0.99	2.7
6a	R1	All MCs	462	3.0	462	3.0	0.678	56.0	LOS D	6.8	49.0	0.98	0.84	0.98	10.1
Appro	ach		492	3.0	492	3.0	0.678	56.6	LOS E	6.8	49.0	0.98	0.84	0.99	9.7
North	Nest:	Pacific H	wy (NV	V)											
27a	L1	All MCs	323	1.3	323	1.3	0.557	23.7	LOS B	18.0	128.7	0.67	0.73	0.67	16.8
28	T1	All MCs	1229	5.7	1229	5.7	* 0.557	13.7	LOS A	18.0	128.7	0.52	0.48	0.52	25.8
Appro	ach		1553	4.7	1553	4.7	0.557	15.8	LOS B	18.0	128.7	0.55	0.53	0.55	23.2
All Ve	hicles		3497	4.5	3497	4.5	0.892	19.4	LOS B	18.0	128.7	0.48	0.44	0.49	24.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perforr	nance								
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.	
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed	
	ped/h	sec		ped	m			sec	m	m/sec	
SouthEast: Pacific Hwy (SE)											
P5 Full	301	55.8	LOS E	1.1	1.1	0.92	0.92	222.5	200.0	0.90	
East: Albany St (E)										
P2 Full	949	57.4	LOS E	3.4	3.4	0.94	0.94	74.1	20.0	0.27	
All Pedestrians	1251	57.0	LOS E	3.4	3.4	0.93	0.93	109.8	63.3	0.58	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST02 [CST02 Pacific Hwy / Oxley St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 767

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 135 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovement	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	UH	[Total	HV]	v/c	990		[Veh.	Dist]		Rate	Cycles	km/h
South	East:	Pacific H	wy (SE)	VCH/H	70	v/C	300		VCII		_	_		NIII/II
1	L2	All MCs	124	3.4	124	3.4	*0.579	17.6	LOS B	12.1	88.0	0.29	0.34	0.29	25.6
2	T1	All MCs	1298	5.0	1298	5.0	0.579	6.9	LOS A	12.6	91.7	0.29	0.30	0.29	37.3
Appro	ach		1422	4.9	1422	4.9	0.579	7.9	LOS A	12.6	91.7	0.29	0.30	0.29	35.2
North	East:	Oxley St ((NE)												
4	L2	All MCs	97	4.3	97	4.3	0.246	49.1	LOS D	5.3	38.2	0.86	0.75	0.86	2.8
5	T1	All MCs	94	2.2	94	2.2	0.221	46.1	LOS D	5.0	35.9	0.86	0.68	0.86	7.9
Approach			191	3.3	191	3.3	0.246	47.6	LOS D	5.3	38.2	0.86	0.72	0.86	5.4
North	West:	Pacific H	wy (NV	V)											
7	L2	All MCs	72	1.5	72	1.5	0.345	9.0	LOS A	3.2	23.4	0.13	0.21	0.13	43.7
8	T1	All MCs	1187	5.9	1187	5.9	0.345	0.9	LOS A	3.2	23.4	0.06	0.08	0.06	52.6
Appro	ach		1259	5.6	1259	5.6	0.345	1.4	LOS A	3.2	23.4	0.06	0.09	0.06	52.0
South	West	: Oxley St	(SW)												
10	L2	All MCs	155	2.7	155	2.7	*0.505	62.4	LOS E	9.3	66.8	0.95	0.80	0.95	5.0
11	T1	All MCs	102	1.0	102	1.0	0.238	50.9	LOS D	5.5	38.9	0.86	0.72	0.86	6.5
12	R2	All MCs	101	3.1	101	3.1	0.433	59.2	LOS E	6.1	43.7	0.94	0.79	0.94	5.1
Appro	Approach		358	2.4	358	2.4	0.505	58.2	LOS E	9.3	66.8	0.92	0.77	0.92	5.4
All Ve	hicles	;	3229	4.8	3229	4.8	0.579	13.3	LOS A	12.6	91.7	0.31	0.30	0.31	23.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Critical Movement (Signal Timing)

Pe	destrian Mov	vement	Perforr	nance									
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed		
		ped/h	sec		l Pea ped	Dist j m		Rate	sec	m	m/sec		
SouthEast: Pacific Hwy (SE)													
P1	Full	1	55.1	LOS E	0.0	0.0	0.90	0.90	71.8	20.0	0.28		
Nor	thEast: Oxley	St (NE)											

P2 Full	1001	57.5	LOS E	3.6	3.6	0.94	0.94	74.2	20.0	0.27		
NorthWest: Pacific Hwy (NW)												
P3 Full	376	56.0	LOS E	1.3	1.3	0.92	0.92	72.7	20.0	0.28		
SouthWest: Oxley St (SW)												
P4 Full	227	55.7	LOS E	0.8	0.8	0.91	0.91	72.3	20.0	0.28		
All Pedestrians	1605	56.9	LOS E	3.6	3.6	0.93	0.93	73.6	20.0	0.27		

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Site: CST03 [CST03 Pacific Hwy / Hume St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 766

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 135 seconds (Network Site User-Given Phase Times)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand lows HV] %	Ar Fl [Total] veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
SouthEast: Pacific Hwy (SE)															
1	L2	All MCs	31	3.4	31	3.4	0.110	6.1	LOS A	0.2	1.6	0.03	0.14	0.03	36.4
2	T1	All MCs	1320	5.1	1320	5.1	*0.433	0.5	LOS A	1.4	10.2	0.04	0.04	0.04	57.8
Appro	ach		1351	5.1	1351	5.1	0.433	0.7	LOS A	1.4	10.2	0.04	0.05	0.04	56.8
NorthWest: Pacific Hwy (NW)															
8	T1	All MCs	1385	5.5	1385	5.5	0.334	3.2	LOS A	10.1	73.7	0.21	0.19	0.21	42.0
Appro	ach		1385	5.5	1385	5.5	0.334	3.2	LOS A	10.1	73.7	0.21	0.19	0.21	42.0
SouthWest: Hume St (SW)															
10	L2	All MCs	101	2.1	101	2.1	*0.501	67.2	LOS E	6.5	46.0	0.99	0.78	0.99	4.2
12	R2	All MCs	38	0.0	38	0.0	0.138	58.8	LOS E	2.2	15.3	0.90	0.73	0.90	4.8
Appro	ach		139	1.5	139	1.5	0.501	64.9	LOS E	6.5	46.0	0.96	0.77	0.96	4.4
All Ve	hicles		2875	5.1	2875	5.1	0.501	5.0	LOS A	10.1	73.7	0.17	0.15	0.17	39.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance												
Mo	·	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.	
ID	Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	Time	Dist.	Speed	
		ped/h	sec		ped	m			sec	m	m/sec	
Sou	SouthEast: Pacific Hwy (SE)											
P1	Full	1	55.1	LOS E	0.0	0.0	0.90	0.90	71.8	20.0	0.28	
NorthWest: Pacific Hwy (NW)												
P3	Full	213	55.6	LOS E	0.7	0.7	0.91	0.91	72.3	20.0	0.28	
SouthWest: Hume St (SW)												
P4	Full	96	58.1	LOS E	0.3	0.3	0.93	0.93	74.8	20.0	0.27	
All F	Pedestrians	309	56.4	LOS E	0.7	0.7	0.92	0.92	73.1	20.0	0.27	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST04 [CST04 Pacific Hwy / Falcon St / Shirley Rd (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 765

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 135 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Total	IOWS	H I Total	IOWS	Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		Trate	Cycles	km/h
South	East:	Pacific H	wy (SE)											
1	L2	All MCs	298	4.9	298	4.9	0.256	15.4	LOS B	7.8	57.0	0.44	0.70	0.44	30.5
2	T1	All MCs	885	5.9	885	5.9	0.638	34.7	LOS C	23.6	173.9	0.86	0.76	0.86	11.5
Appro	ach		1183	5.7	1183	5.7	0.638	29.9	LOS C	23.6	173.9	0.75	0.74	0.75	16.1
East:	Falco	n St (E)													
21b	L3	All MCs	7	0.0	7	0.0	0.921	33.3	LOS C	18.1	130.6	1.00	0.99	1.19	5.0
21a	L1	All MCs	243	3.9	243	3.9	*0.921	67.9	LOS E	18.1	130.6	1.00	0.99	1.19	12.4
23a	R1	All MCs	440	3.3	440	3.3	0.921	58.5	LOS E	18.1	130.6	1.00	0.97	1.16	5.4
Appro	ach		691	3.5	691	3.5	0.921	61.6	LOS E	18.1	130.6	1.00	0.98	1.17	8.4
North:	Willo	ughby Rd	l (N)												
7	L2	All MCs	47	2.2	47	2.2	0.035	3.8	LOS A	0.2	1.6	0.08	0.48	0.08	37.0
Appro	ach		47	2.2	47	2.2	0.035	3.8	LOS A	0.2	1.6	0.08	0.48	0.08	37.0
North	West:	Pacific H	wy (NV	V)											
7a	L1	All MCs	398	6.6	398	6.6	0.501	20.3	LOS B	11.8	87.3	0.78	0.80	0.78	22.7
8	T1	All MCs	1025	4.9	1025	4.9	*0.730	22.8	LOS B	23.8	173.8	0.75	0.67	0.75	21.5
Appro	ach		1423	5.4	1423	5.4	0.730	22.1	LOS B	23.8	173.8	0.76	0.71	0.76	21.8
South	West	Shirley R	Rd (SW)											
10	L2	All MCs	25	4.2	25	4.2	0.928	85.2	LOS F	29.4	210.9	1.00	1.11	1.29	9.0
12a	R1	All MCs	440	2.6	440	2.6	*0.928	76.4	LOS F	29.7	211.7	1.00	1.10	1.29	8.9
12	R2	All MCs	296	1.8	296	1.8	0.928	78.5	LOS F	29.7	211.7	1.00	1.07	1.28	8.7
Appro	ach		761	2.4	761	2.4	0.928	77.5	LOS F	29.7	211.7	1.00	1.09	1.28	8.8
All Ve	hicles		4105	4.6	4105	4.6	0.928	41.0	LOS C	29.7	211.7	0.83	0.83	0.92	13.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE E QUEL [Ped	BACK OF JE Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed

	ped/h	sec		ped	m			sec	m	m/sec
SouthEast: Pacific	c Hwy (S	E)								
P1 Full	125	54.5	LOS E	0.4	0.4	0.90	0.90	71.2	20.0	0.28
East: Falcon St (E)									
P5 Full	295	58.6	LOS E	1.1	1.1	0.94	0.94	75.3	20.0	0.27
NorthWest: Pacifi	c Hwy (N	IW)								
P3 Full	309	54.9	LOS E	1.1	1.1	0.91	0.91	71.6	20.0	0.28
SouthWest: Shirle	ey Rd (SV	N)								
P4 Full	16	57.9	LOS E	0.1	0.1	0.93	0.93	74.6	20.0	0.27
All Pedestrians	745	56.4	LOS E	1.1	1.1	0.92	0.92	73.0	20.0	0.27

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V Site: CST05 [CST05 Clarke St / Oxley St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fi [Total veb/b	nand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh. veh	c Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke S	t (SE)	70	VOII/II			000		Volt					
1 3a	L2 R1	All MCs All MCs	23 59	4.5 3.6	23 59	4.5 3.6	0.085 0.085	5.2 5.9	LOS A LOS A	0.3 0.3	2.1 2.1	0.36 0.36	0.58 0.58	0.36 0.36	32.2 32.2
Appro	ach		82	3.8	82	3.8	0.085	5.7	LOSA	0.3	2.1	0.36	0.58	0.36	32.2
North:	Oxle	y St (N)													
24a	L1	All MCs	107	1.0	107	1.0	0.147	4.4	LOS A	0.0	0.0	0.00	0.53	0.00	29.6
26a	R1	All MCs	167	3.1	167	3.1	0.147	4.1	LOS A	0.0	0.0	0.00	0.53	0.00	29.6
Appro	ach		275	2.3	275	2.3	0.147	4.2	NA	0.0	0.0	0.00	0.53	0.00	29.6
South	West:	Oxley St	t (SW)												
10a	L1	All MCs	114	0.9	114	0.9	0.107	2.8	LOS A	0.4	3.0	0.28	0.54	0.28	22.4
12	R2	All MCs	65	0.0	65	0.0	0.107	4.8	LOS A	0.4	3.0	0.28	0.54	0.28	22.4
Appro	ach		179	0.6	179	0.6	0.107	3.5	NA	0.4	3.0	0.28	0.54	0.28	22.4
All Ve	hicles		536	2.0	536	2.0	0.147	4.2	NA	0.4	3.0	0.15	0.54	0.15	28.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST06 [CST06 Clarke St / Hume St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	orma	ince										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	c Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke S	t (SE)												
21	L2	All MCs	1	100. 0	1	100. 0	0.054	4.0	LOS A	0.1	0.8	0.10	0.11	0.10	19.2
2	T1	All MCs	81	3.9	81	3.9	0.054	0.0	LOS A	0.1	0.8	0.10	0.11	0.10	36.8
23a	R1	All MCs	16	0.0	16	0.0	0.054	3.7	LOS A	0.1	0.8	0.10	0.11	0.10	37.5
Appro	bach		98	4.3	98	4.3	0.054	0.6	NA	0.1	0.8	0.10	0.11	0.10	36.6
North	West:	Clarke S	t (NW)												
27b	L3	All MCs	4	0.0	4	0.0	0.090	4.1	LOS A	0.0	0.2	0.01	0.02	0.01	38.3
8	T1	All MCs	165	0.6	165	0.6	0.090	0.0	LOS A	0.0	0.2	0.01	0.02	0.01	39.6
29	R2	All MCs	3	0.0	3	0.0	0.090	4.0	LOS A	0.0	0.2	0.01	0.02	0.01	20.4
Appro	bach		173	0.6	173	0.6	0.090	0.2	NA	0.0	0.2	0.01	0.02	0.01	39.1
All Ve	hicles		271	1.9	271	1.9	0.090	0.3	NA	0.1	0.8	0.04	0.06	0.04	38.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST07 [CST07 Clarke St / Willoughby Rd (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV] veh/h %	Arrival Flows [Total HV] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% E Qu [Veh. veh	Back Of ieue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Willo	oughby R	d (S)										
1	L2	All MCs	108 2.9	108 2.9	0.177	5.2	LOS A	0.8	6.1	0.43	0.47	0.43	29.3
2	T1	All MCs	55 28.8	55 28.8	0.177	1.8	LOS A	0.8	6.1	0.43	0.47	0.43	34.5
Appro	ach		163 11.6	163 11.6	0.177	4.1	NA	0.8	6.1	0.43	0.47	0.43	31.6
North:	Willo	ughby Ro	d (N)										
8	T1	All MCs	21 60.0	21 60.0	0.065	2.1	LOS A	0.2	2.1	0.42	0.44	0.42	32.7
9	R2	All MCs	23 9.1	23 9.1	0.065	8.1	LOS A	0.2	2.1	0.42	0.44	0.42	32.4
Appro	ach		44 33.3	44 33.3	0.065	5.3	NA	0.2	2.1	0.42	0.44	0.42	32.5
West:	Clark	e St (W)											
10	L2	All MCs	33 0.0	33 0.0	0.154	6.3	LOS A	0.5	3.8	0.50	0.70	0.50	32.4
12	R2	All MCs	87 2.4	87 2.4	0.154	6.3	LOS A	0.5	3.8	0.50	0.70	0.50	26.2
Appro	ach		120 1.8	120 1.8	0.154	6.3	LOS A	0.5	3.8	0.50	0.70	0.50	28.6
All Ve	hicles		327 10.9	327 10.9	0.177	5.0	NA	0.8	6.1	0.45	0.55	0.45	30.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST08 [CST08 Albany St / Willoughby Rd (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 516

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 65 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Arriv	val	Deg.	Aver.	Level of	95%	Back Of	Prop.	Eff.	Aver.	Aver.
TD		Class	FI [Total	OWS HV/1	Flo ^v	WS \/1_	Satn	Delay	Service	Q [\/eb	ueue Dist-1	Que	Stop Rate	No. of	Speed
			veh/h	%	veh/h	v] %	v/c	sec		veh	m		Nate	Cycles	km/h
South	: Willo	oughby Ro	d (S)												
1	L2	All MCs	7	100. 0	7 10	00. 0	0.038	26.7	LOS B	0.2	2.5	0.82	0.64	0.82	22.8
2	T1	All MCs	8	100. 0	₈ 10	00. 0	0.035	20.1	LOS B	0.2	2.8	0.75	0.54	0.75	27.5
3	R2	All MCs	1	0.0	1 (0.0	0.035	24.8	LOS B	0.2	2.8	0.75	0.54	0.75	26.1
Appro	ach		17 9	93.8	17 93	3.8	0.038	23.3	LOS B	0.2	2.8	0.78	0.59	0.78	25.2
East:	Alban	y St (E)													
4	L2	All MCs	1	0.0	1 (0.0	0.261	13.1	LOS A	4.6	32.5	0.57	0.48	0.57	29.3
5	T1	All MCs	368	1.4	368 2	1.4	0.521	9.4	LOS A	6.6	46.9	0.65	0.56	0.65	33.5
6	R2	All MCs	177	2.4	177 2	2.4	* 0.521	21.8	LOS B	6.6	46.9	0.85	0.76	0.85	27.5
Appro	ach		546	1.7	546 ´	1.7	0.521	13.5	LOS A	6.6	46.9	0.71	0.63	0.71	31.2
North	Willo	ughby Ro	1 (N)												
7	L2	All MCs	206	0.5	206 ().5	0.242	16.0	LOS B	4.0	28.0	0.64	0.72	0.64	28.7
8	T1	All MCs	7	100. 0	7 10	00. 0	*0.270	21.4	LOS B	2.4	18.0	0.86	0.75	0.86	24.2
9	R2	All MCs	80	1.3	80 -	1.3	0.270	28.0	LOS B	2.4	18.0	0.86	0.75	0.86	22.6
Appro	ach		294	3.2	294 3	3.2	0.270	19.4	LOS B	4.0	28.0	0.71	0.73	0.71	26.6
West:	Albar	ny St (W)													
10	L2	All MCs	132	2.4	132 2	2.4	0.236	23.0	LOS B	3.2	22.7	0.79	0.75	0.79	24.6
11	T1	All MCs	309	3.1	309 3	3.1	*0.462	17.8	LOS B	7.7	55.5	0.82	0.70	0.82	28.1
Appro	ach		441	2.9	441 2	2.9	0.462	19.3	LOS B	7.7	55.5	0.81	0.71	0.81	26.9
All Ve	hicles		1298	3.6	1298 3	3.6	0.521	16.9	LOS B	7.7	55.5	0.74	0.68	0.74	28.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian	Movem	ent Perf	ormano	e							
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QUI	EUE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Willoug	ghby Rd	(S)									
P1 Full	133	140	26.0	LOS C	0.2	0.2	0.90	0.90	42.7	20.0	0.47

East: Albany S	t (E)										
P2 Full	151	159	26.0	LOS C	0.3	0.3	0.90	0.90	42.7	20.0	0.47
North: Willough	nby Rd (N))									
P3 Full	78	82	26.0	LOS C	0.1	0.1	0.90	0.90	42.6	20.0	0.47
West: Albany S	St (W)										
P4 Full	157	165	26.1	LOS C	0.3	0.3	0.90	0.90	42.7	20.0	0.47
All Pedestrians	519	546	26.0	LOS C	0.3	0.3	0.90	0.90	42.7	20.0	0.47

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V Site: CST09 [CST09 Albany St / Oxley St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

NA Site Category: (None) Roundabout

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Intel I	lows 山い 1	H Intal	lows 山\/ 1	Satn	Delay	Service	[\/eh	Diet 1	Que	Stop Rate	NO. Of	Speed
			veh/h	· · v] %	veh/h	%	v/c	sec		veh	m		Tale	Cycles	km/h
South	: Oxle	ey St (S)													
1	L2	All MCs	44	7.1	44	7.1	0.236	8.0	LOS A	1.6	11.2	0.70	0.66	0.70	21.7
2	T1	All MCs	63	3.3	63	3.3	0.236	7.6	LOS A	1.6	11.2	0.70	0.66	0.70	32.7
3	R2	All MCs	67	0.0	67	0.0	0.236	10.4	LOS A	1.6	11.2	0.70	0.66	0.70	30.9
3u	U	All MCs	1	0.0	1	0.0	0.236	11.8	LOS A	1.6	11.2	0.70	0.66	0.70	21.7
Appro	ach		176	3.0	176	3.0	0.236	8.8	LOS A	1.6	11.2	0.70	0.66	0.70	30.2
East: /	Alban	y St (E)													
4	L2	All MCs	52	2.0	52	2.0	0.528	7.9	LOSA	3.7	26.4	0.61	0.71	0.65	30.4
5	T1	All MCs	328	3.5	328	3.5	0.528	7.7	LOSA	3.7	26.4	0.61	0.71	0.65	30.4
6	R2	All MCs	38	2.8	38	2.8	0.528	10.6	LOSA	3.7	26.4	0.61	0.71	0.65	35.5
6u	U	All MCs	1	0.0	1	0.0	0.528	11.9	LOSA	3.7	26.4	0.61	0.71	0.65	34.9
Appro	ach		419	3.3	419	3.3	0.528	8.0	LOS A	3.7	26.4	0.61	0.71	0.65	31.2
North:	Oxle	v St (N)													
7	12		24	0.0	24	0.0	0.356	8.0	LOSA	24	17.2	0 74	0.68	0 74	35.1
, 8	T1		157	13	157	13	0.000	7.0		2.4	17.2	0.74	0.00	0.74	30.6
g	R2		Q1	0.0	Q1	0.0	0.356	10.7		2.4	17.2	0.74	0.00	0.74	30.6
Qu	11		1	0.0	1	0.0	0.356	10.7		2.4	17.2	0.74	0.00	0.74	35.3
Annro	ach	7 11 11/00	273	0.0	273	0.8	0.356	8.8	LOSA	2.4	17.2	0.74	0.68	0.74	31.3
7.000	uon		210	0.0	210	0.0	0.000	0.0	LOOM	2.7	17.2	0.74	0.00	0.14	01.0
West:	Albar	ny St (W)													
10	L2	All MCs	92	3.4	92	3.4	0.479	5.7	LOS A	3.9	27.6	0.59	0.53	0.59	35.2
11	T1	All MCs	313	1.0	313	1.0	0.479	5.5	LOS A	3.9	27.6	0.59	0.53	0.59	35.1
12	R2	All MCs	78	2.7	78	2.7	0.479	8.4	LOS A	3.9	27.6	0.59	0.53	0.59	27.2
12u	U	All MCs	1	0.0	1	0.0	0.479	9.7	LOS A	3.9	27.6	0.59	0.53	0.59	27.2
Appro	ach		483	1.7	483	1.7	0.479	6.0	LOS A	3.9	27.6	0.59	0.53	0.59	34.4
All Ve	hicles		1351	2.2	1351	2.2	0.528	7.6	LOS A	3.9	27.6	0.64	0.63	0.65	32.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST10 [CST10 Albany St / Clarke Ln (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Give-Way (Two-Way)

Vehio	cle M	ovemen	t Perfo	orma	ince										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Bacł [Veh. veh	COf Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke Li	n (SE)												
21a	L1	All MCs	22	0.0	22	0.0	0.062	4.2	LOS A	3.9	27.5	0.22	0.50	0.22	29.5
23b	R3	All MCs	8	0.0	8	0.0	0.062	13.6	LOS A	3.9	27.5	0.22	0.50	0.22	29.5
Appro	ach		31	0.0	31	0.0	0.062	6.8	LOS A	3.9	27.5	0.22	0.50	0.22	29.5
East:	Alban	y St (E)													
5	T1	All MCs	469	3.1	469	3.1	0.226	0.0	LOS A	9.2	65.9	0.00	0.00	0.00	49.9
Appro	ach		469	3.1	469	3.1	0.226	0.0	NA	9.2	65.9	0.00	0.00	0.00	49.9
West:	Albar	ny St (W)													
11	T1	All MCs	480	1.8	480	1.8	0.250	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	49.9
Appro	ach		480	1.8	480	1.8	0.250	0.0	NA	0.0	0.0	0.00	0.00	0.00	49.9
All Ve	hicles		980	2.4	980	2.4	0.250	0.2	NA	9.2	65.9	0.01	0.02	0.01	48.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST11 [CST11 Oxley St / Clarke Ln (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

NA Site Category: (None) Stop (Two-Way)

Vehio	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand lows	Arri Flo	ival ows	Deg. Satn	Aver. Delay	Level of Service	95% Bad	ck Of Queu	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total H	IV]				[Veh.	Dist]		Rate	Cycles	km/b
South	East:	Clarke Li	n (SE)	70	ven/n	70	V/C	sec	_	ven	111	_	_	_	K[1]/11
1	12	All MCs	1	0.0	1	0.0	0.007	7.7	LOSA	0.0	0.2	0.36	0.84	0.36	26.5
2	 T1	All MCs	1	0.0	1	0.0	0.007	8.9	LOSA	0.0	0.2	0.36	0.84	0.36	26.5
3	R2	All MCs	2	50.0	25	0.0	0.007	12.1	LOS A	0.0	0.2	0.36	0.84	0.36	26.5
Appro	ach		4	25.0	42	5.0	0.007	10.2	LOS A	0.0	0.2	0.36	0.84	0.36	26.5
North	East:	Oxley St	(NE)												
5	T1	All MCs	184	3.4	184	3.4	0.170	0.0	LOS A	0.0	0.3	0.02	0.02	0.02	47.8
6	R2	All MCs	5	0.0	5	0.0	0.170	3.5	LOS A	0.0	0.3	0.02	0.02	0.02	47.8
Appro	ach		189	3.3	189	3.3	0.170	0.1	NA	0.0	0.3	0.02	0.02	0.02	47.8
North	West:	Clarke L	n (NW)												
7	L2	All MCs	17	0.0	17	0.0	0.023	5.0	LOS A	0.1	0.5	0.28	0.52	0.28	25.2
9	R2	All MCs	6	0.0	6	0.0	0.023	6.3	LOS A	0.1	0.5	0.28	0.52	0.28	25.2
Appro	ach		23	0.0	23	0.0	0.023	5.3	LOS A	0.1	0.5	0.28	0.52	0.28	25.2
South	West	Oxley St	: (SW)												
10	L2	All MCs	11	0.0	11	0.0	0.088	2.8	LOS A	0.0	0.0	0.00	0.03	0.00	46.8
11	T1	All MCs	160	0.0	160	0.0	0.088	0.0	LOS A	0.0	0.0	0.00	0.03	0.00	46.8
Appro	ach		171	0.0	171	0.0	0.088	0.2	NA	0.0	0.0	0.00	0.03	0.00	46.8
All Ve	hicles		387	1.9	387	1.9	0.170	0.6	NA	0.1	0.5	0.03	0.07	0.03	41.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST13 [CST13 Pacific Hwy / Alexander St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 763

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 135 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	hand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
Cauth	C. a. t.	Desifie Ll	veh/h	% \	veh/h	%	V/C	sec	_	veh	m	_	_	_	km/h
South	East:	Pacific H	wy (SE)											
2	T1	All MCs	1087	5.6	1087	5.6	0.462	6.9	LOS A	10.2	74.9	0.55	0.49	0.55	38.2
3a	R1	All MCs	303	6.6	303	6.6	0.545	19.7	LOS B	8.5	62.6	0.65	0.77	0.65	23.4
Appro	ach		1391	5.8	1391	5.8	0.545	9.7	LOS A	10.2	74.9	0.57	0.55	0.57	33.4
North:	Alexa	ander St (N)												
24a	L1	All MCs	275	5.4	275	5.4	* 0.500	35.1	LOS C	12.0	88.1	1.00	0.81	1.00	18.5
26b	R3	All MCs	96	6.6	96	6.6	0.707	76.2	LOS F	6.6	48.9	1.00	0.85	1.09	4.6
Appro	ach		371	5.7	371	5.7	0.707	45.7	LOS D	12.0	88.1	1.00	0.82	1.02	13.7
North	Nest:	Pacific H	wy (NV	V)											
7b	L3	All MCs	7	0.0	7	0.0	0.089	13.8	LOS A	0.1	1.1	0.05	0.14	0.05	48.7
8	T1	All MCs	1319	4.2	1319	4.2	*0.679	12.0	LOS A	17.8	126.1	0.50	0.50	0.50	41.2
Appro	ach		1326	4.2	1326	4.2	0.679	12.0	LOS A	17.8	126.1	0.49	0.50	0.49	41.2
All Ve	hicles		3087	5.1	3087	5.1	0.707	15.0	LOS B	17.8	126.1	0.59	0.56	0.59	31.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perforr	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
North: Alexander	St (N)									
P6 Full	161	22.7	LOS C	0.3	0.3	0.81	0.81	39.4	20.0	0.51
NorthWest: Pacifi	c Hwy (N	W)								
P3 Full	61	55.3	LOS E	0.2	0.2	0.91	0.91	71.9	20.0	0.28
All Pedestrians	222	31.6	LOS D	0.3	0.3	0.84	0.84	48.3	20.0	0.41

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST14 [CST14 Falcon St / Alexander St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 764

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 135 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	hand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	٦ [Total]	HV]	ا ت Total I]	HV]	Saur	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	Alex	ander St	(S)												
1	L2	All MCs	1	100. 0	1	100. 0	*0.475	75.4	LOS F	13.4	98.9	1.00	0.86	1.00	5.3
2	T1	All MCs	242	6.1	242	6.1	0.528	61.0	LOS E	13.4	98.9	1.00	0.86	1.00	9.0
3	R2	All MCs	67	6.3	67	6.3	0.528	77.3	LOS F	7.2	52.7	1.00	0.84	1.00	17.2
Appro	ach		311	6.4	311	6.4	0.528	64.6	LOS E	13.4	98.9	1.00	0.85	1.00	11.2
East:	Falco	n St (E)													
4	L2	All MCs	28	0.0	28	0.0	0.429	20.2	LOS B	12.5	90.1	0.51	0.47	0.51	37.4
5	T1	All MCs	712	3.8	712	3.8	0.429	14.6	LOS B	12.5	90.1	0.51	0.46	0.51	37.7
6	R2	All MCs	12	63.6	126	63.6	0.429	20.6	LOS B	10.6	77.6	0.50	0.45	0.50	37.6
Appro	ach		752	4.6	752	4.6	0.429	14.9	LOS B	12.5	90.1	0.51	0.46	0.51	37.7
North:	Alexa	ander St (N)												
7	L2	All MCs	29	0.0	29	0.0	0.454	54.6	LOS D	10.3	74.9	0.91	0.76	0.91	22.0
8	T1	All MCs	349	6.0	349	6.0	0.454	47.4	LOS D	11.2	82.3	0.91	0.76	0.91	6.9
Appro	ach		379	5.6	379	5.6	0.454	47.9	LOS D	11.2	82.3	0.91	0.76	0.91	8.6
West:	Falco	on St (W)													
10	L2	All MCs	143	3.7	143	3.7	*0.364	10.1	LOS A	5.9	42.6	0.23	0.38	0.23	32.0
11	T1	All MCs	749	4.5	749	4.5	0.364	3.1	LOS A	5.9	42.6	0.19	0.24	0.19	52.6
Appro	ach		893	4.4	893	4.4	0.364	4.2	LOS A	5.9	42.6	0.19	0.26	0.19	50.4
All Ve	hicles		2334	4.9	2334	4.9	0.528	22.8	LOS B	13.4	98.9	0.52	0.48	0.52	28.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	lovement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	nod/h	-		[Ped	Dist]		Rate	200	m	mlsoc
South: Alexand	der St (S)	Sec	_	ped		_	_	580	111	m/sec
P1 Full	85	57.2	LOS E	0.3	0.3	0.92	0.92	73.8	20.0	0.27

East: Falcon St (E))									
P2 Full	45	57.1	LOS E	0.2	0.2	0.92	0.92	73.7	20.0	0.27
North: Alexander S	St (N)									
P3 Full	105	57.2	LOS E	0.4	0.4	0.92	0.92	73.9	20.0	0.27
West: Falcon St (W	V)									
P4 Full	226	57.5	LOS E	0.8	0.8	0.93	0.93	74.2	20.0	0.27
All Pedestrians	462	57.3	LOS E	0.8	0.8	0.92	0.92	74.0	20.0	0.27

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Site: CST01 [CST01 Pacific Hwy / Albany St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 768

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar Fl [Total] veh/h	rival ows HV] %	Deg. Satn	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Pacific H	wy (SE)											
22	T1	All MCs	1154	4.1	1154	4.1	0.447	12.0	LOS A	19.4	140.3	0.55	0.43	0.55	38.2
23b	R3	All MCs	135	0.0	135	0.0	*0.864	80.5	LOS F	9.0	62.7	1.00	0.91	1.19	6.5
Appro	ach		1288	3.7	1288	3.7	0.864	19.1	LOS B	19.4	140.3	0.60	0.48	0.62	29.9
East:	Alban	y St (E)													
4b	L3	All MCs	26	4.0	26	4.0	* 0.738	67.7	LOS E	6.9	49.0	1.00	0.87	1.07	2.7
6a	R1	All MCs	472	2.2	472	2.2	0.738	55.8	LOS D	6.9	49.0	1.00	0.87	1.06	10.1
Appro	ach		498	2.3	498	2.3	0.738	56.4	LOS D	6.9	49.0	1.00	0.87	1.06	9.8
North	Nest:	Pacific H	wy (NV	V)											
27a	L1	All MCs	393	0.0	393	0.0	0.419	14.8	LOS B	10.3	72.2	0.44	0.68	0.44	24.1
28	T1	All MCs	912	3.6	912	3.6	*0.419	9.0	LOS A	10.3	72.2	0.40	0.36	0.40	31.6
Appro	ach		1304	2.5	1304	2.5	0.419	10.7	LOS A	10.3	72.2	0.41	0.46	0.41	28.9
All Ve	hicles		3091	3.0	3091	3.0	0.864	21.6	LOS B	19.4	140.3	0.58	0.53	0.60	23.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	ement	Perforr	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
SouthEast: Pacific	Hwy (S	E)								
P5 Full	278	50.8	LOS E	0.9	0.9	0.91	0.91	217.4	200.0	0.92
East: Albany St (E)									
P2 Full	443	51.1	LOS E	1.4	1.4	0.91	0.91	67.8	20.0	0.30
All Pedestrians	721	51.0	LOS E	1.4	1.4	0.91	0.91	125.5	89.4	0.71

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST02 [CST02 Pacific Hwy / Oxley St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 767

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Den F	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	NIC	500		[Veh.	Dist]		Rate	Cycles	km/b
South	East:	Pacific H	wy (SE)		/0	v/C	360		VEIT	111	_	_		KI11/11
1	L2	All MCs	143	, 0.0	143	0.0	0.137	7.6	LOS A	0.9	6.0	0.10	0.56	0.10	24.1
2	T1	All MCs	1104	4.2	1104	4.2	*0.447	3.3	LOS A	7.4	53.4	0.23	0.21	0.23	41.5
Appro	ach		1247	3.7	1247	3.7	0.447	3.8	LOS A	7.4	53.4	0.21	0.25	0.21	37.0
North	East:	Oxley St	(NE)												
4	L2	All MCs	79	0.0	79	0.0	0.204	46.7	LOS D	4.0	28.0	0.87	0.74	0.87	2.9
5	T1	All MCs	97	0.0	97	0.0	0.239	44.5	LOS D	4.9	34.5	0.88	0.69	0.88	8.2
Appro	ach		176	0.0	176	0.0	0.239	45.5	LOS D	4.9	34.5	0.87	0.71	0.87	5.9
North	West:	Pacific H	wy (NV	V)											
7	L2	All MCs	69	0.0	69	0.0	0.252	25.0	LOS B	9.5	67.9	0.58	0.50	0.58	22.8
8	T1	All MCs	868	3.9	868	3.9	0.252	11.0	LOS A	9.5	69.0	0.55	0.41	0.55	26.1
Appro	ach		938	3.6	938	3.6	0.252	12.0	LOS A	9.5	69.0	0.55	0.42	0.55	25.8
South	West	: Oxley St	(SW)												
10	L2	All MCs	183	0.6	183	0.6	*0.754	70.8	LOS F	11.3	79.7	1.00	0.88	1.11	4.7
11	T1	All MCs	168	0.0	168	0.0	0.424	56.2	LOS D	9.0	62.9	0.92	0.77	0.92	6.5
12	R2	All MCs	139	0.0	139	0.0	0.602	58.1	LOS E	8.1	56.7	0.98	0.81	0.98	5.1
Appro	ach		491	0.2	491	0.2	0.754	62.2	LOS E	11.3	79.7	0.96	0.82	1.01	5.4
All Ve	hicles	;	2852	2.8	2852	2.8	0.754	19.1	LOS B	11.3	79.7	0.49	0.43	0.50	17.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Critical Movement (Signal Timing)

Peo	destrian Mov	/ement	Perfor	nance							
Mov	Crossing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
חו	Crossing	Flow	Delay	Service	QUE [Ped	:UE Dist 1	Que	Stop Rate	Time	Dist.	Speed
		ped/h	sec		ped	m		rtato	sec	m	m/sec
Sou	thEast: Pacifi	c Hwy (S	SE)								
P1	Full	1	50.2	LOS E	0.0	0.0	0.90	0.90	66.8	20.0	0.30
Nor	thEast: Oxley	St (NE)									

P2 Full	883	52.1	LOS E	2.9	2.9	0.93	0.93	68.8	20.0	0.29
NorthWest: Pacific	c Hwy (N	W)								
P3 Full	321	50.9	LOS E	1.0	1.0	0.91	0.91	67.5	20.0	0.30
SouthWest: Oxley	st (SW)									
P4 Full	213	50.6	LOS E	0.7	0.7	0.90	0.90	67.3	20.0	0.30
All Pedestrians	1418	51.6	LOS E	2.9	2.9	0.92	0.92	68.3	20.0	0.29

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Site: CST03 [CST03 Pacific Hwy / Hume St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 766

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Den F	nand Iows	Ar Fl	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV] %	[Total	HV]	v/c	880		[Veh.	Dist]		Rate	Cycles	km/h
South	East:	Pacific H	wy (SE)	VCH/H	70	V/C	300		VCIT		_	_		N11//11
1	L2	All MCs	51	0.0	51	0.0	0.294	11.7	LOS A	8.3	59.6	0.37	0.38	0.37	30.7
2	T1	All MCs	1153	4.0	1153	4.0	0.294	6.2	LOS A	8.3	59.6	0.37	0.35	0.37	39.9
Appro	ach		1203	3.8	1203	3.8	0.294	6.4	LOS A	8.3	59.8	0.37	0.35	0.37	39.3
North	East:	Hume St	(NE)												
24	L2	All MCs	19	0.0	19	0.0	0.139	55.5	LOS D	2.0	13.9	0.91	0.70	0.91	2.9
25	T1	All MCs	19	0.0	19	0.0	*0.139	48.7	LOS D	2.0	13.9	0.91	0.69	0.91	6.7
26	R2	All MCs	17	0.0	17	0.0	0.114	59.9	LOS E	1.0	6.8	0.94	0.69	0.94	2.4
Appro	ach		55	0.0	55	0.0	0.139	54.5	LOS D	2.0	13.9	0.92	0.69	0.92	4.1
North	Nest:	Pacific H	wy (NV	V)											
8	T1	All MCs	1086	3.1	1086	3.1	*0.359	6.5	LOS A	10.8	77.8	0.39	0.35	0.39	32.6
Appro	ach		1086	3.1	1086	3.1	0.359	6.5	LOS A	10.8	77.8	0.39	0.35	0.39	32.6
South	West	Hume St	t (SW)												
10	L2	All MCs	78	0.0	78	0.0	0.328	59.2	LOS E	4.4	31.0	0.95	0.76	0.95	4.7
31	T1	All MCs	1	0.0	1	0.0	0.011	66.0	LOS E	0.1	0.2	1.00	0.55	1.00	5.3
12	R2	All MCs	22	0.0	22	0.0	0.109	57.5	LOS E	1.2	8.5	0.92	0.71	0.92	5.0
Appro	ach		101	0.0	101	0.0	0.328	58.9	LOS E	4.4	31.0	0.94	0.75	0.94	4.8
All Ve	hicles		2445	3.3	2445	3.3	0.359	9.7	LOS A	10.8	77.8	0.42	0.37	0.42	30.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Critical Movement (Signal Timing)

Peo	destrian Mov	/ement	Perform	nance							
Mo		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	thEast: Pacifi	c Hwy (S	SE)								
P1	Full	1	50.2	LOS E	0.0	0.0	0.90	0.90	66.8	20.0	0.30
Nor	thEast: Hume	St (NE)									

P6 Full	286	53.5	LOS E	0.9	0.9	0.93	0.93	70.2	20.0	0.28
NorthWest: Pacific	Hwy (N	W)								
P3 Full	189	50.6	LOS E	0.6	0.6	0.90	0.90	67.2	20.0	0.30
SouthWest: Hume	St (SW)									
P4 Full	142	53.2	LOS E	0.5	0.5	0.93	0.93	69.9	20.0	0.29
All Pedestrians	619	52.6	LOS E	0.9	0.9	0.92	0.92	69.2	20.0	0.29

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Site: CST04 [CST04 Pacific Hwy / Falcon St / Shirley Rd (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 765

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Total	IOWS	H I Total	OWS	Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	" %	veh/h	%	v/c	sec		veh	m		Trate	Cycles	km/h
South	East:	Pacific H	wy (SE)											
1	L2	All MCs	352	1.5	352	1.5	0.359	18.6	LOS B	9.8	69.3	0.50	0.72	0.50	28.3
2	T1	All MCs	793	4.2	793	4.2	0.779	40.0	LOS C	22.2	161.3	0.94	0.85	0.98	10.2
Appro	ach		1144	3.4	1144	3.4	0.779	33.4	LOS C	22.2	161.3	0.80	0.81	0.83	15.4
East:	Falco	n St (E)													
21b	L3	All MCs	22	4.8	22	4.8	0.878	19.1	LOS B	18.4	130.6	0.94	0.93	1.04	7.4
21a	L1	All MCs	423	1.2	423	1.2	*0.878	44.6	LOS D	18.4	130.6	0.94	0.93	1.04	17.0
23a	R1	All MCs	336	3.4	336	3.4	0.599	31.3	LOS C	14.5	104.5	0.76	0.77	0.76	9.4
Appro	ach		781	2.3	781	2.3	0.878	38.1	LOS C	18.4	130.6	0.86	0.86	0.92	14.4
North:	Willo	ughby Ro	l (N)												
7	L2	All MCs	76	0.0	76	0.0	0.050	3.8	LOS A	0.4	2.6	0.09	0.49	0.09	36.9
Appro	ach		76	0.0	76	0.0	0.050	3.8	LOS A	0.4	2.6	0.09	0.49	0.09	36.9
North\	Nest:	Pacific H	wy (NV	V)											
7a	L1	All MCs	436	3.9	436	3.9	*0.593	20.2	LOS B	12.4	89.3	0.81	0.81	0.81	22.8
8	T1	All MCs	692	2.4	692	2.4	0.668	57.8	LOS E	20.9	149.2	1.00	0.89	1.00	10.8
Appro	ach		1127	3.0	1127	3.0	0.668	43.2	LOS D	20.9	149.2	0.93	0.86	0.93	13.5
South	West	Shirley R	Rd (SW)											
10	L2	All MCs	75	1.4	75	1.4	*0.534	53.3	LOS D	17.6	124.2	0.93	0.81	0.93	13.0
12a	R1	All MCs	455	1.2	455	1.2	0.534	45.6	LOS D	18.4	131.2	0.92	0.81	0.92	13.1
12	R2	All MCs	156	3.4	156	3.4	0.534	47.4	LOS D	18.4	131.2	0.92	0.81	0.92	13.2
Appro	ach		685	1.7	685	1.7	0.534	46.8	LOS D	18.4	131.2	0.92	0.81	0.92	13.1
All Ve	hicles	i.	3814	2.7	3814	2.7	0.878	39.1	LOS C	22.2	161.3	0.86	0.83	0.88	14.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE E QUEL [Ped	BACK OF JE Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed

	ped/h	sec		ped	m			sec	m	m/sec
SouthEa	ast: Pacific Hwy (SE)								
P1 Ful	96	49.5	LOS E	0.3	0.3	0.89	0.89	66.1	20.0	0.30
East: Fa	lcon St (E)									
P5 Ful	231	53.4	LOS E	0.8	0.8	0.93	0.93	70.1	20.0	0.29
NorthWe	est: Pacific Hwy (NW)								
P3 Ful	379	50.1	LOS E	1.2	1.2	0.90	0.90	66.7	20.0	0.30
SouthW	est: Shirley Rd (S	SW)								
P4 Ful	149	53.2	LOS E	0.5	0.5	0.93	0.93	69.9	20.0	0.29
All Pede	strians 855	51.5	LOS E	1.2	1.2	0.91	0.91	68.1	20.0	0.29

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V Site: CST05 [CST05 Clarke St / Oxley St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Bacl [Veh.	k Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	East:	Clarke S	t (SE)	/0	Ven/m	/0	v/C	360	_	ven		_	_	_	NIII/11
1	L2	All MCs	49	0.0	49	0.0	0.068	4.9	LOS A	0.2	1.7	0.27	0.53	0.27	32.7
3a	R1	All MCs	29	0.0	29	0.0	0.068	5.9	LOS A	0.2	1.7	0.27	0.53	0.27	32.7
Appro	ach		79	0.0	79	0.0	0.068	5.3	LOS A	0.2	1.7	0.27	0.53	0.27	32.7
North:	Oxle	y St (N)													
24a	L1	All MCs	137	0.0	137	0.0	0.136	4.4	LOS A	0.0	0.0	0.00	0.53	0.00	29.5
26a	R1	All MCs	123	0.0	123	0.0	0.136	4.1	LOS A	0.0	0.0	0.00	0.53	0.00	29.5
Appro	ach		260	0.0	260	0.0	0.136	4.3	NA	0.0	0.0	0.00	0.53	0.00	29.5
South	West	Oxley St	t (SW)												
10a	L1	All MCs	176	0.0	176	0.0	0.139	2.8	LOS A	0.5	3.3	0.22	0.54	0.22	23.0
12	R2	All MCs	66	0.0	66	0.0	0.139	4.9	LOS A	0.5	3.3	0.22	0.54	0.22	23.0
Appro	ach		242	0.0	242	0.0	0.139	3.3	NA	0.5	3.3	0.22	0.54	0.22	23.0
All Ve	hicles		581	0.0	581	0.0	0.139	4.0	NA	0.5	3.3	0.13	0.53	0.13	28.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST06 [CST06 Clarke St / Hume St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	orma	ince										
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	c Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke S	t (SE)												
1	L2	All MCs	29	0.0	29	0.0	0.201	5.5	LOS A	0.9	6.3	0.58	0.54	0.58	25.8
2	T1	All MCs	78	0.0	78	0.0	0.201	6.5	LOS A	0.9	6.3	0.58	0.54	0.58	25.8
23a	R1	All MCs	31	0.0	31	0.0	0.201	3.2	LOS A	0.9	6.3	0.58	0.54	0.58	32.3
Appro	bach		138	0.0	138	0.0	0.201	5.5	NA	0.9	6.3	0.58	0.54	0.58	28.2
North	West:	Clarke S	t (NW)												
27b	L3	All MCs	8	0.0	8	0.0	0.413	9.4	LOS A	2.4	16.7	0.73	0.85	1.00	29.1
8	T1	All MCs	177	0.0	177	0.0	0.413	8.7	LOS A	2.4	16.7	0.73	0.85	1.00	27.5
9	R2	All MCs	18	0.0	18	0.0	0.413	21.1	LOS B	2.4	16.7	0.73	0.85	1.00	22.5
Appro	bach		203	0.0	203	0.0	0.413	9.9	NA	2.4	16.7	0.73	0.85	1.00	27.3
All Ve	hicles	;	341	0.0	341	0.0	0.413	8.1	NA	2.4	16.7	0.67	0.73	0.83	27.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST07 [CST07 Clarke St / Willoughby Rd (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Q [Veh. veh	Back Of ueue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Willc	ughby R	d (S)												
1	L2	All MCs	95	0.0	95	0.0	0.192	5.6	LOS A	0.9	6.7	0.37	0.37	0.37	30.6
2	T1	All MCs	91	12.8	91	12.8	0.192	1.0	LOS A	0.9	6.7	0.37	0.37	0.37	35.5
Appro	ach		185	6.2	185	6.2	0.192	3.3	NA	0.9	6.7	0.37	0.37	0.37	33.5
North:	Willo	ughby Ro	d (N)												
8	T1	All MCs	118	4.5	118	4.5	0.176	0.9	LOS A	0.7	5.4	0.31	0.27	0.31	36.0
9	R2	All MCs	43	2.4	43	2.4	0.176	8.5	LOS A	0.7	5.4	0.31	0.27	0.31	35.1
Appro	ach		161	3.9	161	3.9	0.176	2.9	NA	0.7	5.4	0.31	0.27	0.31	35.8
West:	Clark	e St (W)													
10	L2	All MCs	85	0.0	85	0.0	0.233	6.8	LOS A	0.9	6.1	0.57	0.78	0.59	31.6
12	R2	All MCs	82	0.0	82	0.0	0.233	7.7	LOS A	0.9	6.1	0.57	0.78	0.59	25.2
Appro	ach		167	0.0	167	0.0	0.233	7.2	LOS A	0.9	6.1	0.57	0.78	0.59	29.3
All Ve	hicles		514	3.5	514	3.5	0.233	4.5	NA	0.9	6.7	0.42	0.47	0.43	32.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST08 [CST08 Albany St / Willoughby Rd (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 516

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 70 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95%	Back Of	Prop.	Eff.	Aver.	Aver.
U		Class	۲ Total آ	IOWS HV 1	۲۱ Total آ	ows HV 1	Sath	Delay	Service	ي Veh.	ueue Dist 1	Que	Stop Rate	NO. OT Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- ,	km/h
South	: Willo	ughby Ro	d (S)												
1	L2	All MCs	56	13.2	56	13.2	0.150	28.7	LOS C	1.6	12.2	0.83	0.71	0.83	22.8
2	T1	All MCs	94	6.7	94	6.7	0.207	21.5	LOS B	2.6	19.3	0.78	0.63	0.78	27.2
3	R2	All MCs	7	0.0	7	0.0	0.207	29.6	LOS C	2.6	19.3	0.78	0.63	0.78	25.7
Appro	ach		157	8.7	157	8.7	0.207	24.4	LOS B	2.6	19.3	0.80	0.66	0.80	24.8
East:	Alban	y St (E)													
4	L2	All MCs	18	0.0	18	0.0	0.223	16.2	LOS B	4.1	28.8	0.57	0.50	0.57	32.7
5	T1	All MCs	276	0.8	276	0.8	0.447	10.1	LOS A	5.6	39.6	0.63	0.56	0.63	32.4
6	R2	All MCs	153	0.0	153	0.0	*0.447	23.6	LOS B	5.6	39.6	0.83	0.75	0.83	26.3
Appro	ach		446	0.5	446	0.5	0.447	15.0	LOS B	5.6	39.6	0.70	0.62	0.70	30.0
North:	Willo	ughby Ro	I (N)												
7	L2	All MCs	105	0.0	105	0.0	0.120	15.7	LOS B	2.0	14.1	0.59	0.69	0.59	29.0
8	T1	All MCs	118	6.3	118	6.3	0.437	23.6	LOS B	5.3	38.3	0.88	0.77	0.88	24.5
9	R2	All MCs	60	1.8	60	1.8	*0.437	32.7	LOS C	5.3	38.3	0.88	0.77	0.88	22.9
Appro	ach		283	3.0	283	3.0	0.437	22.6	LOS B	5.3	38.3	0.77	0.74	0.77	25.5
West:	Alban	y St (W)													
10	L2	All MCs	97	0.0	97	0.0	0.174	24.3	LOS B	2.5	17.3	0.77	0.73	0.77	24.0
11	T1	All MCs	343	0.0	343	0.0	*0.493	19.2	LOS B	9.3	64.9	0.83	0.71	0.83	27.2
Appro	ach		440	0.0	440	0.0	0.493	20.3	LOS B	9.3	64.9	0.82	0.71	0.82	26.4
All Ve	hicles		1326	1.8	1326	1.8	0.493	19.5	LOS B	9.3	64.9	0.77	0.68	0.77	27.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian I	Moveme	ent Perf	ormand	e							
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
D Clossing	Vol.	Flow	Delay	Service	QUE [Ped	EUE Dist]	Que	Stop Rate	lime	Dist. S	Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Willoug	ghby Rd	(S)									
P1 Full	158	166	28.5	LOS C	0.3	0.3	0.91	0.91	45.2	20.0	0.44
East: Albany S	St (E)										
P2 Full	394	415	28.8	LOS C	0.8	0.8	0.92	0.92	45.5	20.0	0.44

North: Willoug	hby Rd (I	N)									
P3 Full	153	161	28.5	LOS C	0.3	0.3	0.91	0.91	45.2	20.0	0.44
West: Albany S	St (W)										
P4 Full	292	307	28.7	LOS C	0.6	0.6	0.91	0.91	45.4	20.0	0.44
All Pedestrians	997	1049	28.7	LOS C	0.8	0.8	0.91	0.91	45.4	20.0	0.44

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V Site: CST09 [CST09 Albany St / Oxley St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

NA Site Category: (None) Roundabout

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Intel I	lows µ\/1	 Total	lows µ\/1	Satn	Delay	Service	[\/eh	Diet 1	Que	Stop Rate	No. of	Speed
			veh/h	· · v] %	veh/h	%	v/c	sec		veh	m		Trate	Cycles	km/h
South	: Oxle	ey St (S)													
1	L2	All MCs	60	0.0	60	0.0	0.443	8.6	LOS A	3.0	21.5	0.81	0.70	0.84	21.0
2	T1	All MCs	155	2.0	155	2.0	0.443	8.5	LOS A	3.0	21.5	0.81	0.70	0.84	32.2
3	R2	All MCs	74	0.0	74	0.0	0.443	11.3	LOS A	3.0	21.5	0.81	0.70	0.84	30.4
3u	U	All MCs	1	0.0	1	0.0	0.443	12.7	LOS A	3.0	21.5	0.81	0.70	0.84	21.0
Appro	ach		289	1.1	289	1.1	0.443	9.2	LOS A	3.0	21.5	0.81	0.70	0.84	30.3
East:	Alban	y St (E)													
4	L2	All MCs	72	0.0	72	0.0	0.824	12.3	LOSA	6.8	48.6	0.86	0.93	1.12	24.9
5	T1	All MCs	308	3.4	308	3.4	0.824	12.3	LOSA	6.8	48.6	0.86	0.93	1.12	24.9
6	R2	All MCs	42	0.0	42	0.0	0.824	15.0	LOS B	6.8	48.6	0.86	0.93	1.12	31.4
6u	U	All MCs	1	0.0	1	0.0	0.824	16.4	LOS B	6.8	48.6	0.86	0.93	1.12	30.7
Appro	ach		423	2.5	423	2.5	0.824	12.6	LOS A	6.8	48.6	0.86	0.93	1.12	25.9
North:	Oxle	v St (N)													
7	12	All MCs	42	25	42	25	0.369	8.5	LOSA	26	18.4	0.78	0.71	0 78	34.4
8	T1		116	3.6	116	3.6	0.000	83		2.0	18.4	0.78	0.71	0.78	20.8
9	R2	All MCs	103	1.0	103	1.0	0.369	11 1	LOSA	2.0	18.4	0.78	0.71	0.78	20.0
90	112	All MCs	100	0.0	100	0.0	0.369	12.4	LOSA	2.6	18.4	0.78	0.71	0.78	34.7
Appro	ach	7 11100	262	2.4	262	2.4	0.369	9.5	LOSA	2.6	18.4	0.78	0.71	0.78	30.9
West:	Albar	ny St (W)													
10	L2	All MCs	104	0.0	104	0.0	0.579	7.4	LOS A	5.4	37.6	0.78	0.64	0.81	34.0
11	T1	All MCs	339	0.0	339	0.0	0.579	7.2	LOS A	5.4	37.6	0.78	0.64	0.81	33.8
12	R2	All MCs	82	0.0	82	0.0	0.579	10.1	LOS A	5.4	37.6	0.78	0.64	0.81	25.4
12u	U	All MCs	1	0.0	1	0.0	0.579	11.4	LOS A	5.4	37.6	0.78	0.64	0.81	25.4
Appro	ach		526	0.0	526	0.0	0.579	7.7	LOS A	5.4	37.6	0.78	0.64	0.81	33.1
All Ve	hicles		1501	1.3	1501	1.3	0.824	9.7	LOS A	6.8	48.6	0.81	0.74	0.90	30.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST10 [CST10 Albany St / Clarke Ln (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Give-Way (Two-Way)

Vehic	cle M	ovemen	t Perfc	orma	ince										
Mov ID	Turn	Mov Class	Dem Fl [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke Lr	n (SE)												
21a	L1	All MCs	20	0.0	20	0.0	0.049	4.8	LOS A	0.9	6.0	0.42	0.57	0.42	30.0
23b	R3	All MCs	4	0.0	4	0.0	0.049	14.4	LOS A	0.9	6.0	0.42	0.57	0.42	30.0
Appro	ach		24	0.0	24	0.0	0.049	6.5	LOS A	0.9	6.0	0.42	0.57	0.42	30.0
East:	Alban	y St (E)													
5	T1	All MCs	477	2.4	477	2.4	0.125	0.0	LOS A	8.9	63.6	0.00	0.00	0.00	50.0
Appro	ach		477	2.4	477	2.4	0.125	0.0	NA	8.9	63.6	0.00	0.00	0.00	50.0
West:	Albar	ny St (W)													
11	T1	All MCs	526	0.0	526	0.0	0.270	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	49.9
Appro	ach		526	0.0	526	0.0	0.270	0.0	NA	0.0	0.0	0.00	0.00	0.00	49.9
All Ve	hicles		1027	1.1	1027	1.1	0.270	0.2	NA	8.9	63.6	0.01	0.01	0.01	48.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST11 [CST11 Oxley St / Clarke Ln (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

NA Site Category: (None) Stop (Two-Way)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Derr Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
South	East:	Clarke L		%	ven/n	%	V/C	sec	_	ven	m	_	_	_	Km/h
Journ				~ ~	_	~ ~									07.0
1	L2	All MCs	2	0.0	2	0.0	0.010	1.1	LOSA	0.0	0.2	0.32	0.84	0.32	27.3
2	11	All MCs	1	0.0	1	0.0	0.010	9.0	LOS A	0.0	0.2	0.32	0.84	0.32	27.3
3	R2	All MCs	4	0.0	4	0.0	0.010	9.2	LOS A	0.0	0.2	0.32	0.84	0.32	27.3
Appro	ach		7	0.0	7	0.0	0.010	8.8	LOS A	0.0	0.2	0.32	0.84	0.32	27.3
NorthEast: Oxley St (NE)															
5	T1	All MCs	165	0.0	165	0.0	0.120	0.0	LOS A	0.1	0.5	0.04	0.04	0.04	46.5
6	R2	All MCs	7	0.0	7	0.0	0.120	4.9	LOS A	0.1	0.5	0.04	0.04	0.04	46.5
Appro	ach		173	0.0	173	0.0	0.120	0.2	NA	0.1	0.5	0.04	0.04	0.04	46.5
North	West:	Clarke L	n (NW)												
7	L2	All MCs	12	0.0	12	0.0	0.020	5.2	LOS A	0.1	0.5	0.33	0.53	0.33	24.7
9	R2	All MCs	6	0.0	6	0.0	0.020	6.4	LOS A	0.1	0.5	0.33	0.53	0.33	24.7
Appro	ach		18	0.0	18	0.0	0.020	5.6	LOS A	0.1	0.5	0.33	0.53	0.33	24.7
South	West	Oxley St	t (SW)												
10	L2	All MCs	8	0.0	8	0.0	0.121	2.8	LOS A	0.0	0.0	0.00	0.02	0.00	48.1
11	T1	All MCs	226	0.0	226	0.0	0.121	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	48.1
Appro	ach		235	0.0	235	0.0	0.121	0.1	NA	0.0	0.0	0.00	0.02	0.00	48.1
All Ve	hicles		433	0.0	433	0.0	0.121	0.5	NA	0.1	0.5	0.04	0.06	0.04	42.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST12 [CST12 Hume St / Clarke Ln (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Stop (Two-Way)

Vehicle Movement Performance Deg. Mov Turn Mov Level of 95% Back Of Queue Prop. Demand Arrival Aver. ID Class Satn Delay Service Stop No. of Speed Flows Que [Total HV] [Total HV] [Veh. Dist] Rate Cycles % veh/h veh veh/h SouthEast: Clarke Ln (SE) 1 L2 All MCs 7 0.0 7 0.0 0.006 6.7 LOS A 0.0 0.0 0.00 1.00 0.00 28.1 2 T1 All MCs 1 0.0 1 0.0 0.006 LOS A 0.0 0.0 0.00 1.00 0.00 28.1 6.4 Approach 8 0.0 8 0.0 0.006 6.6 LOS A 0.0 0.0 0.00 1.00 0.00 28.1 NorthEast: Hume St (NE) 5 T1 All MCs 46 0.0 46 0.0 0.013 0.0 LOS A 0.0 0.0 0.00 0.03 0.00 38.1 R2 All MCs 3 0.0 LOS A 0.00 6 3 0.0 0.013 34 0.0 0.0 0.00 0.07 36.1 0.00 Approach 49 0.0 49 0.0 0.013 0.2 NA 0.0 0.0 0.00 0.04 38.0 All Vehicles 58 0.0 58 0.0 0.013 1.2 NA 0.0 0.0 0.00 0.18 0.00 33.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST13 [CST13 Pacific Hwy / Alexander St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 763

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Derr Fl [Total veh/h	nand lows HV] %	Ar Fl [Total] veh/h	rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
SouthEast: Pacific Hwy (SE)															
2 3a	T1 R1	All MCs All MCs	1034 225	3.6 3.3	1034 225	3.6 3.3	* 0.464 0.317	8.2 11.7	LOS A LOS A	10.0 4.6	72.1 33.1	0.60 0.44	0.53 0.64	0.60 0.44	35.8 31.1
Appro	ach		1259	3.5	1259	3.5	0.464	8.8	LOS A	10.0	72.1	0.57	0.55	0.57	34.9
North:	Alexa	ander St (N)												
24a 26b	L1 R3	All MCs All MCs	194 111	4.3 1.9	194 111	4.3 1.9	* 0.331 * 0.979	25.2 63.6	LOS B LOS E	6.7 8.0	48.5 57.1	0.81 1.00	0.76 0.99	0.81 1.34	22.7 5.4
Appro	ach		304	3.5	304	3.5	0.979	39.1	LOS C	8.0	57.1	0.88	0.84	1.00	14.5
North	Nest:	Pacific H	wy (NV	V)											
7b	L3	All MCs	18	0.0	18	0.0	0.077	11.5	LOS A	0.3	3.1	0.20	0.41	0.20	31.6
8	T1	All MCs	851	2.7	851	2.7	0.454	5.1	LOS A	7.5	53.0	0.32	0.30	0.32	47.5
Appro	ach		868	2.7	868	2.7	0.454	5.2	LOS A	7.5	53.0	0.31	0.30	0.31	47.3
All Ve	hicles		2432	3.2	2432	3.2	0.979	11.3	LOS A	10.0	72.1	0.52	0.50	0.53	33.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance													
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.			
ID Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	Time	Dist.	Speed			
	ped/h	sec		ped	m			sec	m	m/sec			
North: Alexander St (N)													
P6 Full	154	20.6	LOS C	0.3	0.3	0.80	0.80	37.3	20.0	0.54			
NorthWest: Pacifi	c Hwy (N	W)											
P3 Full	126	50.4	LOS E	0.4	0.4	0.90	0.90	67.1	20.0	0.30			
All Pedestrians	280	34.1	LOS D	0.4	0.4	0.84	0.84	50.7	20.0	0.39			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST14 [CST14 Falcon St / Alexander St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

TCS 764

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	vic	- 		[Veh.	Dist]		Rate	Cycles	km/h
South: Alexander St (S)						/0	v/C	360		Ven		_	_	_	NIII/11
1	L2	All MCs	11	0.0	11	0.0	0.421	68.3	LOS E	8.9	64.5	1.00	0.85	1.00	5.5
2	T1	All MCs	187	3.9	187	3.9	0.468	57.7	LOS E	8.9	64.5	1.00	0.84	1.00	9.3
3	R2	All MCs	45	0.0	45	0.0	0.468	73.6	LOS F	6.0	43.0	1.00	0.83	1.00	18.3
Appro	ach		243	3.0	243	3.0	0.468	61.1	LOS E	8.9	64.5	1.00	0.84	1.00	11.3
East:	Falco	n St (E)													
4	L2	All MCs	38	0.0	38	0.0	0.515	28.4	LOS B	15.6	111.2	0.69	0.62	0.69	31.0
5	T1	All MCs	748	2.1	748	2.1	0.515	22.6	LOS B	15.6	111.2	0.69	0.61	0.69	31.3
6	R2	All MCs	8	75.0	8	75.0	0.515	28.6	LOS C	15.2	109.7	0.68	0.60	0.68	32.4
Appro	ach		795	2.8	795	2.8	0.515	22.9	LOS B	15.6	111.2	0.69	0.61	0.69	31.3
North:	Alexa	ander St (N)												
7	L2	All MCs	34	0.0	34	0.0	*0.439	56.9	LOS E	8.3	59.8	0.93	0.77	0.93	21.7
8	T1	All MCs	284	3.7	284	3.7	0.439	47.6	LOS D	8.9	64.3	0.93	0.76	0.93	6.8
Appro	ach		318	3.3	318	3.3	0.439	48.6	LOS D	8.9	64.3	0.93	0.76	0.93	9.1
West:	Falco	on St (W)													
10	L2	All MCs	219	1.0	219	1.0	*0.355	6.8	LOS A	2.7	19.4	0.10	0.36	0.10	36.3
11	T1	All MCs	768	2.2	768	2.2	0.355	0.6	LOS A	2.7	19.4	0.06	0.14	0.06	56.8
Appro	ach		987	1.9	987	1.9	0.355	2.0	LOS A	2.7	19.4	0.07	0.19	0.07	53.9
All Ve	hicles		2343	2.5	2343	2.5	0.515	21.5	LOS B	15.6	111.2	0.49	0.48	0.49	29.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance												
Mov ID	, Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE BACK OF QUEUE		Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed	
					[Ped	Dist]		Rate				
		ped/h	sec		ped	m			sec	m	m/sec	
Sou	th: Alexander	St (S)										
P1	Full	169	52.4	LOS E	0.6	0.6	0.92	0.92	69.0	20.0	0.29	
East: Falcon St (E)												

P2 Full	81	52.2	LOS E	0.3	0.3	0.92	0.92	68.8	20.0	0.29			
North: Alexander St (N)													
P3 Full	163	52.3	LOS E	0.5	0.5	0.92	0.92	69.0	20.0	0.29			
West: Falcon St (W)													
P4 Full	271	52.6	LOS E	0.9	0.9	0.92	0.92	69.2	20.0	0.29			
All Pedestrians	684	52.4	LOS E	0.9	0.9	0.92	0.92	69.1	20.0	0.29			

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Site: CST01 [CST01 Pacific Hwy / Albany St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 768

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	East:	Pacific H	wy (SE)											
22	T1	All MCs	907	3.7	907	3.7	0.343	1.3	LOS A	2.5	17.7	0.10	0.09	0.10	56.0
23b	R3	All MCs	153	2.1	153	2.1	*0.856	76.3	LOS F	10.1	71.9	1.00	0.91	1.17	6.6
Appro	ach		1060	3.5	1060	3.5	0.856	12.1	LOS A	10.1	71.9	0.23	0.21	0.25	34.9
East:	Alban	y St (E)													
4b	L3	All MCs	49	0.0	49	0.0	*0.726	68.5	LOS E	7.0	49.0	1.00	0.87	1.07	2.7
6a	R1	All MCs	422	0.7	422	0.7	0.726	55.4	LOS D	7.0	49.0	1.00	0.86	1.05	10.1
Appro	ach		472	0.7	472	0.7	0.726	56.8	LOS E	7.0	49.0	1.00	0.87	1.05	9.4
North	Nest:	Pacific H	wy (NV	V)											
27a	L1	All MCs	284	0.4	284	0.4	0.242	9.5	LOS A	3.8	26.6	0.24	0.63	0.24	30.6
28	T1	All MCs	815	2.7	815	2.7	*0.394	9.8	LOS A	9.4	67.3	0.41	0.36	0.41	30.3
Appro	ach		1099	2.1	1099	2.1	0.394	9.7	LOS A	9.4	67.3	0.37	0.43	0.37	30.4
All Ve	hicles		2631	2.4	2631	2.4	0.856	19.1	LOS B	10.1	71.9	0.42	0.42	0.44	23.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	ement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Dem. Aver. A 9 Flow Delay ped/h sec Pacific Hwy (SE)		Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
SouthEast: Pacific	: Hwy (S	E)								
P5 Full	284	50.8	LOS E	0.9	0.9	0.91	0.91	217.4	200.0	0.92
East: Albany St (E)									
P2 Full	253	50.7	LOS E	0.8	0.8	0.91	0.91	67.4	20.0	0.30
All Pedestrians	537	50.7	LOS E	0.9	0.9	0.91	0.91	146.8	115.3	0.79

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST02 [CST02 Pacific Hwy / Oxley St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 767

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	vio			[Veh.	Dist]		Rate	Cycles	km/b
South	East:	Pacific H	wv (SE)	ven/n	/0	v/C	366		VEIT	111	_	_		KI11/11
1	L2	All MCs	118	, 1.8	118	1.8	0.107	9.9	LOS A	1.4	9.6	0.20	0.61	0.20	22.2
2	T1	All MCs	906	3.7	906	3.7	*0.349	2.8	LOSA	5.1	36.6	0.19	0.17	0.19	43.7
Appro	ach		1024	3.5	1024	3.5	0.349	3.6	LOS A	5.1	36.6	0.19	0.22	0.19	37.5
North	East:	Oxley St	(NE)												
4	L2	All MCs	116	1.8	116	1.8	0.318	48.9	LOS D	5.7	40.8	0.90	0.77	0.90	2.8
5	T1	All MCs	77	1.4	77	1.4	0.200	45.0	LOS D	3.9	27.7	0.87	0.68	0.87	8.1
Appro	ach		193	1.6	193	1.6	0.318	47.3	LOS D	5.7	40.8	0.89	0.73	0.89	5.0
North	Nest:	Pacific H	lwy (NV	V)											
7	L2	All MCs	65	3.2	65	3.2	0.237	24.0	LOS B	7.8	56.2	0.54	0.50	0.54	24.1
8	T1	All MCs	799	2.5	799	2.5	0.237	10.6	LOS A	8.9	63.8	0.51	0.40	0.51	27.6
Appro	ach		864	2.6	864	2.6	0.237	11.6	LOS A	8.9	63.8	0.51	0.41	0.51	27.3
South	West	Oxley S	t (SW)												
10	L2	All MCs	154	2.1	154	2.1	*0.529	57.5	LOS E	8.8	62.5	0.97	0.80	0.97	5.1
11	T1	All MCs	102	0.0	102	0.0	0.262	47.0	LOS D	5.3	37.0	0.89	0.73	0.89	6.6
12	R2	All MCs	99	1.1	99	1.1	0.489	58.8	LOS E	5.7	40.5	0.96	0.79	0.96	5.1
Appro	ach		355	1.2	355	1.2	0.529	54.8	LOS D	8.8	62.5	0.94	0.78	0.94	5.5
All Ve	hicles		2436	2.7	2436	2.7	0.529	17.4	LOS B	8.9	63.8	0.47	0.41	0.47	18.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mov	vement	Perform	nance							
Mo		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
		17			[Ped	Dist J		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	ithEast: Pacifi	c Hwy (S	SE)								
P1	Full	1	50.2	LOS E	0.0	0.0	0.90	0.90	66.8	20.0	0.30
Nor	thEast: Oxley	St (NE)									

P2 Full	486	51.2	LOS E	1.6	1.6	0.91	0.91	67.9	20.0	0.29
NorthWest: Pacific	Hwy (N	N)								
P3 Full	259	50.7	LOS E	0.8	0.8	0.91	0.91	67.4	20.0	0.30
SouthWest: Oxley	St (SW)									
P4 Full	201	50.6	LOS E	0.6	0.6	0.90	0.90	67.3	20.0	0.30
All Pedestrians	947	50.9	LOS E	1.6	1.6	0.91	0.91	67.6	20.0	0.30

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Site: CST03 [CST03 Pacific Hwy / Hume St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 766

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovement	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand lows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			l Iotal veh/h	HV J %	l Iotal veh/h	HV J %	v/c	sec		[Veh. veh	Dist J m		Rate	Cycles	km/h
South	East:	Pacific H	wy (SE)											
1	L2	All MCs	31	0.0	31	0.0	0.084	14.2	LOS A	1.9	13.9	0.30	0.36	0.30	31.0
2	T1	All MCs	957	3.7	957	3.7	*0.331	6.8	LOS A	9.2	66.4	0.35	0.32	0.35	41.8
Appro	ach		987	3.6	987	3.6	0.331	7.0	LOS A	9.2	66.4	0.35	0.32	0.35	41.2
North	West:	Pacific H	wy (NV	V)											
8	T1	All MCs	1014	2.3	1014	2.3	0.321	5.3	LOS A	9.0	63.9	0.35	0.31	0.35	35.2
Appro	ach		1014	2.3	1014	2.3	0.321	5.3	LOS A	9.0	63.9	0.35	0.31	0.35	35.2
South	West:	Hume St	(SW)												
10	L2	All MCs	67	0.0	67	0.0	*0.349	62.4	LOS E	3.9	27.6	0.97	0.76	0.97	4.5
12	R2	All MCs	18	5.9	18	5.9	0.075	55.9	LOS D	1.0	7.1	0.90	0.69	0.90	5.1
Appro	ach		85	1.2	85	1.2	0.349	61.0	LOS E	3.9	27.6	0.96	0.75	0.96	4.6
All Ve	hicles		2086	2.9	2086	2.9	0.349	8.4	LOS A	9.2	66.4	0.37	0.33	0.37	33.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pec	lestrian Mov	vement	Perforr	nance							
Mov	·	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUI [Ped	EUE Dist]	Que	Stop Rate	Time	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Sou	thEast: Pacifi	c Hwy (S	SE)								
P1	Full	1	50.2	LOS E	0.0	0.0	0.90	0.90	66.8	20.0	0.30
Nor	thWest: Pacifi	c Hwy (N	W)								
P3	Full	115	50.4	LOS E	0.4	0.4	0.90	0.90	67.1	20.0	0.30
Sou	thWest: Hum	e St (SW	')								
P4	Full	81	53.1	LOS E	0.3	0.3	0.92	0.92	69.7	20.0	0.29
All F	Pedestrians	197	51.5	LOS E	0.4	0.4	0.91	0.91	68.2	20.0	0.29

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST04 [CST04 Pacific Hwy / Falcon St / Shirley Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 765

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Total	lows H\/1	H I Total	IOWS	Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		T tato	Cycles	km/h
South	East:	Pacific H	wy (SE)											
1	L2	All MCs	306	1.0	306	1.0	0.302	17.8	LOS B	8.1	57.3	0.47	0.71	0.47	28.8
2	T1	All MCs	565	4.5	565	4.5	0.557	38.2	LOS C	14.5	105.3	0.87	0.74	0.87	10.6
Appro	ach		872	3.3	872	3.3	0.557	31.1	LOS C	14.5	105.3	0.73	0.73	0.73	16.7
East:	Falco	n St (E)													
21b	L3	All MCs	12	9.1	12	9.1	0.915	25.0	LOS B	18.5	130.6	0.99	0.98	1.16	6.3
21a	L1	All MCs	355	0.6	355	0.6	*0.915	52.7	LOS D	18.5	130.6	0.99	0.98	1.16	15.0
23a	R1	All MCs	359	2.9	359	2.9	0.672	34.1	LOS C	16.8	120.9	0.82	0.80	0.82	8.7
Appro	ach		725	1.9	725	1.9	0.915	43.1	LOS D	18.5	130.6	0.90	0.89	0.99	12.7
North:	Willo	ughby Ro	l (N)												
7	L2	All MCs	92	0.0	92	0.0	0.060	3.8	LOS A	0.4	2.9	0.08	0.48	0.08	37.0
Appro	ach		92	0.0	92	0.0	0.060	3.8	LOS A	0.4	2.9	0.08	0.48	0.08	37.0
North	West:	Pacific H	wy (NV	V)											
7a	L1	All MCs	408	3.1	408	3.1	*0.575	18.6	LOS B	11.0	78.7	0.75	0.78	0.75	23.9
8	T1	All MCs	623	1.9	623	1.9	0.665	53.1	LOS D	20.3	144.4	1.00	0.87	1.00	11.6
Appro	ach		1032	2.3	1032	2.3	0.665	39.4	LOS C	20.3	144.4	0.90	0.83	0.90	14.5
South	West	Shirley F	Rd (SW)											
10	L2	All MCs	63	0.0	63	0.0	*0.647	59.0	LOS E	16.6	116.1	0.96	0.83	0.96	12.7
12a	R1	All MCs	447	0.0	447	0.0	0.647	46.4	LOS D	18.7	131.8	0.95	0.83	0.95	12.9
12	R2	All MCs	137	1.5	137	1.5	0.647	47.9	LOS D	18.7	131.8	0.94	0.83	0.94	13.1
Appro	ach		647	0.3	647	0.3	0.647	47.9	LOS D	18.7	131.8	0.95	0.83	0.95	12.9
All Ve	hicles		3367	2.0	3367	2.0	0.915	38.7	LOS C	20.3	144.4	0.84	0.81	0.86	14.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE E QUEL [Ped	BACK OF JE Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed

	ped/h	sec		ped	m			sec	m	m/sec
SouthEas	t: Pacific Hwy (SE)								
P1 Full	79	49.4	LOS E	0.2	0.2	0.89	0.89	66.1	20.0	0.30
East: Falo	con St (E)									
P5 Full	204	53.4	LOS E	0.7	0.7	0.93	0.93	70.0	20.0	0.29
NorthWes	st: Pacific Hwy (NW)								
P3 Full	341	50.0	LOS E	1.1	1.1	0.90	0.90	66.7	20.0	0.30
SouthWe	st: Shirley Rd (S	SW)								
P4 Full	81	53.1	LOS E	0.3	0.3	0.92	0.92	69.7	20.0	0.29
All Pedes	trians 705	51.3	LOS E	1.1	1.1	0.91	0.91	67.9	20.0	0.29

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V Site: CST05 [CST05 Clarke St / Oxley St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows HV]	Ar Fl [Total]	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Bacl [Veh.	k Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
Oth	E t.		veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
South	East:	Clarke S	t (SE)												
1	L2	All MCs	38	0.0	38	0.0	0.076	5.0	LOS A	0.3	1.8	0.32	0.55	0.32	32.5
3a	R1	All MCs	42	0.0	42	0.0	0.076	5.7	LOS A	0.3	1.8	0.32	0.55	0.32	32.5
Appro	ach		80	0.0	80	0.0	0.076	5.4	LOS A	0.3	1.8	0.32	0.55	0.32	32.5
North:	Oxle	y St (N)													
24a	L1	All MCs	127	0.0	127	0.0	0.154	4.4	LOS A	0.0	0.0	0.00	0.53	0.00	29.6
26a	R1	All MCs	156	1.4	156	1.4	0.154	4.1	LOS A	0.0	0.0	0.00	0.53	0.00	29.6
Appro	ach		283	0.7	283	0.7	0.154	4.2	NA	0.0	0.0	0.00	0.53	0.00	29.6
South	West:	Oxley St	t (SW)												
10a	L1	All MCs	123	1.7	123	1.7	0.106	2.8	LOS A	0.4	2.7	0.25	0.54	0.25	22.7
12	R2	All MCs	56	0.0	56	0.0	0.106	4.9	LOS A	0.4	2.7	0.25	0.54	0.25	22.7
Appro	ach		179	1.2	179	1.2	0.106	3.4	NA	0.4	2.7	0.25	0.54	0.25	22.7
All Ve	hicles		542	0.8	542	0.8	0.154	4.1	NA	0.4	2.7	0.13	0.54	0.13	28.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST06 [CST06 Clarke St / Hume St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke S	t (SE)												
21	L2	All MCs	1	0.0	1	0.0	0.053	3.4	LOS A	0.1	0.8	0.12	0.13	0.12	19.5
2	T1	All MCs	80	0.0	80	0.0	0.053	0.0	LOS A	0.1	0.8	0.12	0.13	0.12	36.4
23a	R1	All MCs	18	0.0	18	0.0	0.053	3.8	LOS A	0.1	0.8	0.12	0.13	0.12	37.4
Appro	ach		99	0.0	99	0.0	0.053	0.7	NA	0.1	0.8	0.12	0.13	0.12	36.4
North	West:	Clarke S	t (NW)												
27b	L3	All MCs	9	0.0	9	0.0	0.095	4.1	LOS A	0.0	0.1	0.00	0.03	0.00	38.3
8	T1	All MCs	173	0.0	173	0.0	0.095	0.0	LOS A	0.0	0.1	0.00	0.03	0.00	39.5
29	R2	All MCs	1	0.0	1	0.0	0.095	3.9	LOS A	0.0	0.1	0.00	0.03	0.00	20.4
Appro	ach		183	0.0	183	0.0	0.095	0.2	NA	0.0	0.1	0.00	0.03	0.00	39.3
All Ve	hicles	;	282	0.0	282	0.0	0.095	0.4	NA	0.1	0.8	0.05	0.06	0.05	38.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST07 [CST07 Clarke St / Willoughby Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total] veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% I Qu Veh. veh	Back Of Jeue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Willo	oughby R	d (S)												
1	L2	All MCs	92	0.0	92	0.0	0.218	5.8	LOS A	1.1	7.7	0.38	0.36	0.38	31.1
2	T1	All MCs	119	6.2	119	6.2	0.218	1.0	LOS A	1.1	7.7	0.38	0.36	0.38	35.8
Appro	ach		211	3.5	211	3.5	0.218	3.1	NA	1.1	7.7	0.38	0.36	0.38	34.2
North:	Willo	ughby Ro	d (N)												
8	T1	All MCs	148	3.5	148	3.5	0.214	1.0	LOS A	0.9	6.7	0.33	0.27	0.33	36.0
9	R2	All MCs	46	0.0	46	0.0	0.214	9.1	LOS A	0.9	6.7	0.33	0.27	0.33	35.1
Appro	ach		195	2.7	195	2.7	0.214	2.9	NA	0.9	6.7	0.33	0.27	0.33	35.8
West:	Clark	e St (W)													
10	L2	All MCs	61	0.0	61	0.0	0.212	7.2	LOS A	0.8	5.3	0.61	0.80	0.62	31.1
12	R2	All MCs	74	1.4	74	1.4	0.212	8.4	LOS A	0.8	5.3	0.61	0.80	0.62	24.5
Appro	ach		135	0.8	135	0.8	0.212	7.9	LOS A	0.8	5.3	0.61	0.80	0.62	28.3
All Ve	nicles		540	2.5	540	2.5	0.218	4.2	NA	1.1	7.7	0.42	0.44	0.42	33.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST08 [CST08 Albany St / Willoughby Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 516

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 65 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95%	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI Total	lows	FI Total	OWS	Satn	Delay	Service	Q [\/ob		Que	Stop	No. of	Speed
			veh/h	пvј %	veh/h	⊓vj %	v/c	sec		ven.	m Dist j		Nale	Cycles	km/h
South	: Willo	ughby Ro	d (S)												
1	L2	All MCs	53	8.0	53	8.0	0.133	25.4	LOS B	1.4	10.2	0.83	0.71	0.83	23.5
2	T1	All MCs	85	3.7	85	3.7	0.166	18.2	LOS B	2.1	15.4	0.76	0.61	0.76	28.3
3	R2	All MCs	5	0.0	5	0.0	0.166	25.4	LOS B	2.1	15.4	0.76	0.61	0.76	26.7
Appro	ach		143	5.1	143	5.1	0.166	21.1	LOS B	2.1	15.4	0.79	0.65	0.79	26.3
East: /	Alban	y St (E)													
4	L2	All MCs	14	0.0	14	0.0	0.113	14.9	LOS B	1.8	12.6	0.53	0.46	0.53	33.3
5	T1	All MCs	303	0.0	303	0.0	0.567	10.8	LOS A	8.7	60.8	0.74	0.65	0.74	31.0
6	R2	All MCs	163	0.0	163	0.0	*0.567	21.8	LOS B	8.7	60.8	0.84	0.74	0.84	28.5
Appro	ach		480	0.0	480	0.0	0.567	14.7	LOS B	8.7	60.8	0.77	0.68	0.77	30.1
North:	Willo	ughby Ro	l (N)												
7	L2	All MCs	121	0.0	121	0.0	0.137	14.8	LOS B	2.1	15.0	0.59	0.69	0.59	29.6
8	T1	All MCs	136	3.1	136	3.1	*0.391	21.1	LOS B	4.8	34.1	0.86	0.75	0.86	26.0
9	R2	All MCs	44	0.0	44	0.0	0.391	30.0	LOS C	4.8	34.1	0.86	0.75	0.86	24.5
Appro	ach		301	1.4	301	1.4	0.391	19.9	LOS B	4.8	34.1	0.75	0.72	0.75	27.0
West:	Alban	y St (W)													
10	L2	All MCs	94	0.0	94	0.0	0.182	24.2	LOS B	2.3	16.1	0.80	0.73	0.80	24.1
11	T1	All MCs	261	0.4	261	0.4	*0.397	18.0	LOS B	6.4	45.2	0.81	0.68	0.81	27.9
Appro	ach		355	0.3	355	0.3	0.397	19.6	LOS B	6.4	45.2	0.81	0.69	0.81	26.8
All Ve	hicles		1279	1.0	1279	1.0	0.567	18.0	LOS B	8.7	60.8	0.78	0.69	0.78	27.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian N	Noveme	ent Perf	ormand	e:							
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QUE [Ped	=UE Dist]	Que	Stop Rate	lime	Dist. S	Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Willoug	hby Rd ((S)									
P1 Full	162	171	26.1	LOS C	0.3	0.3	0.90	0.90	42.7	20.0	0.47
East: Albany S	St (E)										
P2 Full	311	327	26.2	LOS C	0.5	0.5	0.90	0.90	42.9	20.0	0.47

North: Willoug	hby Rd (N)									
P3 Full	144	152	26.0	LOS C	0.3	0.3	0.90	0.90	42.7	20.0	0.47
West: Albany S	St (W)										
P4 Full	353	372	26.3	LOS C	0.6	0.6	0.91	0.91	43.0	20.0	0.47
All Pedestrians	970	1021	26.2	LOS C	0.6	0.6	0.90	0.90	42.9	20.0	0.47

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V Site: CST09 [CST09 Albany St / Oxley St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA Site Category: (None) Roundabout

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Total	OWS 山い1	 Total	lows 山\/ 1	Satn	Delay	Service	[\/ob	Diet 1	Que	Stop	No. of	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		Tale	Cycles	km/h
South	: Oxle	y St (S)													
1	L2	All MCs	64	0.0	64	0.0	0.267	7.3	LOS A	1.7	12.2	0.68	0.63	0.68	22.7
2	T1	All MCs	86	0.0	86	0.0	0.267	7.1	LOS A	1.7	12.2	0.68	0.63	0.68	33.7
3	R2	All MCs	55	0.0	55	0.0	0.267	10.0	LOS A	1.7	12.2	0.68	0.63	0.68	31.8
3u	U	All MCs	1	0.0	1	0.0	0.267	11.4	LOS A	1.7	12.2	0.68	0.63	0.68	22.7
Appro	ach		206	0.0	206	0.0	0.267	8.0	LOS A	1.7	12.2	0.68	0.63	0.68	31.0
East: /	Alban	y St (E)													
4	L2	All MCs	54	3.9	54	3.9	0.445	6.6	LOS A	2.8	19.9	0.52	0.62	0.52	32.5
5	T1	All MCs	301	1.0	301	1.0	0.445	6.3	LOSA	2.8	19.9	0.52	0.62	0.52	32.5
6	R2	All MCs	43	0.0	43	0.0	0.445	9.2	LOSA	2.8	19.9	0.52	0.62	0.52	36.8
6u	U	All MCs	1	0.0	1	0.0	0.445	10.6	LOSA	2.8	19.9	0.52	0.62	0.52	36.3
Appro	ach		399	1.3	399	1.3	0.445	6.7	LOS A	2.8	19.9	0.52	0.62	0.52	33.3
North:	Oxle	v St (N)													
7	12		34	0.0	34	0.0	0 275	7 1	LOSA	17	12 1	0.66	0.64	0.66	36.1
8	T1		120	0.0	120	0.0	0.275	6.9		17	12.1	0.66	0.64	0.66	32.1
9	R2	All MCs	66	0.0	66	0.0	0.275	9.8	LOSA	1.7	12.1	0.66	0.64	0.00	32.1
911	11		1	0.0	1	0.0	0.275	11.2	LOSA	1.7	12.1	0.00	0.64	0.00	36.3
Appro	ach	7 11 11/00	221	0.0	221	0.0	0.275	7.8	LOSA	1.7	12.1	0.66	0.64	0.66	33.0
West:	Albar	ny St (W)													
10	L2	All MCs	99	1.1	99	1.1	0.402	5.7	LOS A	3.2	22.3	0.56	0.54	0.56	35.4
11	T1	All MCs	245	0.9	245	0.9	0.402	5.5	LOS A	3.2	22.3	0.56	0.54	0.56	35.3
12	R2	All MCs	78	1.4	78	1.4	0.402	8.4	LOS A	3.2	22.3	0.56	0.54	0.56	27.4
12u	U	All MCs	1	0.0	1	0.0	0.402	9.7	LOS A	3.2	22.3	0.56	0.54	0.56	27.4
Appro	ach		423	1.0	423	1.0	0.402	6.0	LOS A	3.2	22.3	0.56	0.54	0.56	34.4
All Ve	hicles		1249	0.8	1249	0.8	0.445	6.9	LOS A	3.2	22.3	0.58	0.60	0.58	33.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CST10 [CST10 Albany St / Clarke Ln (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfo	orma	ince										
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Clarke Li	n (SE)												
21a	L1	All MCs	21	0.0	21	0.0	0.043	4.2	LOS A	2.9	20.3	0.17	0.50	0.17	31.7
23b	R3	All MCs	4	0.0	4	0.0	0.043	12.8	LOS A	2.9	20.3	0.17	0.50	0.17	31.7
Appro	ach		25	0.0	25	0.0	0.043	5.6	LOS A	2.9	20.3	0.17	0.50	0.17	31.7
East:	Alban	y St (E)													
5	T1	All MCs	451	0.7	451	0.7	0.212	0.0	LOS A	8.7	61.1	0.00	0.00	0.00	49.9
Appro	ach		451	0.7	451	0.7	0.212	0.0	NA	8.7	61.1	0.00	0.00	0.00	49.9
West:	Albar	ny St (W)													
11	T1	All MCs	437	1.0	437	1.0	0.226	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	49.9
Appro	ach		437	1.0	437	1.0	0.226	0.0	NA	0.0	0.0	0.00	0.00	0.00	49.9
All Ve	hicles		913	0.8	913	0.8	0.226	0.2	NA	8.7	61.1	0.00	0.01	0.00	48.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST11 [CST11 Oxley St / Clarke Ln (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA Site Category: (None) Stop (Two-Way)

Vehic	cle M	ovemen	t Perfc	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Bacl	k Of Queue	e Prop.	Eff.	Aver.	Aver.
UI		Class	Fi Total	IOWS HV 1	۲۱ Total I آ	ows HV 1	Sath	Delay	Service	[Veh.	Dist 1	Que	Stop Rate	NO. OT Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	East:	Clarke Li	n (SE)												
1	L2	All MCs	1	0.0	1	0.0	0.005	7.7	LOS A	0.0	0.1	0.32	0.83	0.32	27.6
2	T1	All MCs	1	0.0	1	0.0	0.005	8.8	LOS A	0.0	0.1	0.32	0.83	0.32	27.6
3	R2	All MCs	2	0.0	2	0.0	0.005	9.0	LOS A	0.0	0.1	0.32	0.83	0.32	27.6
Appro	ach		4	0.0	4	0.0	0.005	8.6	LOS A	0.0	0.1	0.32	0.83	0.32	27.6
North	East:	Oxley St	(NE)												
5	T1	All MCs	185	1.1	185	1.1	0.109	0.0	LOS A	0.5	3.7	0.02	0.03	0.02	47.5
6	R2	All MCs	6	0.0	6	0.0	0.109	3.6	LOS A	0.5	3.7	0.02	0.03	0.02	47.5
Appro	ach		192	1.1	192	1.1	0.109	0.1	NA	0.5	3.7	0.02	0.03	0.02	47.5
North	West:	Clarke L	n (NW)												
7	L2	All MCs	12	0.0	12	0.0	0.016	5.0	LOS A	0.1	0.5	0.29	0.51	0.29	25.1
9	R2	All MCs	5	20.0	52	20.0	0.016	6.7	LOS A	0.1	0.5	0.29	0.51	0.29	25.1
Appro	ach		17	6.3	17	6.3	0.016	5.5	LOS A	0.1	0.5	0.29	0.51	0.29	25.1
South	West	Oxley St	t (SW)												
10	L2	All MCs	1	0.0	1	0.0	0.086	2.8	LOS A	0.0	0.0	0.00	0.00	0.00	49.6
11	T1	All MCs	164	1.3	164	1.3	0.086	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	49.6
Appro	ach		165	1.3	165	1.3	0.086	0.0	NA	0.0	0.0	0.00	0.00	0.00	49.6
All Ve	hicles		378	1.4	378	1.4	0.109	0.4	NA	0.5	3.7	0.03	0.05	0.03	43.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CST13 [CST13 Pacific Hwy / Alexander St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: CST-N1 [CST Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 763

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows HV]	Ar Fl [Total]	rival ows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	Foot	Dooifio H	veh/h	% \	veh/h	%	V/C	sec	_	veh	m	_	_	_	km/h
South	Easi.		wy (SE)											
2	T1	All MCs	760	3.3	760	3.3	*0.311	5.4	LOS A	5.8	41.7	0.47	0.40	0.47	41.4
3a	R1	All MCs	202	1.6	202	1.6	0.264	8.9	LOS A	3.4	24.3	0.35	0.59	0.35	35.3
Appro	ach		962	3.0	962	3.0	0.311	6.1	LOS A	5.8	41.7	0.44	0.44	0.44	39.9
North:	Alexa	ander St ((N)												
24a	L1	All MCs	192	2.7	192	2.7	*0.358	30.8	LOS C	7.7	55.5	0.87	0.77	0.87	20.2
26b	R3	All MCs	112	2.8	112	2.8	*0.883	64.2	LOS E	7.2	51.7	1.00	0.87	1.14	5.3
Appro	ach		303	2.8	303	2.8	0.883	43.1	LOS D	7.7	55.5	0.92	0.81	0.97	13.5
North	Nest:	Pacific H	lwy (NV	V)											
7b	L3	All MCs	13	8.3	13	8.3	0.035	10.5	LOS A	0.1	1.4	0.18	0.47	0.18	32.0
8	T1	All MCs	752	1.8	752	1.8	*0.359	3.3	LOS A	3.8	26.9	0.24	0.22	0.24	51.3
Appro	ach		764	1.9	764	1.9	0.359	3.4	LOS A	3.8	26.9	0.24	0.22	0.24	51.1
All Ve	hicles		2029	2.5	2029	2.5	0.883	10.6	LOS A	7.7	55.5	0.44	0.41	0.44	34.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perforr	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
North: Alexander	St (N)									
P6 Full	122	22.2	LOS C	0.2	0.2	0.80	0.80	38.8	20.0	0.52
NorthWest: Pacifi	c Hwy (N	W)								
P3 Full	75	50.3	LOS E	0.2	0.2	0.90	0.90	67.0	20.0	0.30
All Pedestrians	197	32.9	LOS D	0.2	0.2	0.84	0.84	49.5	20.0	0.40

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: CST14 [CST14 Falcon St / Alexander St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

TCS 764

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 125 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
Cauth		andan Ct	veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
South	Alex	ander St	(5)												
1	L2	All MCs	6	16.7	6	16.7	0.407	64.4	LOS E	9.9	71.0	1.00	0.84	1.00	5.8
2	T1	All MCs	160	2.0	160	2.0	0.407	55.4	LOS D	9.9	71.0	1.00	0.84	1.00	10.0
3	R2	All MCs	48	0.0	48	0.0	0.478	72.6	LOS F	3.1	21.9	1.00	0.77	1.00	16.9
Appro	ach		215	2.0	215	2.0	0.478	59.5	LOS E	9.9	71.0	1.00	0.83	1.00	12.0
East:	Falco	n St (E)													
4	L2	All MCs	31	0.0	31	0.0	0.672	17.8	LOS B	17.9	127.1	0.55	0.50	0.55	40.2
5	T1	All MCs	859	2.0	859	2.0	0.672	11.8	LOS A	17.9	127.1	0.54	0.49	0.54	40.5
6	R2	All MCs	4	75.0	4	75.0	0.672	17.9	LOS B	9.4	67.5	0.53	0.47	0.53	39.9
Appro	ach		894	2.2	894	2.2	0.672	12.1	LOS A	17.9	127.1	0.54	0.49	0.54	40.5
North	Alexa	ander St ((N)												
7	L2	All MCs	66	0.0	66	0.0	0.203	82.1	LOS F	3.5	24.4	0.89	0.75	0.89	21.0
8	T1	All MCs	283	3.0	283	3.0	* 0.767	79.7	LOS F	16.9	121.7	1.00	0.90	1.07	6.4
Appro	ach		349	2.4	349	2.4	0.767	80.2	LOS F	16.9	121.7	0.98	0.87	1.04	10.1
West:	Falco	on St (W)													
10	L2	All MCs	220	0.5	220	0.5	0.184	17.3	LOS B	6.7	47.2	0.55	0.66	0.55	21.3
11	T1	All MCs	726	1.9	726	1.9	*0.558	2.4	LOS A	8.2	58.4	0.21	0.19	0.21	55.5
Appro	ach		946	1.6	946	1.6	0.558	5.9	LOS A	8.2	58.4	0.28	0.30	0.28	47.7
All Ve	hicles		2404	2.0	2404	2.0	0.767	23.8	LOS B	17.9	127.1	0.54	0.50	0.55	31.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mo	vement	Perform	nance							
Mo∖ ID	, Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Alexander	St (S)									
P1	Full	157	52.3	LOS E	0.5	0.5	0.92	0.92	69.0	20.0	0.29
Eas	t: Falcon St (I	Ξ)									

P2 Full	133	52.3	LOS E	0.4	0.4	0.92	0.92	68.9	20.0	0.29
North: Alexander S	t (N)									
P3 Full	166	52.3	LOS E	0.5	0.5	0.92	0.92	69.0	20.0	0.29
West: Falcon St (W	/)									
P4 Full	229	52.5	LOS E	0.8	0.8	0.92	0.92	69.2	20.0	0.29
All Pedestrians	685	52.4	LOS E	0.8	0.8	0.92	0.92	69.0	20.0	0.29

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Site: VIC01 [VIC01 Pacific Hwy / Berry St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: VIC-N1 [VIC Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 1206

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand Iows HV 1	Ar Fl [Total]	rival lows HV 1	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist 1	e Prop. Que	Eff. Stop Rate	Aver. No. of Cvcles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- 5	km/h
South	East:	Pacific H	wy (SE)											
1	L2	All MCs	61	1.7	61	1.7	0.243	10.0	LOS A	7.4	54.0	0.44	0.37	0.44	37.4
2	T1	All MCs	929	5.9	929	5.9	0.243	3.5	LOS A	7.4	54.0	0.25	0.21	0.25	51.5
23b	R3	All MCs	272	1.9	272	1.9	*0.852	31.7	LOS C	8.5	60.8	0.98	0.87	1.10	19.3
Appro	ach		1262	4.8	1262	4.8	0.852	9.9	LOS A	8.5	60.8	0.42	0.36	0.44	41.1
North	West:	Pacific H	wy (NV	V)											
27a	L1	All MCs	838	5.0	838	5.0	0.413	8.7	LOS A	7.6	55.2	0.32	0.68	0.32	33.4
8	T1	All MCs	488	4.3	488	4.3	*0.668	23.3	LOS B	15.0	108.7	0.88	0.84	0.88	19.0
Appro	ach		1326	4.8	1326	4.8	0.668	14.1	LOS A	15.0	108.7	0.53	0.74	0.53	26.1
South	West	Berry St	(SW)												
10	L2	All MCs	53	0.0	53	0.0	0.114	5.9	LOS A	0.6	4.4	0.22	0.54	0.22	36.0
Appro	ach		53	0.0	53	0.0	0.114	5.9	LOS A	0.6	4.4	0.22	0.54	0.22	36.0
All Ve	hicles		2641	4.7	2641	4.7	0.852	11.9	LOS A	15.0	108.7	0.47	0.55	0.48	34.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian	Movement	t Perforr	nance										
Mov ID Crossing	Dem. 9 Flow	Dem. Aver. Level of Flow Delay Service		AVERAGE QU [Ped	E BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed			
	ped/h	sec		ped	m			sec	m	m/sec			
SouthEast: Pacific Hwy (SE)													
P1 Full	324	51.4	LOS E	1.0	1.0	0.95	0.95	68.1	20.0	0.29			
East: Berry S	St (E)												
P2 Full	435	51.6	LOS E	1.4	1.4	0.96	0.96	218.3	200.0	0.92			
NorthWest: F	Pacific Hwy (NW)											
P3B Slip/ Bypass	1	50.7	LOS E	0.0	0.0	0.94	0.94	67.4	20.0	0.30			
SouthWest: E	Berry St (SW	/)											
P4 Full	287	26.4	LOS C	0.6	0.6	0.91	0.91	43.0	20.0	0.46			

All Pedestrians	1047	44.6	LOS E	1.4	1.4	0.94	0.94	123.6	94.7	0.77
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Project: C:\Users\WanJ2\OneDrive - AECOM\General - ANZ-NAC-Sydney Metro-Sydney Metro C&SW Operational Monitoring\400_Technical \432_Traffic Analysis\SIDRA Modelling\04 Block 4\02 SIDRA Models with volumes\03 SM C&SW_VIC (Block 4).sip9

Site: VIC02 [VIC02 Miller St / Berry St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: VIC-N1 [VIC Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 874

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Iotal veh/h	HV J %	[lotal veh/h	HV J %	v/c	sec		[Veh. veh	Dist J m		Rate	Cycles	km/h
South	: Mille	er St (S)													
2	T1	All MCs	360	8.8	360	8.8	0.974	33.8	LOS C	25.0	186.2	0.80	0.76	0.92	13.1
3	R2	All MCs	264	7.2	264	7.2	*0.974	87.7	LOS F	25.0	186.2	1.00	1.30	1.49	11.0
Appro	ach		624	8.1	624	8.1	0.974	56.6	LOS E	25.0	186.2	0.88	0.98	1.16	11.8
North:	Mille	r St (N)													
7	L2	All MCs	195	4.9	195	4.9	0.793	60.5	LOS E	11.5	83.8	1.00	0.93	1.16	14.2
8	T1	All MCs	307	15.1	307	15.1	0.773	47.8	LOS D	17.1	135.2	0.99	0.92	1.08	11.5
Appro	ach		502	11.1	502	11.1	0.793	52.7	LOS D	17.1	135.2	1.00	0.92	1.11	12.8
West:	Berry	' St (W)													
10	L2	All MCs	195	3.8	195	3.8	*0.695	43.5	LOS D	16.5	118.5	0.86	0.78	0.86	10.3
11	T1	All MCs	982	1.6	982	1.6	0.695	31.8	LOS C	21.0	155.6	0.87	0.77	0.87	18.3
12	R2	All MCs	65	38.7	65	38.7	0.695	41.0	LOS C	21.0	155.6	0.88	0.78	0.88	10.8
Appro	ach		1242	3.9	1242	3.9	0.695	34.1	LOS C	21.0	155.6	0.87	0.77	0.87	16.0
All Ve	hicles		2368	6.5	2368	6.5	0.974	44.0	LOS D	25.0	186.2	0.90	0.86	1.00	13.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mov	rement	Perform	nance							
Moʻ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	ith: Miller St (S	5)									
P1	Full	417	51.6	LOS E	1.3	1.3	0.96	0.96	68.3	20.0	0.29
Eas	st: Berry St (E)										
P2	Full	267	51.3	LOS E	0.8	0.8	0.95	0.95	68.0	20.0	0.29
Nor	th: Miller St (N)									
P3	Full	273	51.3	LOS E	0.8	0.8	0.95	0.95	68.0	20.0	0.29
We	st: Berry St (W	')									
P4	Full	516	51.8	LOS E	1.6	1.6	0.96	0.96	68.5	20.0	0.29

All Pedestrians	1473	51.6	LOS E	1.6	1.6	0.95	0.95	68.2	20.0	0.29
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Site: VIC03 [VIC03 Miller St / McLaren St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: VIC-N1 [VIC Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 1156

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
U		Class	H [Total	lows HV 1	FI [Total]	lows HV/1	Satn	Delay	Service	[Veh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		i tato	0,0100	km/h
South	: Mille	er St (S)													
1	L2	All MCs	127	3.3	127	3.3	0.101	11.8	LOS A	2.2	15.5	0.34	0.58	0.34	34.3
2	T1	All MCs	397	8.5	397	8.5	0.437	13.1	LOS A	11.8	88.6	0.56	0.51	0.56	32.6
3	R2	All MCs	31	3.4	31	3.4	*0.437	27.6	LOS B	11.8	88.6	0.56	0.51	0.56	26.9
Appro	ach		555	7.0	555	7.0	0.437	13.6	LOS A	11.8	88.6	0.51	0.53	0.51	31.2
East:	McLa	ren St (E)												
4	L2	All MCs	47	6.7	47	6.7	0.752	88.5	LOS F	2.9	21.2	1.00	0.86	1.29	7.7
5	T1	All MCs	122	4.3	122	4.3	*0.649	71.6	LOS F	6.6	47.7	1.00	0.83	1.06	18.1
Appro	ach		169	5.0	169	5.0	0.752	76.3	LOS F	6.6	47.7	1.00	0.84	1.12	12.3
North	Mille	r St (N)													
7	L2	All MCs	74	0.0	74	0.0	0.388	19.6	LOS B	11.4	84.4	0.53	0.60	0.53	19.4
8	T1	All MCs	388	7.9	388	7.9	0.388	11.1	LOS A	11.4	84.4	0.54	0.61	0.54	27.2
9	R2	All MCs	162	3.2	162	3.2	0.388	23.2	LOS B	6.1	43.9	0.64	0.73	0.64	29.0
Appro	ach		624	5.7	624	5.7	0.388	15.2	LOS B	11.4	84.4	0.56	0.64	0.56	26.6
West:	McLa	ren St (V	V)												
10	L2	All MCs	115	3.7	115	3.7	0.441	52.2	LOS D	5.8	41.5	0.95	0.79	0.95	18.9
11	T1	All MCs	103	3.1	103	3.1	0.783	49.4	LOS D	7.7	55.6	1.00	0.97	1.20	15.4
12	R2	All MCs	46	4.5	46	4.5	*0.783	58.0	LOS E	7.7	55.6	1.00	0.97	1.20	12.9
Appro	ach		264	3.6	264	3.6	0.783	52.1	LOS D	7.7	55.6	0.98	0.89	1.09	16.7
All Ve	hicles		1613	5.7	1613	5.7	0.783	27.1	LOS B	11.8	88.6	0.66	0.66	0.69	22.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	/ement	Perforr	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Miller St (S	5)									
P1	Full	441	49.1	LOS E	1.3	1.3	0.95	0.95	65.8	20.0	0.30

East: McLaren St	East: McLaren St (E)														
P2 Full	334	48.9	LOS E	1.0	1.0	0.95	0.95	65.6	20.0	0.31					
North: Miller St (N)														
P3 Full	438	49.1	LOS E	1.3	1.3	0.95	0.95	65.8	20.0	0.30					
West: McLaren St	(W)														
P4 Full	141	48.5	LOS E	0.4	0.4	0.94	0.94	65.2	20.0	0.31					
All Pedestrians	1354	49.0	LOS E	1.3	1.3	0.95	0.95	65.7	20.0	0.30					

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Site: VIC04 [VIC04 Pacific Hwy / Miller St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: VIC-N1 [VIC Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 630

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Network Site User-Given Phase Times)

Vehicle Movement Performance														
Mov	Turn	Mov	Dem	and	Arriva	l Deg.	Aver.	Level of	95% Bacl	k Of Queue	e Prop.	Eff.	Aver.	Aver.
ID		Class	H Total I	OWS	Flow: //H I Total	s Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h %	v/c	sec		veh	m		Tate	Cyclc3	km/h
South	: Mille	er St (S)												
1	L2	All MCs	36	66.7	3 66.	0.398	25.1	LOS B	6.8	56.7	0.88	0.75	0.88	20.9
1a	L1	All MCs	952	25.6	95 25.0	0.398	29.0	LOS C	6.8	56.7	0.88	0.75	0.88	12.6
2	T1	All MCs	169 ⁻	13.0	169 13.0	0.663	50.3	LOS D	10.1	77.3	0.95	0.81	0.97	11.7
3b	R3	All MCs	64	6.6	64 6.0	* 0.663	55.8	LOS D	10.1	77.3	0.97	0.84	1.00	18.5
Appro	ach		332 2	15.9	332 15.9	0.663	45.0	LOS D	10.1	77.3	0.93	0.80	0.95	13.7
South	East:	Pacific H	wy (SE))										
21b	L3	All MCs	173	4.3	173 4.3	0.823	40.2	LOS C	8.7	62.4	0.78	0.93	1.01	24.1
21a	L1	All MCs	61	0.0	61 0.0	* 0.823	61.7	LOS E	8.7	62.4	0.78	0.93	1.01	26.2
22	T1	All MCs	860	4.0	860 4.0	0.754	40.1	LOS C	22.3	161.5	0.97	0.86	0.99	15.4
23a	R1	All MCs	455	6.3	455 6.3	* 0.882	48.8	LOS D	23.3	171.8	1.00	1.04	1.20	13.6
Appro	ach		1548	4.6	1548 4.6	0.882	43.5	LOS D	23.3	171.8	0.95	0.93	1.06	16.4
North	Mille	r St (N)												
7a	L1	All MCs	982	24.7	98 24.1	0.097	6.1	LOS A	1.1	9.1	0.21	0.44	0.21	38.1
8	T1	All MCs	245	18.5	245 18.	0.405	17.1	LOS B	4.1	33.5	0.49	0.42	0.49	26.6
9	R2	All MCs	21	5.0	21 5.0	0.405	24.4	LOS B	3.1	24.6	0.48	0.45	0.48	27.7
9b	R3	All MCs	8 -	12.5	8 12.5	0.405	25.1	LOS B	3.1	24.6	0.48	0.45	0.48	19.8
Appro	ach		373 ⁻	19.2	373 19.2	0.405	14.8	LOS B	4.1	33.5	0.41	0.43	0.41	29.0
North	West:	Pacific H	wy (NW	/)										
28	T1	All MCs	274	54	274 54	0.405	29.1	LOSIC	5.1	37.7	0.69	0.56	0.69	29.2
29a	R1	All MCs	227	3.2	227 3.2	0.488	37.6	LOS C	9.9	71.5	0.82	0.77	0.82	22.8
Appro	ach		501	4.4	501 4.4	0.488	33.0	LOS C	9.9	71.5	0.75	0.65	0.75	26.0
A 11 \ 7	l. : . I .		0754	7.0	0754 74	0.000	07.0		00.0	474.0	0.04	0.70	0.00	10.0
All Ve	nicles		2754	7.9	2/54 /.9	0.882	37.9	LOSIC	23.3	171.8	0.84	0.79	0.90	19.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance						
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE BACK OF QUEUE [Ped Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver Speed

		ped/h	sec		ped	m			sec	m	m/sec
Sout	th: Miller St (S	S)									
P1	Full	544	51.9	LOS E	1.7	1.7	0.96	0.96	68.6	20.0	0.29
Sout	thEast: Pacifi	c Hwy (S	E)								
P5	Full	993	52.9	LOS E	3.2	3.2	0.98	0.98	69.6	20.0	0.29
Nort	h: Miller St (N	1)									
P3	Full	1648	54.5	LOS E	5.4	5.4	1.01	1.01	71.1	20.0	0.28
Nort	hWest: Pacifi	c Hwy (N	IW)								
P7	Full	758	52.4	LOS E	2.4	2.4	0.97	0.97	69.0	20.0	0.29
All P	Pedestrians	3943	53.3	LOS E	5.4	5.4	0.99	0.99	70.0	20.0	0.29

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: VIC01 [VIC01 Pacific Hwy / Berry St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 1206

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 100 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[lotal veh/h	HV J %	l lotal veh/h	HV J %	v/c	sec		[Veh. veh	Dist J m		Rate	Cycles	km/h
South	SouthEast: Pacific Hwy (SE)														
1	L2	All MCs	26	0.0	26	0.0	0.057	15.1	LOS B	2.4	17.5	0.74	0.37	0.74	31.6
2	T1	All MCs	767	5.5	767	5.5	0.283	3.4	LOS A	5.0	36.3	0.25	0.20	0.25	52.2
23b	R3	All MCs	200	4.2	200	4.2	*0.855	48.8	LOS D	7.6	55.0	1.00	0.92	1.17	13.9
Appro	ach		994	5.1	994	5.1	0.855	12.9	LOS A	7.6	55.0	0.41	0.35	0.45	37.9
North	Nest:	Pacific H	wy (NV	V)											
27a	L1	All MCs	436	6.8	436	6.8	0.153	7.4	LOS A	2.4	17.5	0.24	0.64	0.24	35.7
8	T1	All MCs	365	3.5	365	3.5	*0.461	17.9	LOS B	11.0	79.5	0.76	0.68	0.76	22.5
Appro	ach		801	5.3	801	5.3	0.461	12.2	LOS A	11.0	79.5	0.48	0.66	0.48	28.2
South	West:	Berry St	(SW)												
10	L2	All MCs	38	2.8	38	2.8	0.066	4.0	LOS A	0.2	1.4	0.11	0.49	0.11	38.1
Appro	ach		38	2.8	38	2.8	0.066	4.0	LOS A	0.2	1.4	0.11	0.49	0.11	38.1
All Ve	hicles		1833	5.1	1833	5.1	0.855	12.4	LOS A	11.0	79.5	0.43	0.49	0.45	34.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	/ement	Perforr	nance							
Mo∖ ID	, Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
		ped/h	sec		ped	m		Trate	sec	m	m/sec
Sou	thEast: Pacifi	c Hwy (S	E)								
P1	Full	228	43.7	LOS E	0.6	0.6	0.94	0.94	60.3	20.0	0.33
Eas	t: Berry St (E)										
P2	Full	171	43.6	LOS E	0.5	0.5	0.94	0.94	210.2	200.0	0.95
Nor	thWest: Pacifi	c Hwy (N	1W)								
P3E	Slip/ Bypass	1	43.2	LOS E	0.0	0.0	0.93	0.93	59.9	20.0	0.33
Sou	thWest: Berry	St (SW))								
P4	Full	217	22.1	LOS C	0.4	0.4	0.89	0.89	38.8	20.0	0.52

All Pedestrians	617	36.1	LOS D	0.6	0.6	0.92	0.92	94.2	69.8	0.74
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Site: VIC02 [VIC02 Miller St / Berry St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 874

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 100 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total I veb/b	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Mille	er St (S)													
2	T1	All MCs	361 ⁻	12.8	361	12.8	0.360	12.6	LOS A	9.7	75.3	0.60	0.52	0.60	23.5
3	R2	All MCs	248	5.1	248	5.1	*0.639	28.6	LOS C	7.1	51.7	0.88	0.89	0.88	20.7
Appro	ach		609	9.7	609	9.7	0.639	19.1	LOS B	9.7	75.3	0.71	0.67	0.71	21.9
North	Mille	r St (N)													
7	L2	All MCs	192	3.8	192	3.8	*0.826	56.2	LOS D	10.2	73.8	1.00	0.98	1.25	14.9
8	T1	All MCs	175	9.6	175	9.6	0.409	35.0	LOS C	7.3	55.3	0.89	0.73	0.89	14.2
Appro	ach		366	6.6	366	6.6	0.826	46.1	LOS D	10.2	73.8	0.95	0.86	1.08	14.6
West:	Berry	/ St (W)													
10	L2	All MCs	125	10.1	125	10.1	0.481	50.4	LOS D	9.3	69.2	1.00	0.84	1.00	8.0
11	T1	All MCs	565	2.4	565	2.4	0.481	38.0	LOS C	12.8	91.6	0.99	0.82	0.99	15.7
12	R2	All MCs	364	47.1	36	47.1	*0.481	53.8	LOS D	11.5	86.9	1.00	0.83	1.00	8.7
Appro	ach		726	5.9	726	5.9	0.481	40.9	LOS C	12.8	91.6	0.99	0.82	0.99	14.3
All Ve	hicles	;	1702	7.4	1702	7.4	0.826	34.2	LOS C	12.8	91.6	0.88	0.78	0.91	16.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	ement	Perform	nance							
Mo∿ ⊓	, Crossing	Dem.	Aver. Delav	Level of	AVERAGE		Prop.	Eff. Stop	Travel Time	Travel Dist	Aver.
			Delay	Cervice	[Ped	Dist]	Que	Rate	TIME	Dist.	
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Miller St (S)									
P1	Full	398	44.0	LOS E	1.1	1.1	0.95	0.95	60.6	20.0	0.33
Eas	t: Berry St (E)										
P2	Full	274	43.7	LOS E	0.7	0.7	0.94	0.94	60.4	20.0	0.33
Nor	th: Miller St (N)	1									
P3	Full	252	43.7	LOS E	0.7	0.7	0.94	0.94	60.4	20.0	0.33
Wes	st: Berry St (W))									
P4	Full	677	44.5	LOS E	1.8	1.8	0.96	0.96	61.2	20.0	0.33

All Pedestrians	1600	44.1	LOS E	1.8	1.8	0.95	0.95	60.8	20.0	0.33
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Project: C:\Users\WanJ2\OneDrive - AECOM\General - ANZ-NAC-Sydney Metro-Sydney Metro C&SW Operational Monitoring\400_Technical \432_Traffic Analysis\SIDRA Modelling\04 Block 4\02 SIDRA Models with volumes\03 SM C&SW_VIC (Block 4).sip9

Site: VIC03 [VIC03 Miller St / McLaren St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 1156

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 75 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov	Turn	Mov	Dem	hand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
		Class	Total veh/h	HV] %	Total veh/h	HV] %	Saur v/c	sec	Service	[Veh. veh	Dist] m	Que	Rate	Cycles	speed km/h
South	: Mille	er St (S)													
1	L2	All MCs	109	3.8	109	3.8	0.290	14.7	LOS B	5.3	40.5	0.60	0.59	0.60	32.0
2	T1	All MCs	339	14.9	339	14.9	0.290	10.2	LOS A	5.3	40.5	0.60	0.56	0.60	32.5
3	R2	All MCs	38	11.1	38	11.1	0.290	21.2	LOS B	4.8	37.5	0.60	0.54	0.60	27.1
Appro	ach		486	12.1	486	12.1	0.290	12.1	LOS A	5.3	40.5	0.60	0.57	0.60	32.0
East:	McLa	ren St (E))												
4	L2	All MCs	38	0.0	38	0.0	0.255	42.3	LOS C	1.4	9.8	0.97	0.72	0.97	11.6
5	T1	All MCs	98	0.0	98	0.0	*0.269	28.9	LOS C	3.2	22.1	0.90	0.70	0.90	25.3
Appro	Approach			0.0	136	0.0	0.269	32.7	LOS C	3.2	22.1	0.92	0.71	0.92	21.7
North:	Mille	r St (N)													
7	L2	All MCs	57	1.9	57	1.9	0.197	19.2	LOS B	3.4	26.2	0.58	0.62	0.58	19.1
8	T1	All MCs	317	16.6	317	16.6	0.476	12.5	LOS A	7.7	59.1	0.68	0.68	0.68	24.6
9	R2	All MCs	98	0.0	98	0.0	*0.476	25.9	LOS B	7.7	59.1	0.73	0.72	0.73	31.3
Appro	ach		472	11.4	472	11.4	0.476	16.1	LOS B	7.7	59.1	0.68	0.68	0.68	25.6
West:	McLa	aren St (V	/)												
10	L2	All MCs	79	4.0	79	4.0	0.300	35.9	LOS C	2.7	19.3	0.92	0.76	0.92	23.1
11	T1	All MCs	64	1.6	64	1.6	0.440	30.0	LOS C	3.6	25.4	0.96	0.82	0.96	20.6
12	R2	All MCs	41	2.6	41	2.6	*0.440	38.1	LOS C	3.6	25.4	0.96	0.82	0.96	17.7
Appro	ach		184	2.9	184	2.9	0.440	34.3	LOS C	3.6	25.4	0.94	0.79	0.94	21.3
All Ve	hicles		1278	9.2	1278	9.2	0.476	19.0	LOS B	7.7	59.1	0.71	0.66	0.71	26.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mo	vement	Perform	nance							
Mov	/ Crossina	Dem.	Aver.	Level of	AVERAGE		Prop.	Eff.	Travel	Travel	Aver.
שו	Creecing	FIOW	Delay	Service	[Ped	Dist]	Que	Rate	Time	DISI.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Miller St (S)									
P1	Full	273	31.2	LOS D	0.5	0.5	0.92	0.92	47.8	20.0	0.42
Eas	t: McLaren St	:(E)									

P2 Full	439	31.4	LOS D	0.9	0.9	0.92	0.92	48.1	20.0	0.42
North: Miller St (N)									
P3 Full	414	31.4	LOS D	0.8	0.8	0.92	0.92	48.0	20.0	0.42
West: McLaren St	(W)									
P4 Full	404	31.4	LOS D	0.8	0.8	0.92	0.92	48.0	20.0	0.42
All Pedestrians	1529	31.3	LOS D	0.9	0.9	0.92	0.92	48.0	20.0	0.42

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Project: C:\Users\WanJ2\OneDrive - AECOM\General - ANZ-NAC-Sydney Metro-Sydney Metro C&SW Operational Monitoring\400_Technical \432_Traffic Analysis\SIDRA Modelling\04 Block 4\02 SIDRA Models with volumes\03 SM C&SW_VIC (Block 4).sip9

Site: VIC04 [VIC04 Pacific Hwy / Miller St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 630

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 100 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfo	orma	ince										
Mov	Turn	Mov	Dem	nand	Arri	ival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	Fi [Total	IOWS HV 1	FIO Total H	ws IV 1	Sath	Delay	Service	[Veh	Dist 1	Que	Stop Rate	NO. OF Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		11010		km/h
South	: Mille	er St (S)													
1	L2	All MCs	5	0.0	5	0.0	0.467	27.3	LOS B	6.6	53.0	0.92	0.77	0.92	21.1
1a	L1	All MCs	97	17.4	97 1	7.4	0.467	32.0	LOS C	6.6	53.0	0.92	0.77	0.92	12.8
2	T1	All MCs	246	16.7	246 1	6.7	0.778	48.1	LOS D	11.3	89.8	0.98	0.92	1.11	11.7
3b	R3	All MCs	23	13.6	23 1	3.6	*0.778	61.4	LOS E	11.3	89.8	1.00	0.96	1.16	18.7
Appro	ach		372	16.4	372 1	6.4	0.778	44.4	LOS D	11.3	89.8	0.97	0.88	1.06	12.7
South	East:	Pacific H	wy (SE)											
21b	L3	All MCs	146	7.2	146	7.2	0.607	17.5	LOS B	3.6	26.7	0.59	0.76	0.59	35.9
21a	L1	All MCs	57	1.9	57	1.9	*0.607	28.5	LOS C	3.6	26.7	0.59	0.76	0.59	36.7
22	T1	All MCs	680	3.6	680	3.6	0.488	26.1	LOS B	12.8	92.0	0.82	0.71	0.82	20.8
23a	R1	All MCs	363	4.9	363 4	4.9	*0.799	36.1	LOS C	12.7	93.0	0.99	0.97	1.11	17.1
Appro	ach		1246	4.3	1246	4.3	0.799	28.1	LOS B	12.8	93.0	0.83	0.79	0.87	22.2
North	Mille	r St (N)													
7a	L1	All MCs	60	35.1	60 3	5.1	0.072	7.7	LOS A	0.7	6.8	0.27	0.47	0.27	36.6
8	T1	All MCs	120	7.9	120	7.9	0.294	12.0	LOS A	1.8	13.4	0.36	0.31	0.36	29.6
9	R2	All MCs	13	8.3	13	8.3	0.294	18.7	LOS B	0.8	6.0	0.36	0.49	0.36	28.7
9b	R3	All MCs	18	11.8	18 1	1.8	0.294	19.4	LOS B	0.8	6.0	0.36	0.49	0.36	21.0
Appro	ach		211	16.0	211 1	6.0	0.294	11.8	LOS A	1.8	13.4	0.33	0.38	0.33	30.7
North	West:	Pacific H	wy (NV	V)											
28	T1	All MCs	216	4.9	216	4.9	0.252	22.1	LOS B	3.1	22.4	0.61	0.48	0.61	33.3
29a	R1	All MCs	180	1.8	180	1.8	0.505	52.4	LOS D	8.9	63.2	1.00	0.86	1.00	18.8
Appro	ach		396	3.5	396	3.5	0.505	35.9	LOS C	8.9	63.2	0.79	0.65	0.79	24.8
All Ve	hicles		2224	7.3	2224	7.3	0.799	30.7	LOS C	12.8	93.0	0.80	0.74	0.83	21.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec					

South: Miller St (S	S)									
P1 Full	587	44.3	LOS E	1.6	1.6	0.95	0.95	61.0	20.0	0.33
SouthEast: Pacifi	c Hwy (S	E)								
P5 Full	458	44.1	LOS E	1.2	1.2	0.95	0.95	60.8	20.0	0.33
North: Miller St (N	V)									
P3 Full	992	45.1	LOS E	2.7	2.7	0.97	0.97	61.8	20.0	0.32
NorthWest: Pacifi	ic Hwy (N	W)								
P7 Full	708	44.6	LOS E	1.9	1.9	0.96	0.96	61.2	20.0	0.33
All Pedestrians	2745	44.6	LOS E	2.7	2.7	0.96	0.96	61.3	20.0	0.33

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Site: VIC01 [VIC01 Pacific Hwy / Berry St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 1206

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	East:	Pacific H	lwy (SE)											
1	L2	All MCs	18	0.0	18	0.0	0.049	4.7	LOS A	0.3	1.9	0.10	0.20	0.10	42.2
2	T1	All MCs	669	2.7	669	2.7	0.230	1.2	LOS A	1.6	11.4	0.10	0.10	0.10	56.9
23b	R3	All MCs	92	2.3	92	2.3	*0.649	54.2	LOS D	4.3	30.5	1.00	0.82	1.09	12.8
Appro	Approach 779 2.6 779						0.649	7.5	LOS A	4.3	30.5	0.21	0.19	0.22	45.1
North	West:	Pacific H	łwy (NV	V)											
27a	L1	All MCs	457	3.2	457	3.2	0.161	8.0	LOS A	2.6	18.8	0.28	0.65	0.28	34.6
8	T1	All MCs	348	0.9	348	0.9	0.325	11.5	LOS A	8.2	57.9	0.58	0.50	0.58	29.0
Appro	ach		805	2.2	805	2.2	0.325	9.5	LOS A	8.2	57.9	0.41	0.59	0.41	31.9
South	West	Berry St	: (SW)												
10	L2	All MCs	34	0.0	34	0.0	0.024	4.4	LOS A	0.1	0.7	0.14	0.50	0.14	38.1
Appro	ach		34	0.0	34	0.0	0.024	4.4	LOS A	0.1	0.7	0.14	0.50	0.14	38.1
All Ve	hicles		1618	2.3	1618	2.3	0.649	8.4	LOS A	8.2	57.9	0.31	0.39	0.31	39.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	vement	Perform	nance							
Mo∨ ID	, Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec
Sou	thEast: Pacifi	c Hwy (S	SE)								
P1	Full	122	38.5	LOS D	0.3	0.3	0.93	0.93	55.1	20.0	0.36
Eas	t: Berry St (E)										
P2	Full	153	38.5	LOS D	0.4	0.4	0.93	0.93	205.2	200.0	0.97
Nor	thWest: Pacifi	c Hwy (N	W)								
P3E	Slip/ Bypass	1	38.3	LOS D	0.0	0.0	0.92	0.92	54.9	20.0	0.36
Sou	thWest: Berry	St (SW))								
P4	Full	142	1.6	LOS A	0.0	0.0	0.27	0.27	18.3	20.0	1.09

All Pedestrians	418	26.0	LOS C	0.4	0.4	0.70	0.70	97.4	85.7	0.88
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Site: VIC02 [VIC02 Miller St / Berry St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 874

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	· Mille	er St (S)	veh/h	%	veh/h	%	V/C	sec	_	veh	m	_	_	_	km/h
0	. mine		000	F 0	000	F 0	0.000	47.0		0.4	00.4	0.70	0.05	0.70	40.0
2	11		293	5.0	293	5.0	0.682	17.2	LOSB	9.4	69.1	0.76	0.65	0.76	18.9
3	R2	All MCs	199	5.3	199	5.3	*0.682	34.3	LOS C	9.4	69.1	0.88	0.93	0.91	20.2
Appro	ach		492	5.1	492	5.1	0.682	24.1	LOS B	9.4	69.1	0.81	0.76	0.82	19.6
North	Mille	r St (N)													
7	L2	All MCs	119	1.8	119	1.8	0.839	60.6	LOS E	5.9	42.0	1.00	1.02	1.38	15.0
8	T1	All MCs	211	5.5	211	5.5	0.609	40.8	LOS C	8.7	63.4	0.96	0.80	0.96	13.8
Appro	ach		329	4.2	329	4.2	0.839	48.0	LOS D	8.7	63.4	0.98	0.87	1.11	13.5
West:	Berry	st (W)													
10	L2	All MCs	82	1.3	82	1.3	0.192	37.8	LOS C	3.3	23.1	0.94	0.76	0.94	9.2
11	T1	All MCs	505	1.3	505	1.3	0.431	30.3	LOS C	10.5	74.6	0.93	0.78	0.93	18.2
12	R2	All MCs	25	37.5	25	37.5	*0.431	44.1	LOS D	10.1	73.6	0.95	0.79	0.95	10.5
Appro	ach		613	2.7	613	2.7	0.431	31.9	LOS C	10.5	74.6	0.93	0.78	0.93	16.9
All Ve	hicles		1434	3.9	1434	3.9	0.839	32.9	LOS C	10.5	74.6	0.90	0.79	0.94	16.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedest	rian Movemer	nt Perfor	mance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
	FIOW	Delay	Service	[Ped	Dist]	Que	Rate	Time	DISI.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: M	liller St (S)									
P1 Full	232	38.6	LOS D	0.6	0.6	0.93	0.93	55.3	20.0	0.36
East: Be	rry St (E)									
P2 Full	337	38.8	LOS D	0.8	0.8	0.94	0.94	55.5	20.0	0.36
North: M	liller St (N)									
P3 Full	145	38.5	LOS D	0.3	0.3	0.93	0.93	55.2	20.0	0.36
West: Be	erry St (W)									
P4 Full	348	38.8	LOS D	0.8	0.8	0.94	0.94	55.5	20.0	0.36

All Pedestrians	1062	38.7	LOS D	0.8	0.8	0.93	0.93	55.4	20.0	0.36
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Site: VIC03 [VIC03 Miller St / McLaren St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 1156

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	hand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
		Class	٦ [Total veh/h	HV] %	Total veh/h	HV] %	v/c	sec	Service	[Veh. veh	Dist] m	Que	Rate	Cycles	speed km/h
South	: Mille	er St (S)													
1	L2	All MCs	71	1.5	71	1.5	0.063	9.4	LOS A	1.2	8.2	0.38	0.57	0.38	34.3
2	T1	All MCs	256	5.3	256	5.3	0.317	9.5	LOS A	6.8	49.3	0.55	0.51	0.55	33.2
3	R2	All MCs	48	2.2	48	2.2	0.317	19.1	LOS B	6.8	49.3	0.55	0.51	0.55	27.5
Appro	ach		375	4.2	375	4.2	0.317	10.7	LOS A	6.8	49.3	0.52	0.52	0.52	32.8
East:	McLa	ren St (E))												
4	L2	All MCs	40	0.0	40	0.0	0.485	54.8	LOS D	1.9	13.3	1.00	0.74	1.02	9.4
5	T1	All MCs	78	1.4	78	1.4	*0.331	40.5	LOS C	3.3	23.1	0.96	0.73	0.96	21.1
Appro	Approach			0.9	118	0.9	0.485	45.3	LOS D	3.3	23.1	0.97	0.74	0.98	17.3
North	Mille	r St (N)													
7	L2	All MCs	73	0.0	73	0.0	0.080	16.3	LOS B	1.6	11.6	0.52	0.66	0.52	18.3
8	T1	All MCs	267	5.5	267	5.5	0.402	11.7	LOS A	8.6	62.5	0.60	0.65	0.60	26.4
9	R2	All MCs	87	0.0	87	0.0	*0.402	21.0	LOS B	8.6	62.5	0.60	0.65	0.60	33.7
Appro	ach		427	3.4	427	3.4	0.402	14.4	LOS A	8.6	62.5	0.59	0.65	0.59	26.1
West:	McLa	aren St (V	V)												
10	L2	All MCs	48	0.0	48	0.0	0.147	39.3	LOS C	1.8	12.8	0.88	0.73	0.88	22.2
11	T1	All MCs	66	1.6	66	1.6	0.330	30.6	LOS C	3.4	23.8	0.93	0.73	0.93	20.7
12	R2	All MCs	23	0.0	23	0.0	*0.330	38.4	LOS C	3.4	23.8	0.93	0.73	0.93	17.8
Appro	Approach			0.8	138	0.8	0.330	34.9	LOS C	3.4	23.8	0.91	0.73	0.91	20.9
All Ve	hicles		1058	3.1	1058	3.1	0.485	19.2	LOS B	8.6	62.5	0.65	0.62	0.65	26.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pede	strian Mo	/ement	Perform	nance							
Mov	Crossina	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
	Jiessing	FIOW	Delay	Service	[Ped	Dist]	Que	Rate	Time	DISI.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
South	n: Miller St (S	5)									
P1 F	Full	75	38.4	LOS D	0.2	0.2	0.93	0.93	55.1	20.0	0.36
East:	McLaren St	(E)									

P2 Full	307	38.8	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
North: Miller St (N)										
P3 Full	142	38.5	LOS D	0.3	0.3	0.93	0.93	55.2	20.0	0.36
West: McLaren St	(W)									
P4 Full	161	38.5	LOS D	0.4	0.4	0.93	0.93	55.2	20.0	0.36
All Pedestrians	685	38.6	LOS D	0.7	0.7	0.93	0.93	55.3	20.0	0.36

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Site: VIC04 [VIC04 Pacific Hwy / Miller St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [VIC Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 630

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
U		Class	FI [Total	IOWS	FI [Total]	IOWS	Sath	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	NO. Of Cycles	Speed
			veh/h	<u>%</u>	veh/h	%	v/c	sec		veh	m		rtato	Cycles	km/h
South	: Mille	er St (S)													
1	L2	All MCs	1	0.0	1	0.0	0.344	23.0	LOS B	4.9	37.5	0.88	0.74	0.88	22.8
1a	L1	All MCs	97	13.0	97	13.0	0.344	26.2	LOS B	4.9	37.5	0.88	0.74	0.88	14.4
2	T1	All MCs	202	6.8	202	6.8	*0.573	39.8	LOS C	7.5	55.8	0.95	0.78	0.95	13.6
3b	R3	All MCs	13	8.3	13	8.3	0.573	46.9	LOS D	7.5	55.8	0.96	0.79	0.96	21.4
Appro	ach		313	8.8	313	8.8	0.573	35.8	LOS C	7.5	55.8	0.93	0.77	0.93	14.3
South	East:	Pacific H	wy (SE)											
21b	L3	All MCs	61	6.9	61	6.9	0.133	11.2	LOS A	1.8	12.9	0.64	0.68	0.64	32.4
21a	L1	All MCs	12	0.0	12	0.0	0.133	35.1	LOS C	1.8	12.9	0.64	0.68	0.64	33.8
22	T1	All MCs	579	1.6	579	1.6	* 0.507	29.1	LOS C	10.4	73.8	0.88	0.75	0.88	19.6
23a	R1	All MCs	289	4.0	289	4.0	*0.676	39.2	LOS C	12.0	86.6	0.97	0.84	0.99	16.1
Appro	ach		941	2.7	941	2.7	0.676	31.1	LOS C	12.0	86.6	0.89	0.77	0.89	19.4
North	Mille	r St (N)													
7a	L1	All MCs	64	18.0	64	18.0	0.086	6.1	LOS A	0.4	3.6	0.17	0.42	0.17	38.2
8	T1	All MCs	149	6.3	149	6.3	0.478	32.4	LOS C	5.0	36.5	0.87	0.69	0.87	20.6
9	R2	All MCs	4	0.0	4	0.0	0.478	44.9	LOS D	5.0	36.5	0.90	0.73	0.90	22.3
9b	R3	All MCs	18	0.0	18	0.0	0.478	44.5	LOS D	5.0	36.5	0.90	0.73	0.90	13.9
Appro	ach		236	8.9	236	8.9	0.478	26.4	LOS B	5.0	36.5	0.68	0.62	0.68	23.3
North	West:	Pacific H	wy (NV	V)											
28	T1	All MCs	176	1.8	176	1.8	0.159	13.7	LOS A	1.7	11.8	0.45	0.35	0.45	40.1
29a	R1	All MCs	183	0.6	183	0.6	0.413	45.1	LOS D	8.1	56.9	1.00	0.85	1.00	20.6
Appro	ach		359	1.2	359	1.2	0.413	29.7	LOS C	8.1	56.9	0.73	0.61	0.73	27.2
All Ve	hicles		1848	4.2	1848	4.2	0.676	31.0	LOS C	12.0	86.6	0.84	0.72	0.84	20.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	nod/h			[Ped Dist]			Rate		m	mlaaa
	peu/n	sec		pea	m			sec	m	m/sec

South: Miller St (S)									
P1 Full	148	38.5	LOS D	0.4	0.4	0.93	0.93	55.2	20.0	0.36
SouthEast: Pacific	Hwy (Sl	E)								
P5 Full	216	38.6	LOS D	0.5	0.5	0.93	0.93	55.3	20.0	0.36
North: Miller St (N))									
P3 Full	459	39.0	LOS D	1.1	1.1	0.94	0.94	55.7	20.0	0.36
NorthWest: Pacific	Hwy (N	W)								
P7 Full	166	38.5	LOS D	0.4	0.4	0.93	0.93	55.2	20.0	0.36
All Pedestrians	989	38.8	LOS D	1.1	1.1	0.93	0.93	55.4	20.0	0.36

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V Site: BGU01 [BGU01 Hickson Rd / Towns PI (Site Folder: Block 4 - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Site Category: (None) Give-Way (Two-Way)

Vehic	cle M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV] veh/h %	Arrival Flows [Total HV] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
East:	Hicks	on Rd (E)										
4a	L1	All MCs	95 13.3	95 13.3	0.147	4.3	LOS A	0.6	4.9	0.38	0.53	0.38	34.8
6a	R1	All MCs	56 13.2	56 13.2	0.147	6.3	LOS A	0.6	4.9	0.38	0.53	0.38	34.8
Appro	ach		151 13.3	151 13.3	0.147	5.0	NA	0.6	4.9	0.38	0.53	0.38	34.8
North	West:	Towns P	I (NW)										
27a	L1	All MCs	82 11.5	82 11.5	0.246	5.1	LOS A	0.9	7.0	0.56	0.74	0.59	33.8
29	R2	All MCs	91 7.0	91 7.0	0.246	8.5	LOS A	0.9	7.0	0.56	0.74	0.59	24.1
Appro	ach		173 9.1	173 9.1	0.246	6.9	LOS A	0.9	7.0	0.56	0.74	0.59	31.2
South	West	Hickson	Rd (SW)										
30	L2	All MCs	85 11.1	85 11.1	0.205	5.3	LOS A	1.0	7.6	0.35	0.47	0.35	33.1
32a	R1	All MCs	173 11.6	173 11.6	0.205	3.1	LOS A	1.0	7.6	0.35	0.47	0.35	36.6
Appro	ach		258 11.4	258 11.4	0.205	3.8	NA	1.0	7.6	0.35	0.47	0.35	36.0
All Ve	hicles		581 11.2	581 11.2	0.246	5.0	NA	1.0	7.6	0.42	0.57	0.43	34.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: BGU02 [BGU02 Dalgety Rd / Towns PI (Site Folder: Block 4 - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Site Category: (None) Roundabout

Vehi	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Ba [Veh. veh	ck Of Que Dist] m	ue Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Dalg	ety Rd (S	S)												
30	L2	All MCs	31	0.0	31	0.0	0.138	6.1	LOS A	0.8	6.0	0.14	0.57	0.14	24.4
3b	R3	All MCs	161	9.2	161	9.2	0.138	6.4	LOS A	0.8	6.0	0.14	0.57	0.14	32.0
32u	U	All MCs	1	0.0	1	0.0	0.138	7.0	LOS A	0.8	6.0	0.14	0.57	0.14	34.6
Appro	bach		193	7.7	193	7.7	0.138	6.4	LOS A	0.8	6.0	0.14	0.57	0.14	30.3
South	East:	Towns Pl	(SE)												
21b	L3	All MCs	115	13.8	115	13.8	0.092	2.7	LOS A	0.5	4.2	0.04	0.49	0.04	35.1
21a	L1	All MCs	17	6.3	17	6.3	0.092	8.2	LOS A	0.5	4.2	0.04	0.49	0.04	18.8
23u	U	All MCs	9	0.0	9	0.0	0.092	6.9	LOS A	0.5	4.2	0.04	0.49	0.04	29.7
Appro	bach		141	11.9	141	11.9	0.092	3.6	LOS A	0.5	4.2	0.04	0.49	0.04	33.3
West	Parki	ng Acces	s (W)												
12a	R1	All MCs	2	50.0	2	50.0	0.004	1.5	LOS A	0.0	0.2	0.38	0.16	0.38	9.6
29	R2	All MCs	1	0.0	1	0.0	0.004	1.0	LOS A	0.0	0.2	0.38	0.16	0.38	21.2
29u	U	All MCs	1	0.0	1	0.0	0.004	1.0	LOS A	0.0	0.2	0.38	0.16	0.38	9.8
Appro	bach		4	25.0	4	25.0	0.004	1.2	LOS A	0.0	0.2	0.38	0.16	0.38	13.5
All Ve	hicles		338	9.7	338	9.7	0.138	5.1	LOS A	0.8	6.0	0.10	0.53	0.10	31.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: BGU03 [BGU03 Kent St / Argyle St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovement	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% E	ack Of	Prop.	Eff.	Aver.	Aver.
שו		Class	Flows [Total HV]	FIOWS [Total HV]	Sath	Delay	Service	Qu [Veh	eue Dist 1	Que	Stop Rate	NO. OT Cvcles	Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: Kent	: St (S)											
1	L2	All MCs	84 5.0	84 5.0	0.750	10.0	LOS A	7.5	55.6	0.81	1.32	1.84	31.5
2	T1	All MCs	35 6.1	35 6.1	0.750	12.3	LOS A	7.5	55.6	0.81	1.32	1.84	29.5
3	R2	All MCs	308 7.8	308 7.8	0.750	18.3	LOS B	7.5	55.6	0.81	1.32	1.84	30.0
Appro	ach		427 7.1	427 7.1	0.750	16.2	LOS B	7.5	55.6	0.81	1.32	1.84	30.3
East: /	Argyle	e St (E)											
4	L2	All MCs	279 3.0	279 3.0	0.349	4.9	LOS A	1.9	13.6	0.42	0.48	0.42	36.3
5	T1	All MCs	86 11.0	86 11.0	0.349	1.5	LOS A	1.9	13.6	0.42	0.48	0.42	35.8
6	R2	All MCs	8 25.0	8 25.0	0.349	5.0	LOS A	1.9	13.6	0.42	0.48	0.42	29.9
Approach 374			374 5.4	374 5.4	0.349	4.1	NA	1.9	13.6	0.42	0.48	0.42	36.1
North:	Kent	St (N)											
7	L2	All MCs	5 0.0	5 0.0	0.034	7.5	LOS A	0.1	0.8	0.49	0.92	0.49	26.4
8	T1	All MCs	14 0.0	14 0.0	0.034	11.0	LOS A	0.1	0.8	0.49	0.92	0.49	33.0
9	R2	All MCs	2 50.0	2 50.0	0.034	11.9	LOS A	0.1	0.8	0.49	0.92	0.49	29.6
Appro	ach		21 5.0	21 5.0	0.034	10.2	LOS A	0.1	0.8	0.49	0.92	0.49	31.7
West:	Argyl	e PI (W)											
10	L2	All MCs	4 0.0	4 0.0	0.143	5.2	LOS A	0.7	5.1	0.41	0.41	0.41	34.7
11	T1	All MCs	69 15.2	69 15.2	0.143	1.2	LOS A	0.7	5.1	0.41	0.41	0.41	36.4
12	R2	All MCs	65 8.1	65 8.1	0.143	6.0	LOS A	0.7	5.1	0.41	0.41	0.41	37.1
Appro	ach		139 11.4	139 11.4	0.143	3.6	NA	0.7	5.1	0.41	0.41	0.41	36.8
All Ve	hicles		961 7.0	961 7.0	0.750	9.5	NA	7.5	55.6	0.59	0.85	1.05	33.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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CCG MOVEMENT SUMMARY

□+□ Common Control Group: CCG1 [TCS 4272] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 AM Peak)]

EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (CCG User-Given Phase Times)

Vehi	cle M	ovemen	t Perfo	orma	nce (CC	G)								
Mov ID	Turn	Mov Class	Dem Fl [Total]	nand lows HV]	Arriva Flow [Total HV	al Deg. s Satn]	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
Site: I	BGU0	4 [BGU04	ven/n Pedes	% strian	Mid-block	° V/C Crossing	sec st Kent St	near Gas	l nl	m	-	-	_	Km/n
South	n Ken	t St	i i ouoc	, and an		Crocollig		nour ouo						
2	T1	All MCs	519	73	519 7	3 0.486	48	LOSA	7 1	52.6	0.38	0.33	0.38	36.2
Appro	bach		519	7.3	519 7.	3 0.486	4.8	LOSA	7.1	52.6	0.38	0.33	0.38	36.2
North	: Kent	St												
8	T1	All MCs	316	11.3	316 11.	3 0.555	34.7	LOS C	5.9	45.5	0.97	0.78	0.97	21.5
Appro	bach		316	11.3	316 11.	3 0.555	34.7	LOS C	5.9	45.5	0.97	0.78	0.97	21.5
All Ve	hicles	;	835	8.8	835 8.	8 0.555	16.1	LOS B	7.1	52.6	0.60	0.50	0.60	29.2
Site: I	BGU0	5 [BGU05	5 Kent S	St / S	ydney Ha	bour Bridg	e (SHB) (On-ramp]						
South	n: Ken	t St (S)												
2	T1	All MCs	560	4.1	560 4.	1 0.563	8.1	LOS A	11.4	82.7	0.56	0.50	0.56	28.6
3a	R1	All MCs	285	9.2	285 9.	2 * 0.456	28.7	LOS C	9.1	68.9	0.92	0.79	0.92	21.4
Appro	bach		845	5.9	845 5.	9 0.563	15.1	LOS B	11.4	82.7	0.68	0.60	0.68	24.9
East:	Clare	nce St (E))											
4	L2	All MCs	134	6.3	134 6.	3 0.324	31.2	LOS C	4.4	32.6	0.87	0.76	0.87	13.7
6	R2	All MCs	180	8.2	180 8.	2 *0.491	32.8	LOS C	6.3	47.0	0.91	0.79	0.91	13.2
Appro	bach		314	7.4	314 7.	4 0.491	32.1	LOS C	6.3	47.0	0.89	0.78	0.89	13.4
North	East:	SHB On-I	ramp (N	IE)										
24a	L1	All MCs	38	0.0	38 0.	0 0.037	30.5	LOS C	1.2	3.3	0.87	0.64	0.87	20.1
Appro	bach		38	0.0	38 0.	0 0.037	30.5	LOS C	1.2	3.3	0.87	0.64	0.87	20.1
North	: Kent	St (N)												
7b	L3	All MCs	139	5.3	139 5.	3 0.379	40.4	LOS C	5.5	40.4	1.00	0.84	1.00	12.9
8	T1	All MCs	83	34.2	83 34.	2 * 0.509	26.0	LOS B	2.6	23.9	0.79	0.61	0.79	7.5
Appro	bach		222	16.1	222 16.	1 0.509	35.0	LOS C	5.5	40.4	0.92	0.75	0.92	11.5
All Ve	hicles	;	1419	7.6	1419 7.	6 0.563	22.4	LOS B	11.4	82.7	0.77	0.66	0.77	19.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance (C	CG)					
Mov	Dem.	Aver.	Level of	AVERAGE BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUEUE	Que	Stop	Time	Dist.	Speed

				[Ped	Dist]		Rate			
	ped/h	sec		ped	m			sec	m	m/sec
Site: BGU04 [BGU	04 Pede	strian N	/lid-block Cro	ossing at K	ent St near	Gas Ln]				
South: Kent St										
P1 Full	299	33.7	LOS D	0.6	0.6	0.92	0.92	200.4	200.0	1.00
All Pedestrians	299	33.7	LOS D	0.6	0.6	0.92	0.92	200.4	200.0	1.00
Site: BGU05 [BGU	105 Kent	St / Syd	dney Harbou	ır Bridge (S	SHB) On-ram	ıp]				
South: Kent St (S)										
P1 Full	2	29.8	LOS C	0.0	0.0	0.86	0.86	46.4	20.0	0.43
All Pedestrians	2	29.8	LOS C	0.0	0.0	0.86	0.86	46.4	20.0	0.43

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Site: BGU06 [BGU06 Hickson Rd / Napoleon St / Sussex St (Site Folder: Block 4 - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 4625

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 75 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[lotal veh/h	HV J %	[Iotal∶ veh/h	HV J %	v/c	sec		[Veh. veh	Dist J m		Rate	Cycles	km/h
South	Sus	sex St (S))												
2	T1	All MCs	337	6.3	337	6.3	0.336	10.1	LOS A	6.8	50.4	0.59	0.51	0.59	30.4
3	R2	All MCs	151	3.5	151	3.5	*0.347	18.3	LOS B	3.6	26.0	0.79	0.74	0.79	22.3
Appro	ach		487	5.4	487	5.4	0.347	12.7	LOS A	6.8	50.4	0.65	0.58	0.65	27.8
East: I	Napol	ean St (E)												
4	L2	All MCs	197	18.2	197	18.2	0.352	22.9	LOS B	5.4	43.5	0.78	0.75	0.78	15.0
6	R2	All MCs	160	9.9	160	9.9	*0.420	30.9	LOS C	5.2	39.3	0.90	0.78	0.90	20.7
Appro	ach		357	14.5	357	14.5	0.420	26.5	LOS B	5.4	43.5	0.83	0.76	0.83	18.4
North:	Hick	son Rd (N	1)												
7	L2	All MCs	93	8.0	93	8.0	0.156	21.9	LOS B	2.4	17.6	0.73	0.70	0.73	23.7
8	T1	All MCs	247	9.8	247	9.8	*0.360	18.2	LOS B	6.5	49.4	0.76	0.64	0.76	21.9
Appro	ach		340	9.3	340	9.3	0.360	19.2	LOS B	6.5	49.4	0.75	0.66	0.75	22.5
West:	Car F	Park Acce	ss (W)												
10	L2	All MCs	1	0.0	1	0.0	0.051	46.3	LOS D	0.1	0.4	1.00	0.57	1.00	11.3
11	T1	All MCs	2	0.0	2	0.0	*0.243	47.9	LOS D	0.3	1.9	1.00	0.63	1.00	8.2
12	R2	All MCs	4	0.0	4	0.0	0.243	48.0	LOS D	0.3	1.9	1.00	0.64	1.00	2.1
Appro	ach		7	0.0	7	0.0	0.243	47.8	LOS D	0.3	1.9	1.00	0.63	1.00	5.6
All Vel	nicles		1192	9.2	1192	9.2	0.420	18.9	LOS B	6.8	50.4	0.74	0.66	0.74	23.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pede	strian Mov	vement	Perform	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID C	crossing	Flow	Delay	Service	QUI	EUE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
South	: Sussex St	(S)									
P1 F	ull	86	30.9	LOS D	0.2	0.2	0.91	0.91	47.6	20.0	0.42
East: I	Napolean St	t (E)									
P2 F	ull	312	31.2	LOS D	0.6	0.6	0.92	0.92	47.9	20.0	0.42

North: Hickson Rd	(N)										
P3 Full	40	30.9	LOS D	0.1	0.1	0.91	0.91	47.5	20.0	0.42	
West: Car Park Access (W)											
P4 Full	161	31.0	LOS D	0.3	0.3	0.91	0.91	47.7	20.0	0.42	
All Pedestrians	599	31.1	LOS D	0.6	0.6	0.92	0.92	47.8	20.0	0.42	

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Site: BGU07 [BGU07 Margaret St / Kent St / Napoleon St (Site Folder: Block 4 - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

TCS 308

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfori	manc	9									
Mov	Turn	Mov	Dema	nd	Arrival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	/IOH Total H	WS V/1[Tc	Flows	Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	% ve	n/h %	v/c	sec		veh	m		Titato	Cycleo	km/h
South	: Kent	t St (S)												
1a	L1	All MCs	71 22	2.4	71 22.4	*0.674	34.2	LOS C	16.5	124.2	0.86	0.77	0.86	18.6
2	T1	All MCs	688 5	5.7 6	88 5.7	0.674	21.5	LOS B	16.5	124.2	0.89	0.78	0.91	7.8
3	R2	All MCs	42 2	2.5	42 2.5	0.674	67.0	LOS E	7.1	51.8	0.98	0.87	1.06	6.0
Appro	ach		801 7	7.0 8	01 7.0	0.674	25.0	LOS B	16.5	124.2	0.89	0.78	0.91	8.9
East:	Marga	aret St (E)											
4	L2	All MCs	84 3	3.8	84 3.8	0.290	38.3	LOS C	3.1	22.7	0.97	0.77	0.97	8.1
6a	R1	All MCs	233 14	4.9 2	33 14.9	0.744	29.1	LOS C	8.4	65.3	0.94	0.88	1.02	15.0
6	R2	All MCs	78 5	5.4	78 5.4	*0.744	33.0	LOS C	8.4	65.3	0.94	0.88	1.02	6.6
Appro	ach		395 10).7 3	95 10.7	0.744	31.8	LOS C	8.4	65.3	0.95	0.86	1.01	12.1
North	Kent	St (N)												
7	L2	All MCs	46 13	3.6	46 13.6	0.432	40.5	LOS C	5.9	43.8	0.82	0.70	0.82	18.3
8	T1	All MCs	240 2	2.6 2	40 2.6	0.432	25.5	LOS B	5.9	43.8	0.88	0.71	0.88	17.6
9b	R3	All MCs	71 9	9.0	71 9.0	0.346	25.3	LOS B	1.9	14.5	0.69	0.71	0.69	22.1
Appro	ach		357 5	5.3 3	57 5.3	0.432	27.4	LOS B	5.9	43.8	0.83	0.71	0.83	18.6
North	West:	Napoleo	n St (NW))										
27b	L3	All MCs	161 3	3.9 1	61 3.9	0.665	18.0	LOS B	7.1	52.7	0.84	0.91	0.89	16.8
27a	L1	All MCs	98 14	4.0	98 14.0	0.665	34.6	LOS C	7.1	52.7	0.84	0.91	0.89	16.8
Appro	ach		259 7	7.7 2	59 7.7	0.665	24.3	LOS B	7.1	52.7	0.84	0.91	0.89	16.8
All Ve	hicles		1812 7	7.6 18	12 7.6	0.744	26.8	LOS B	16.5	124.2	0.89	0.80	0.91	13.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance											
Mov Dem. Aver. Level of AVERAGE BACK OF Prop. Eff. Travel Travel Aver.														
ID Crossing	Flow	Delay	Service	QUEUE		Que	Stop	Time	Dist.	Speed				
				[Ped	Dist]		Rate							
	ped/h	sec		ped	m			sec	m	m/sec				
South: Kent St (S	5)													

P1 Full	1965	29.7	LOS C	4.0	4.0	0.90	0.90	46.3	20.0	0.43
East: Margaret St	t (E)									
P2 Full	152	31.7	LOS D	0.3	0.3	0.89	0.89	48.4	20.0	0.41
North: Kent St (N)									
P3 Full	338	26.8	LOS C	0.6	0.6	0.82	0.82	43.5	20.0	0.46
NorthWest: Napo	leon St (I	WV)								
P7 Full	1361	28.0	LOS C	2.7	2.7	0.86	0.86	194.7	200.0	1.03
All Pedestrians	3816	28.9	LOS C	4.0	4.0	0.88	0.88	99.1	84.2	0.85

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Site: BGU08 [BGU08 Margaret St / Clarence St (Site Folder: Block 4 - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 319

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network Site User-Given Phase Times)

Vehio	cle M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV] veh/h %	Arrival Flows [Total HV] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Clar	ence St (S	S)										
1	L2	All MCs	77 2.7	77 2.7	0.166	31.9	LOS C	3.0	24.1	0.84	0.72	0.84	13.4
2	T1	All MCs	569 35.7	569 35.7	*0.706	21.4	LOS B	11.9	83.5	0.81	0.71	0.83	19.2
3	R2	All MCs	2 ^{100.} 0	2 ^{100.} 0	*0.706	43.3	LOS D	11.9	83.5	0.85	0.77	0.89	15.7
Appro	ach		648 32.0	648 32.0	0.706	22.7	LOS B	11.9	83.5	0.81	0.71	0.84	18.4
East:	Marga	aret St (E))										
5	T1	All MCs	318 12.6	318 12.6	0.505	17.6	LOS B	6.3	48.6	0.75	0.64	0.75	9.3
6	R2	All MCs	75 78.9	75 78.9	0.505	37.3	LOS C	5.7	55.4	0.99	0.80	0.99	9.3
Appro	ach		393 25.2	393 25.2	0.505	21.4	LOS B	6.3	55.4	0.80	0.67	0.80	9.3
West:	Marg	aret St (V	V)										
10	L2	All MCs	91 11.6	91 11.6	*0.862	55.2	LOS D	8.3	63.1	1.00	1.05	1.32	6.9
11	T1	All MCs	95 10.0	95 10.0	0.862	42.2	LOS C	8.3	63.1	1.00	1.05	1.32	3.8
Appro	ach		185 10.8	185 10.8	0.862	48.5	LOS D	8.3	63.1	1.00	1.05	1.32	5.4
All Ve	hicles		1226 26.6	<mark>1247</mark> 26.2	0.862	25.8	LOS B	11.9	83.5	0.82	0.74	0.88	13.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	vement	Perforn	nance							
Mo	0	Dem.	Aver.	Level of	AVERAGE E	ACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUEL [Ped	JE Dist]	Que	Stop Rate	Time	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Clarence S	St (S)									
P1	Full	1717	34.9	LOS D	3.8	3.8	0.97	0.97	51.6	20.0	0.39
Eas	t: Margaret St	(E)									
P2	Full	445	33.0	LOS D	0.9	0.9	0.92	0.92	49.7	20.0	0.40
Nor	th: Clarence S	st (N)									
P3	Full	508	31.3	LOS D	1.0	1.0	0.89	0.89	48.0	20.0	0.42
Wes	st: Margaret S	t (W)									

P4 Full	937	29.2	LOS C	1.9	1.9	0.87	0.87	45.9	20.0	0.44
All Pedestrians	3607	32.7	LOS D	3.8	3.8	0.93	0.93	49.3	20.0	0.41

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Site: BGU09 [BGU09 Margaret St / York St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 3042 Site Cetegory

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network User-Given Cycle Time)

Vehic	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh	Of Queue Dist 1	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
East:	Marga	aret St (E)										
4	L2	All MCs	58 16.4	58 16.4	0.194	24.8	LOS B	2.5	22.3	0.76	0.68	0.76	17.8
5	T1	All MCs	89 63.5	89 63.5	0.194	21.6	LOS B	2.5	22.3	0.76	0.62	0.76	9.4
Appro	ach		147 45.0	147 45.0	0.194	22.8	LOS B	2.5	22.3	0.76	0.65	0.76	13.6
North:	York	St (N)											
8	T1	All MCs	766 31.5	766 31.5	0.336	12.4	LOS A	5.9	52.4	0.63	0.54	0.63	25.1
9	R2	All MCs	298 13.4	298 13.4	*0.407	17.0	LOS B	7.2	56.4	0.66	0.75	0.66	11.9
Appro	ach		1064 26.4	1064 26.4	0.407	13.7	LOS A	7.2	56.4	0.64	0.60	0.64	22.2
West:	Marg	aret St (V	V)										
12	R2	All MCs	75 11.3	<mark>62</mark> 13.7	0.170	35.6	LOS C	2.4	18.9	1.00	0.75	1.00	13.1
Appro	ach		75 11.3	<mark>62</mark> 13.7	0.170	35.6	LOS C	2.4	18.9	1.00	0.75	1.00	13.1
All Ve	hicles		1286 27.7	<mark>1273</mark> 27.9	0.407	15.8	LOS B	7.2	56.4	0.67	0.61	0.67	20.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mov	vement	Perforn	nance								
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed	
	ped/h	sec		ped	m		Trate	sec	m	m/sec	
South: York St (S))										
P1 Full	1448	29.9	LOS C	2.9	2.9	0.89	0.89	46.5	20.0	0.43	
East: Margaret St (E)											
P2 Full	1907	30.5	LOS D	3.9	3.9	0.91	0.91	47.1	20.0	0.42	
North: York St (N)											
P3 Full	884	27.4	LOS C	1.7	1.7	0.84	0.84	44.1	20.0	0.45	
West: Margaret S	t (W)										
P4 Full	606	31.4	LOS D	1.2	1.2	0.90	0.90	48.1	20.0	0.42	
All Pedestrians	4846	29.9	LOS C	3.9	3.9	0.89	0.89	46.5	20.0	0.43	

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Site: BGU10 [BGU10 Pedestrian Mid-block Crossing at Sussex St under Exchange PI (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 3939 (?)

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 70 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	and ows	Arri Flo	ival bws	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total l veh/h	HV] %	[Total H veh/h	IV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Sus	sex St (S))												
2	T1	All MCs	432	3.7	432	3.7	0.191	6.8	LOS A	3.3	23.9	0.48	0.40	0.48	26.7
Appro	ach		432	3.7	432	3.7	0.191	6.8	LOS A	3.3	23.9	0.48	0.40	0.48	26.7
North:	Suss	sex St (N)													
8	T1	All MCs	404	16.7	404 1	6.7	*0.202	6.8	LOS A	3.1	25.1	0.48	0.41	0.48	24.9
Appro	ach		404	16.7	404 1	6.7	0.202	6.8	LOS A	3.1	25.1	0.48	0.41	0.48	24.9
All Ve	hicles		836	9.9	836	9.9	0.202	6.8	LOS A	3.3	25.1	0.48	0.40	0.48	25.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
	ped/h	sec		[Ped ped	Dist J m		Rate	sec	m	m/sec				
South: Sussex St	(S)													
P1 Full	499	29.0	LOS C	0.9	0.9	0.92	0.92	45.6	20.0	0.44				
All Pedestrians	499	29.0	LOS C	0.9	0.9	0.92	0.92	45.6	20.0	0.44				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: BGU11 [BGU11 Pedestrian Mid-block Crossing at Kent St near Margaret St (Site Folder: Block 4 - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 4109

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehio	cle M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Kent St (S)															
2	T1	All MCs	719	8.8	719	8.8	*0.445	10.7	LOS A	5.2	39.7	0.76	0.63	0.76	21.5
Appro	ach		719	8.8	719	8.8	0.445	10.7	LOS A	5.2	39.7	0.76	0.63	0.76	21.5
North	Kent	St (N)													
8	T1	All MCs	266	1.6	266	1.6	0.255	9.6	LOS A	2.9	20.7	0.68	0.55	0.68	14.7
Appro	ach		266	1.6	266	1.6	0.255	9.6	LOS A	2.9	20.7	0.68	0.55	0.68	14.7
All Ve	hicles		985	6.8	985	6.8	0.445	10.4	LOS A	5.2	39.7	0.74	0.61	0.74	20.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		ped	m		Trate	sec	m	m/sec					
South: Kent St (S)														
P1 Full	2375	16.0	LOS B	2.7	2.7	0.89	0.89	32.6	20.0	0.61					
All Pedestrians	2375	16.0	LOS B	2.7	2.7	0.89	0.89	32.6	20.0	0.61					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: BGU12 [BGU12 Sussex St / Erskine St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 310

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ט ו		Class	۲۱ Total آ	IOWS HV 1	۲۱ Total آ	ows HV 1	Sath	Delay	Service	[Veh.	Dist 1	Que	Stop Rate	NO. OF Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- 5	km/h
South	: Sus	sex St (S)													
1	L2	All MCs	58	7.3	58	7.3	0.360	36.4	LOS C	5.7	42.0	0.87	0.73	0.87	15.1
2	T1	All MCs	286	4.0	286	4.0	*0.360	28.4	LOS B	6.7	48.4	0.85	0.71	0.85	15.7
Appro	ach		344	4.6	344	4.6	0.360	29.7	LOS C	6.7	48.4	0.86	0.71	0.86	15.6
East:	Erskir	ne St (E)													
4	L2	All MCs	474	3.3	474	3.3	0.528	10.5	LOS A	8.3	60.1	0.43	0.64	0.43	27.3
5	T1	All MCs	109	4.8	109	4.8	0.225	2.1	LOS A	0.8	5.8	0.12	0.20	0.12	28.7
6	R2	All MCs	38	19.4	38	19.4	0.225	6.5	LOS A	0.8	5.8	0.12	0.20	0.12	28.7
Appro	ach		621	4.6	621	4.6	0.528	8.8	LOS A	8.3	60.1	0.36	0.54	0.36	27.4
North:	Suss	ex St (N)													
7	L2	All MCs	43	51.2	43	51.2	0.110	26.3	LOS B	1.3	13.4	0.72	0.68	0.72	14.0
8	T1	All MCs	386	8.7	386	8.7	0.294	21.8	LOS B	6.0	45.4	0.75	0.63	0.75	23.7
9	R2	All MCs	21	20.0	21	20.0	*0.096	30.8	LOS C	0.7	5.8	0.84	0.69	0.84	12.5
Appro	ach		451	13.3	451	13.3	0.294	22.7	LOS B	6.0	45.4	0.75	0.63	0.75	22.5
West:	Erski	ne St (W)													
10	L2	All MCs	165	4.5	165	4.5	0.394	14.2	LOS A	9.6	70.7	0.61	0.61	0.61	12.0
11	T1	All MCs	222	7.1	222	7.1	0.394	13.2	LOS A	9.6	70.7	0.61	0.61	0.61	12.0
12	R2	All MCs	221	5.2	221	5.2	*0.563	24.7	LOS B	7.5	54.9	0.80	0.79	0.80	18.7
Appro	ach		608	5.7	608	5.7	0.563	17.6	LOS B	9.6	70.7	0.68	0.67	0.68	15.9
All Ve	hicles		2024	6.9	2024	6.9	0.563	18.1	LOS B	9.6	70.7	0.63	0.63	0.63	20.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec
South: Sussex S	t (S)									
P1 Full	224	38.6	LOS D	0.5	0.5	0.93	0.93	55.3	20.0	0.36

East: Erskine St (E)									
P2 Full	192	38.6	LOS D	0.5	0.5	0.93	0.93	55.2	20.0	0.36
North: Sussex St	(N)									
P3 Full	498	39.1	LOS D	1.2	1.2	0.94	0.94	55.7	20.0	0.36
West: Erskine St	(W)									
P4 Full	198	38.6	LOS D	0.5	0.5	0.93	0.93	55.3	20.0	0.36
All Pedestrians	1112	38.8	LOS D	1.2	1.2	0.94	0.94	55.5	20.0	0.36

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Site: BGU13 [BGU13 Kent St / Erskine St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 307

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar Fl	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
0			veh/h	%	veh/h	%	V/C	sec	_	veh	m	_	_	_	km/h
South	: Ken	(S)													
1	L2	All MCs	144	7.3	144	7.3	0.171	17.2	LOS B	3.5	26.1	0.59	0.67	0.59	21.3
2	T1	All MCs	592	8.4	592	8.4	*0.278	12.7	LOS A	6.2	47.2	0.58	0.49	0.58	23.3
3	R2	All MCs	3	0.0	3	0.0	0.029	7.0	LOS A	1.6	4.3	0.35	0.28	0.35	25.5
Appro	ach		739	8.1	739	8.1	0.278	13.6	LOS A	6.2	47.2	0.58	0.52	0.58	22.9
East:	Erskir	ne St (E)													
5	T1	All MCs	286	4.8	286	4.8	*0.452	35.6	LOS C	6.4	46.3	0.93	0.76	0.93	5.2
6	R2	All MCs	15	7.1	15	7.1	0.452	43.6	LOS D	5.7	41.7	0.93	0.76	0.93	5.2
Appro	ach		301	4.9	301	4.9	0.452	36.0	LOS C	6.4	46.3	0.93	0.76	0.93	5.2
North:	Kent	St (N)													
7	L2	All MCs	6	0.0	6	0.0	0.022	6.8	LOS A	1.3	3.6	0.34	0.28	0.34	22.6
8	T1	All MCs	92	0.0	92	0.0	0.022	4.9	LOS A	1.3	3.6	0.34	0.28	0.34	26.3
9	R2	All MCs	191	2.2	191	2.2	*0.940	65.7	LOS E	10.8	76.7	1.00	1.22	1.61	4.8
Appro	ach		288	1.5	288	1.5	0.940	45.1	LOS D	10.8	76.7	0.78	0.90	1.18	9.3
West:	Erski	ne St (W)												
10	L2	All MCs	97	7.6	97	7.6	0.422	33.7	LOS C	4.9	37.2	0.84	0.73	0.84	7.3
11	T1	All MCs	169	18.6	169	18.6	0.422	32.1	LOS C	5.0	40.3	0.87	0.71	0.87	10.2
Appro	ach		266	14.6	266	14.6	0.422	32.7	LOS C	5.0	40.3	0.86	0.72	0.86	9.2
All Ve	hicles		1595	7.4	1595	7.4	0.940	26.7	LOS B	10.8	76.7	0.73	0.67	0.80	13.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	/ement	Perform	nance							
Mo\ ID	′ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Kent St (S)									
P1	Full	398	38.9	LOS D	1.0	1.0	0.94	0.94	55.6	20.0	0.36
Eas	t: Erskine St (E)									

P2 Full	314	38.8	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
North: Kent St (N)										
P3 Full	409	38.9	LOS D	1.0	1.0	0.94	0.94	55.6	20.0	0.36
West: Erskine St ((W)									
P4 Full	524	39.1	LOS D	1.3	1.3	0.94	0.94	55.8	20.0	0.36
All Pedestrians	1645	39.0	LOS D	1.3	1.3	0.94	0.94	55.6	20.0	0.36

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Site: BGU14 [BGU14 Sussex St / King St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 4 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 284

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total veh/h	nand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
East:	King S	St (E)													
4a	L1	All MCs	27	0.0	27	0.0	0.064	44.8	LOS D	1.2	3.2	1.00	0.69	1.00	18.3
Appro	ach		27	0.0	27	0.0	0.064	44.8	LOS D	1.2	3.2	1.00	0.69	1.00	18.3
North:	Suss	ex St (N)													
7	L2	All MCs	78	14.9	78	14.9	0.658	31.6	LOS C	15.8	117.0	0.89	0.79	0.89	16.8
8	T1	All MCs	799	4.7	799	4.7	0.658	25.6	LOS B	16.6	120.7	0.89	0.78	0.89	24.1
Appro	ach		877	5.6	877	5.6	0.658	26.1	LOS B	16.6	120.7	0.89	0.78	0.89	23.6
South	West:	King St ((SW)												
30a	L1	All MCs	404	4.2	404	4.2	*0.462	17.3	LOS B	8.2	59.4	0.71	0.75	0.71	37.0
32a	R1	All MCs	1519	3.7	1519	3.7	*0.802	31.1	LOS C	26.7	193.4	0.91	0.87	0.96	27.1
32b	R3	All MCs	376	8.4	376	8.4	0.501	23.1	LOS B	11.1	83.0	0.71	0.80	0.71	33.1
Appro	ach		2299	4.5	2299	4.5	0.802	27.3	LOS B	26.7	193.4	0.85	0.84	0.88	28.8
All Ve	hicles		3203	4.8	3203	4.8	0.802	27.2	LOS B	26.7	193.4	0.86	0.82	0.88	27.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestriar	n Movement	Perforr	nance							
Mov D Crossir	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
	'9 FIOW	Delay	Service	[Ped	Dist]	Que	Rate	Time	DISI.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Suss	ex St (S)									
P1 Full	164	40.4	LOS E	0.4	0.4	0.95	0.95	57.1	20.0	0.35
East: King S	St (E)									
P2 Full	245	38.7	LOS D	0.6	0.6	0.93	0.93	55.3	20.0	0.36
North: Suss	ex St (N)									
P3 Full	440	39.0	LOS D	1.1	1.1	0.94	0.94	55.7	20.0	0.36
SouthWest:	King St (SW)									
P8 Full	288	38.7	LOS D	0.7	0.7	0.93	0.93	205.4	200.0	0.97

P8B Slip/ Bypass	212	40.5	LOS E	0.5	0.5	0.95	0.95	207.2	200.0	0.97
All Pedestrians	1349	39.3	LOS D	1.1	1.1	0.94	0.94	111.5	86.7	0.78

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Site: BGU15 [BGU15 Kent St / King St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 4 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 283

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov D	Turn	Mov Class	Derr F	nand	Ar Fl	rival	Deg. Satn	Aver. Delav	Level of Service	95% Back	Of Queue	Prop.	Eff. Stop	Aver.	Aver. Speed
		01000	[Total	HV]	[Total	HV]	Call	Delay	0011100	[Veh.	Dist]	Que	Rate	Cycles	opeca
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Ken	t St (S)													
1	L2	All MCs	3	0.0	3	0.0	0.219	46.4	LOS D	4.0	10.7	0.98	0.72	0.98	13.2
2	T1	All MCs	496	11.0	496	11.0	*0.623	38.5	LOS C	8.5	66.1	0.97	0.79	0.98	20.2
3	R2	All MCs	171	7.4	171	7.4	*0.616	44.9	LOS D	6.6	49.4	0.99	0.80	1.01	13.6
Appro	ach		669	10.1	669	10.1	0.623	40.2	LOS C	8.5	66.1	0.98	0.80	0.99	18.6
East:	King S	St (E)													
5	T1	All MCs	12	0.0	12	0.0	0.084	44.2	LOS D	0.8	2.1	0.99	0.66	0.99	4.4
6	R2	All MCs	6	0.0	6	0.0	0.084	50.4	LOS D	0.8	2.1	0.99	0.66	0.99	13.4
Appro	ach		18	0.0	18	0.0	0.084	46.4	LOS D	0.8	2.1	0.99	0.66	0.99	8.2
North	Kent	St (N)													
7	L2	All MCs	17	0.0	17	0.0	0.174	46.2	LOS D	3.1	8.4	0.98	0.71	0.98	10.7
8	T1	All MCs	58	0.0	58	0.0	0.174	42.8	LOS D	3.1	8.4	0.98	0.71	0.98	18.9
9	R2	All MCs	17	0.0	17	0.0	0.075	47.4	LOS D	0.7	1.9	0.97	0.66	0.97	11.9
Appro	ach		92	0.0	92	0.0	0.174	44.2	LOS D	3.1	8.4	0.98	0.70	0.98	16.2
West:	King	St (W)													
10	L2	All MCs	276	2.3	276	2.3	* 0.553	15.2	LOS B	5.8	37.8	0.42	0.49	0.42	26.4
11	T1	All MCs	1304	4.7	1304	4.7	*0.553	4.1	LOS A	5.8	37.8	0.20	0.19	0.20	28.7
Appro	ach		1580	4.3	1580	4.3	0.553	6.0	LOS A	5.8	37.8	0.24	0.25	0.24	27.9
All Ve	hicles		2359	5.7	2359	5.7	0.623	17.5	LOS B	8.5	66.1	0.48	0.42	0.49	21.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perform	nance							
Mo\ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Kent St (S)									
P1	Full	404	38.9	LOS D	1.0	1.0	0.94	0.94	55.6	20.0	0.36
Eas	t: King St (E)										

P2 Full	397	38.9	LOS D	1.0	1.0	0.94	0.94	55.6	20.0	0.36
North: Kent St (N)										
P3 Full	577	39.2	LOS D	1.4	1.4	0.94	0.94	55.9	20.0	0.36
West: King St (W)										
P4 Full	494	39.1	LOS D	1.2	1.2	0.94	0.94	55.7	20.0	0.36
All Pedestrians	1872	39.1	LOS D	1.4	1.4	0.94	0.94	55.7	20.0	0.36

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★ Site: BGU16 [BGU16 Pedestrian Mid-block Crossing at Hickson Rd (North of Metro) (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

NA

Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehic	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	and lows	Ar Fl	rival lows	Deg. Satn	Aver. Delav	Level of Service	95% Bac	k Of Queu	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	v/c	sec		[Veh. veh	Dist]		Rate	Cycles	km/h
South	: Hick	son Rd (S)	70	veni/m	70	10	000		Von					KIII/II
2	T1	All MCs	284	9.6	284	9.6	0.178	2.1	LOS A	0.8	5.7	0.01	0.36	0.01	37.6
Appro	ach		284	9.6	284	9.6	0.178	2.1	LOS A	0.8	5.7	0.01	0.36	0.01	37.6
North	Hick	son Rd (N	1)												
8	T1	All MCs	162	11.0	162	11.0	0.102	2.1	LOS A	0.4	3.0	0.01	0.36	0.01	36.1
Appro	ach		162	11.0	162	11.0	0.102	2.1	LOS A	0.4	3.0	0.01	0.36	0.01	36.1
All Ve	hicles		446	10.1	446	10.1	0.178	2.1	NA	0.8	5.7	0.01	0.36	0.01	37.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: BGU17 [BGU17 Pedestrian Mid-block Crossing at Hickson Rd (South of Metro) (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

New Site Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehicle Movement Performance

				ina															
Mov ID	Turn	Mov Class	Dem Fl	and ows	Ar Fl	rival ows	Deg Satr	. Av 1 De	ver. elay	Level of Service	959	% Bac	k O1	Queue	Prop. Que	l St	Eff. top	Aver No. o	. Aver. f Speed
			[Total ł veh/h	HV] %	[Total l veh/h	HV] %	v/c	; ;	sec			Veh. veh		Dist] m		R	ate	Cycles	s km/h
South	: Hick	son Rd (S	5)																
2	T1	All MCs	284	9.6	284	9.6	0.183	3	2.2	LOS A		0.8		5.9	0.12	0	.35	0.12	35.7
Appro	ach		284	9.6	284	9.6	0.183	3	2.2	LOS A		0.8		5.9	0.12	0.	.35	0.12	35.7
North	Hick	son Rd (N)																
8	T1	All MCs	162	11.0	162	11.0	0.105	5	2.2	LOS A		0.4		3.1	0.11	0	.35	0.11	37.8
Appro	ach		162	11.0	162	11.0	0.105	5	2.2	LOS A		0.4		3.1	0.11	0	.35	0.11	37.8
All Ve	hicles	;	446 ⁻	10.1	446	10.1	0.183	3	2.2	NA		0.8		5.9	0.11	0	.35	0.11	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: BGU18 [BGU18 Shelley St / Erskine St (Site Folder: Block 4 - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 305

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perform	ance									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	[Total HV	[Total HV	Sain	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: She	lley St (S))										
1	L2	All MCs	20 15.8	20 15.8	0.082	10.7	LOS A	1.5	11.2	0.38	0.37	0.38	20.3
2	T1	All MCs	75 7.0	75 7.0	0.082	5.6	LOS A	1.5	11.2	0.38	0.37	0.38	28.9
3	R2	All MCs	376 2.0	376 2.0	*0.554	13.6	LOS A	9.4	67.0	0.61	0.73	0.61	17.1
Appro	ach		471 3.4	471 3.4	0.554	12.2	LOS A	9.4	67.0	0.56	0.66	0.56	19.1
East:	Erskir	ne St (E)											
4	L2	All MCs	37 2.9	37 2.9	0.131	40.6	LOS C	1.4	10.2	0.90	0.71	0.90	10.5
5	T1	All MCs	120 8.8	120 8.8	*0.547	37.3	LOS C	6.3	47.3	0.96	0.78	0.96	8.3
6	R2	All MCs	32 6.7	32 6.7	0.547	48.1	LOS D	6.3	47.3	0.96	0.78	0.96	9.4
Appro	ach		188 7.3	188 7.3	0.547	39.8	LOS C	6.3	47.3	0.95	0.77	0.95	8.7
North	: Shel	ley St (N))										
7	L2	All MCs	54 5.9	54 5.9	0.091	10.4	LOS A	0.9	6.8	0.41	0.60	0.41	17.7
8	T1	All MCs	4 0.0	4 0.0	0.021	6.2	LOS A	0.3	2.2	0.39	0.49	0.39	26.4
9	R2	All MCs	13 25.0	13 25.0	0.021	10.1	LOS A	0.3	2.2	0.39	0.49	0.39	16.0
Appro	ach		71 9.0	71 9.0	0.091	10.1	LOS A	0.9	6.8	0.40	0.57	0.40	17.8
West:	Erski	ne St (W))										
10	L2	All MCs	17 12.5	17 12.5	0.457	43.8	LOS D	3.6	27.9	0.93	0.75	0.93	9.9
11	T1	All MCs	179 13.5	179 13.5	0.457	35.7	LOS C	4.7	37.1	0.93	0.75	0.93	5.6
12	R2	All MCs	6 50.0	6 50.0	0.457	44.0	LOS D	4.7	37.1	0.93	0.75	0.93	11.5
Appro	ach		202 14.6	202 14.6	0.457	36.7	LOS C	4.7	37.1	0.93	0.75	0.93	6.3
All Ve	hicles	i	932 7.0	932 7.0	0.554	22.9	LOS B	9.4	67.0	0.71	0.69	0.71	12.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Movement Performance												
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed		
	ned/h	500		[Ped	Dist]		Rate	202	m	mleec		
South: Shelley St (S)												
P1 Full	102	38.4	LOS D	0.2	0.2	0.93	0.93	205.1	200.0	0.98		

East: Erskine St (E)												
P2 Full	75	38.4	LOS D	0.2	0.2	0.93	0.93	205.1	200.0	0.98		
North: Shelley St (N)												
P3 Full	161	38.5	LOS D	0.4	0.4	0.93	0.93	205.2	200.0	0.97		
West: Erskine St (W)												
P4 Full	140	38.5	LOS D	0.3	0.3	0.93	0.93	205.2	200.0	0.97		
All Pedestrians	478	38.5	LOS D	0.4	0.4	0.93	0.93	205.1	200.0	0.97		

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V Site: BUG01 [BGU01 Hickson Rd / Towns PI (Site Folder: Block 4 - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Site Category: (None) Give-Way (Two-Way)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Dem Fi [Total	nand lows HV]	Ar Fl [Total] veb/b	rival ows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
East: Hickson Rd (E)								300		VCIT					K111/11
4a	L1	All MCs	182	4.6	182	4.6	0.289	4.4	LOS A	1.4	9.7	0.48	0.57	0.48	34.1
6a	R1	All MCs	95	0.0	95	0.0	0.289	8.4	LOS A	1.4	9.7	0.48	0.57	0.48	34.1
Appro	ach		277	3.0	277	3.0	0.289	5.8	NA	1.4	9.7	0.48	0.57	0.48	34.1
NorthWest: Towns PI (NW)															
27a	L1	All MCs	134	1.6	134	1.6	0.351	6.8	LOS A	1.5	10.9	0.67	0.90	0.86	32.5
29	R2	All MCs	73	7.2	73	7.2	0.351	13.3	LOS A	1.5	10.9	0.67	0.90	0.86	21.4
Appro	ach		206	3.6	206	3.6	0.351	9.1	LOS A	1.5	10.9	0.67	0.90	0.86	30.6
SouthWest: Hickson Rd (SW)															
30	L2	All MCs	145	5.1	145	5.1	0.319	5.7	LOS A	1.7	12.2	0.42	0.50	0.42	32.7
32a	R1	All MCs	254	2.9	254	2.9	0.319	3.2	LOS A	1.7	12.2	0.42	0.50	0.42	36.4
Appro	ach		399	3.7	399	3.7	0.319	4.1	NA	1.7	12.2	0.42	0.50	0.42	35.7
All Ve	hicles		882	3.5	882	3.5	0.351	5.8	NA	1.7	12.2	0.49	0.61	0.54	34.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: BGU02 [BGU02 Dalgety Rd / Towns PI (Site Folder: Block 4 - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

Site Category: (None) Roundabout

Vehic	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Dalg	ety Rd (S	S)												
30	L2	All MCs	1	0.0	1	0.0	0.130	6.1	LOS A	0.8	5.6	0.12	0.56	0.12	24.5
3b	R3	All MCs	181	4.1	181	4.1	0.130	6.3	LOS A	0.8	5.6	0.12	0.56	0.12	32.2
32u	U	All MCs	6	0.0	6	0.0	0.130	7.0	LOS A	0.8	5.6	0.12	0.56	0.12	34.7
Appro	ach		188	3.9	188	3.9	0.130	6.3	LOS A	0.8	5.6	0.12	0.56	0.12	32.3
South	East:	Towns P	I (SE)												
21b	L3	All MCs	220	3.3	220	3.3	0.165	2.7	LOS A	1.1	7.6	0.14	0.42	0.14	35.4
21a	L1	All MCs	5	0.0	5	0.0	0.165	8.3	LOS A	1.1	7.6	0.14	0.42	0.14	18.8
23u	U	All MCs	15	0.0	15	0.0	0.165	7.0	LOS A	1.1	7.6	0.14	0.42	0.14	30.1
Appro	ach		240	3.1	240	3.1	0.165	3.1	LOS A	1.1	7.6	0.14	0.42	0.14	35.0
West:	Parki	ng Acces	s (W)												
12a	R1	All MCs	11	0.0	11	0.0	0.025	1.2	LOS A	0.1	0.9	0.39	0.20	0.39	9.6
29	R2	All MCs	16	0.0	16	0.0	0.025	1.2	LOS A	0.1	0.9	0.39	0.20	0.39	21.6
29u	U	All MCs	1	0.0	1	0.0	0.025	1.2	LOS A	0.1	0.9	0.39	0.20	0.39	9.8
Appro	ach		27	0.0	27	0.0	0.025	1.2	LOS A	0.1	0.9	0.39	0.20	0.39	18.0
All Ve	hicles	i	456	3.2	456	3.2	0.165	4.3	LOS A	1.1	7.6	0.14	0.46	0.14	32.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: BGU03 [BGU03 Kent St / Argyle St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Site Category: (None) Give-Way (Two-Way)

Vehic	cle M	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	hand	Arriv	/al [Deg.	Aver.	Level of	95%	Back Of	Prop.	Eff.	Aver.	Aver.
שו		Class	FI [Total]	ows HV 1	FIO Total H	ws : V1	Sath	Delay	Service	[Veh	ueue Dist 1	Que	Stop Rate	NO. OT Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Kent	: St (S)													
1	L2	All MCs	72	7.4	72 7	7.4 0.	454	6.4	LOS A	2.4	17.5	0.66	0.93	0.96	33.9
2	T1	All MCs	29 ⁻	14.3	29 14	1.3 0.	454	8.8	LOS A	2.4	17.5	0.66	0.93	0.96	32.4
3	R2	All MCs	158	0.0	158 ().0 0.	454	12.6	LOS A	2.4	17.5	0.66	0.93	0.96	32.7
Appro	ach		259	3.7	259 3	3.7 0	454	10.4	LOS A	2.4	17.5	0.66	0.93	0.96	33.0
East:	Argyle	e St (E)													
4	L2	All MCs	185	1.1	185 1	l.1 0.	280	5.4	LOS A	1.4	9.8	0.44	0.47	0.44	36.5
5	T1	All MCs	91	0.0	91 ().0 0.	280	1.2	LOS A	1.4	9.8	0.44	0.47	0.44	36.0
6	R2	All MCs	63	33.3	6 33	3.3 0.	280	5.3	LOS A	1.4	9.8	0.44	0.47	0.44	30.0
Appro	ach		282	1.5	282 1	1.5 0.	280	4.0	NA	1.4	9.8	0.44	0.47	0.44	36.3
North:	Kent	St (N)													
7	L2	All MCs	8	12.5	8 12	2.5 0	039	8.2	LOS A	0.1	1.0	0.50	0.91	0.50	26.3
8	T1	All MCs	15	0.0	15 ().0 0.	039	11.6	LOS A	0.1	1.0	0.50	0.91	0.50	32.9
9	R2	All MCs	1	0.0	1 ().0 0.	039	8.8	LOS A	0.1	1.0	0.50	0.91	0.50	29.9
Appro	ach		24	4.3	24 4	1.3 0.	039	10.3	LOS A	0.1	1.0	0.50	0.91	0.50	31.4
West:	Argyl	e PI (W)													
10	L2	All MCs	8	0.0	8 (0.0 0.	186	5.1	LOS A	0.9	6.5	0.42	0.41	0.42	34.8
11	T1	All MCs	97	4.3	97 4	1.3 0.	186	1.2	LOS A	0.9	6.5	0.42	0.41	0.42	36.5
12	R2	All MCs	87	0.0	87 ().0 0.	186	5.9	LOS A	0.9	6.5	0.42	0.41	0.42	37.2
Appro	ach		193	2.2	193 2	2.2 0	186	3.5	NA	0.9	6.5	0.42	0.41	0.42	36.8
All Ve	hicles		758	2.5	758 2	2.5 0.	454	6.3	NA	2.4	17.5	0.51	0.62	0.61	35.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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CCG MOVEMENT SUMMARY

□ Common Control Group: CCG1 [TCS 4272] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 PM Peak)]

EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (CCG User-Given Phase Times)

Vehio	cle M	ovement	t Perfo	orma	nce <u>(</u> C	CG)									
Mov ID	Turn	Mov Class	Dem Fl [Total] veh/ <u>h</u>	nand lows HV] <u>%</u>	Ar Fl [Total] veh/ <u>h</u>	rival ows HV] %_	Deg. Satn v/ <u>c</u>	Aver. Delay se <u>c</u>	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/ <u>h</u>
Site: E	3GU0	4 [BGU04	Pedes	strian	Mid-bl	ock C	crossing at	t Kent St	near Gas	Ln]					
South	: Ken	t St													
2	T1	All MCs	353	3.0	353	3.0	0.275	1.6	LOS A	2.0	14.1	0.14	0.12	0.14	38.6
Appro	bach		353	3.0	353	3.0	0.275	1.6	LOS A	2.0	14.1	0.14	0.12	0.14	38.6
North	: Kent	St													
8	T1	All MCs	368	2.0	368	2.0	0.413	32.0	LOS C	7.0	49.7	0.90	0.74	0.90	22.3
Appro	bach		368	2.0	368	2.0	0.413	32.0	LOS C	7.0	49.7	0.90	0.74	0.90	22.3
All Ve	hicles	i	721	2.5	721	2.5	0.413	17.2	LOS B	7.0	49.7	0.52	0.43	0.52	28.5
Site: E	3GU0	5 [BGU05	Kent S	St / S	ydney l	Harbo	our Bridge	(SHB) C	n-ramp]						
South	: Ken	t St (S)													
2	T1	All MCs	560	4.1	560	4.1	0.441	4.9	LOS A	8.7	63.4	0.39	0.35	0.39	32.2
3a	R1	All MCs	285	9.2	285	9.2	*0.420	18.9	LOS B	6.9	52.1	0.65	0.63	0.65	25.7
Appro	bach		845	5.9	845	5.9	0.441	9.6	LOS A	8.7	63.4	0.48	0.44	0.48	28.9
East:	Clare	nce St (E))												
4	L2	All MCs	134	6.3	134	6.3	0.494	42.3	LOS C	5.6	41.2	0.96	0.79	0.96	11.1
6	R2	All MCs	180	8.2	180	8.2	*0.633	42.9	LOS D	7.7	57.8	0.98	0.83	1.01	11.0
Appro	bach		314	7.4	314	7.4	0.633	42.7	LOS D	7.7	57.8	0.97	0.81	0.99	11.0
North	East:	SHB On-r	amp (N	IE)											
24a	L1	All MCs	38	0.0	38	0.0	0.025	28.5	LOS B	1.2	3.3	0.80	0.60	0.80	20.5
Appro	bach		38	0.0	38	0.0	0.025	28.5	LOS B	1.2	3.3	0.80	0.60	0.80	20.5
North	: Kent	St (N)													
7b	L3	All MCs	139	5.3	139	5.3	0.540	49.3	LOS D	6.3	46.2	1.00	0.84	1.00	11.1
8	T1	All MCs	83	34.2	83	34.2	*0.303	13.8	LOS A	1.6	14.1	0.43	0.34	0.43	12.1
Appro	bach		222	16.1	222	16.1	0.540	36.0	LOS C	6.3	46.2	0.79	0.65	0.79	11.3
All Ve	hicles		1419	7.6	1419	7.6	0.633	21.6	LOS B	8.7	63.4	0.64	0.56	0.65	19.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance (C	CG)					
Mov	Dem.	Aver.	Level of	AVERAGE BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUEUE	Que	Stop	Time	Dist.	Speed

				[Ped	Dist]		Rate			
	ped/h	sec		ped	m			sec	m	m/sec
Site: BGU04 [BGU	J04 Pede	estrian N	/lid-block Cro	ossing at K	(ent St near	Gas Ln]				
South: Kent St										
P1 Full	212	38.6	LOS D	0.5	0.5	0.93	0.93	205.3	200.0	0.97
All Pedestrians	212	38.6	LOS D	0.5	0.5	0.93	0.93	205.3	200.0	0.97
Site: BGU05 [BGL	J05 Kent	St / Syd	dney Harbou	ır Bridge (S	SHB) On-ram	ıp]				
South: Kent St (S)										
P1 Full	18	34.7	LOS D	0.0	0.0	0.88	0.88	51.4	20.0	0.39
All Pedestrians	18	34.7	LOS D	0.0	0.0	0.88	0.88	51.4	20.0	0.39

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Site: BGU06 [BGU06 Hickson Rd / Napoleon St / Sussex St (Site Folder: Block 4 - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 4625

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 80 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Iotal veh/h	HV J %	[Iotal∶ veh/h	HV J %	v/c	sec		[Veh. veh	Dist J m		Rate	Cycles	km/h
South	: Sus	sex St (S))												
2	T1	All MCs	385	0.8	385	0.8	0.409	14.1	LOS A	9.6	67.7	0.68	0.59	0.68	27.7
3	R2	All MCs	131	1.6	131	1.6	*0.492	28.2	LOS B	4.2	29.8	0.93	0.78	0.93	18.2
Appro	ach		516	1.0	516	1.0	0.492	17.7	LOS B	9.6	67.7	0.75	0.64	0.75	25.0
East:	Napol	ean St (E)												
4	L2	All MCs	148	12.1	148	12.1	0.314	28.0	LOS B	4.6	35.7	0.83	0.75	0.83	13.1
6	R2	All MCs	155	4.1	155	4.1	*0.464	35.5	LOS C	5.6	40.3	0.94	0.78	0.94	19.3
Appro	ach		303	8.0	303	8.0	0.464	31.8	LOS C	5.6	40.3	0.88	0.77	0.88	17.1
North:	Hick	son Rd (N	1)												
7	L2	All MCs	113	0.0	113	0.0	0.180	29.3	LOS C	3.1	21.8	0.74	0.71	0.74	23.0
8	T1	All MCs	375	3.7	375	3.7	* 0.597	28.1	LOS B	12.0	86.8	0.87	0.75	0.87	19.5
Appro	ach		487	2.8	487	2.8	0.597	28.4	LOS B	12.0	86.8	0.84	0.74	0.84	18.4
West:	Car F	Park Acce	ss (W)												
10	L2	All MCs	2	0.0	2	0.0	0.122	40.7	LOS C	0.6	4.1	0.97	0.67	0.97	12.1
11	T1	All MCs	49	0.0	49	0.0	*0.583	42.7	LOS D	2.9	20.3	0.99	0.79	1.05	8.8
12	R2	All MCs	33	0.0	33	0.0	0.583	43.2	LOS D	2.9	20.3	1.00	0.83	1.08	2.3
Appro	ach		84	0.0	84	0.0	0.583	42.9	LOS D	2.9	20.3	1.00	0.80	1.06	6.7
All Ve	hicles		1391	3.1	1391	3.1	0.597	26.0	LOS B	12.0	86.8	0.82	0.71	0.83	19.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perform	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	EUE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Sussex St	(S)									
P1	Full	56	33.4	LOS D	0.1	0.1	0.91	0.91	50.1	20.0	0.40
Eas	t: Napolean S	st (E)									
P2	Full	102	33.4	LOS D	0.2	0.2	0.92	0.92	50.1	20.0	0.40

North: Hickson Rd	(N)												
P3 Full	57	33.4	LOS D	0.1	0.1	0.91	0.91	50.1	20.0	0.40			
West: Car Park Acc	Vest: Car Park Access (W)												
P4 Full	171	33.5	LOS D	0.4	0.4	0.92	0.92	50.2	20.0	0.40			
All Pedestrians	385	33.5	LOS D	0.4	0.4	0.92	0.92	50.1	20.0	0.40			

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Site: BGU07 [BGU07 Margaret St / Kent St / Napoleon St (Site Folder: Block 4 - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

TCS 308

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	FI [Total	lows HV 1	FI Total	ows HV/1	Satn	Delay	Service	[Veh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		i tato	0,0100	km/h
South	: Kent	t St (S)													
1a	L1	All MCs	59	17.9	59	17.9	*0.400	25.8	LOS B	9.7	72.1	0.68	0.61	0.68	20.5
2	T1	All MCs	469	4.3	469	4.3	0.400	19.5	LOS B	9.7	72.1	0.76	0.65	0.76	8.4
3	R2	All MCs	45	2.3	45	2.3	0.400	52.3	LOS D	5.5	40.2	0.86	0.73	0.86	6.9
Appro	ach		574	5.5	574	5.5	0.400	22.7	LOS B	9.7	72.1	0.76	0.65	0.76	9.7
East:	Marga	aret St (E)												
4	L2	All MCs	37	2.9	37	2.9	0.108	24.6	LOS B	0.9	6.6	0.58	0.62	0.58	11.2
6a	R1	All MCs	185	6.3	185	6.3	0.435	16.2	LOS B	5.0	37.0	0.57	0.57	0.57	20.9
6	R2	All MCs	26	0.0	26	0.0	*0.435	18.7	LOS B	5.0	37.0	0.57	0.57	0.57	10.8
Appro	ach		248	5.1	248	5.1	0.435	17.7	LOS B	5.0	37.0	0.57	0.57	0.57	18.7
North	Kent	St (N)													
7	L2	All MCs	46	4.5	46	4.5	0.313	38.4	LOS C	5.9	42.0	0.76	0.66	0.76	18.5
8	T1	All MCs	218	0.5	218	0.5	0.313	26.6	LOS B	5.9	42.0	0.84	0.68	0.84	17.3
9b	R3	All MCs	59	3.6	59	3.6	0.177	8.8	LOS A	0.4	3.1	0.17	0.56	0.17	30.6
Appro	ach		323	1.6	323	1.6	0.313	25.0	LOS B	5.9	42.0	0.71	0.66	0.71	19.4
North	West:	Napoleo	n St (N\	N)											
27b	L3	All MCs	201	1.0	201	1.0	0.455	9.0	LOS A	5.5	38.6	0.67	0.71	0.67	23.0
27a	L1	All MCs	92	0.0	92	0.0	0.455	21.8	LOS B	5.5	38.6	0.67	0.71	0.67	23.0
Appro	ach		293	0.7	293	0.7	0.455	13.0	LOS A	5.5	38.6	0.67	0.71	0.67	23.0
All Ve	hicles		1438	3.6	1438	3.6	0.455	20.4	LOS B	9.7	72.1	0.70	0.65	0.70	16.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUI [Ped	BACK OF EUE Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Kent St (S	5)									

P1 Full	635	33.0	LOS D	1.4	1.4	0.87	0.87	49.6	20.0	0.40
East: Margaret St	:(E)									
P2 Full	146	36.7	LOS D	0.3	0.3	0.91	0.91	53.3	20.0	0.37
North: Kent St (N))									
P3 Full	283	31.6	LOS D	0.6	0.6	0.84	0.84	48.3	20.0	0.41
NorthWest: Napol	leon St (N	WV)								
P7 Full	503	31.9	LOS D	1.1	1.1	0.85	0.85	198.6	200.0	1.01
All Pedestrians	1567	32.7	LOS D	1.4	1.4	0.86	0.86	97.6	77.8	0.80

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Site: BGU08 [BGU08 Margaret St / Clarence St (Site Folder: Block 4 - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 319

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	ance									
Mov ID	Turn	Mov Class	Demand Flows [Total HV] veh/h %	Arrival Flows [Total HV] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Clar	ence St (S)										
1	L2	All MCs	0.0	0 0.0	0.000	0.0	NA	0.0	0.0	0.00	0.00	0.00	0.0
2	T1	All MCs	863 18.5	863 18.5	*0.532	22.0	LOS B	13.5	95.0	0.79	0.68	0.79	19.0
3	R2	All MCs	60 1.8	60 1.8	*0.190	30.0	LOS C	3.4	33.3	0.76	0.69	0.76	14.5
Appro	ach		923 17.4	923 17.4	0.532	22.5	LOS B	13.5	95.0	0.79	0.68	0.79	18.7
East:	Marga	aret St (E))										
5	T1	All MCs	224 5.6	224 5.6	0.254	8.8	LOS A	3.8	27.9	0.41	0.35	0.41	15.4
6	R2	All MCs	100 63.2	100 63.2	0.278	21.2	LOS B	2.7	29.6	0.82	0.75	0.82	12.0
Appro	ach		324 23.4	324 23.4	0.278	12.6	LOS A	3.8	29.6	0.54	0.47	0.54	13.6
West:	Marg	aret St (V	V)										
10	L2	All MCs	115 0.0	115 0.0	*0.627	45.6	LOS D	7.8	55.3	0.98	0.82	1.00	8.0
11	T1	All MCs	68 4.6	68 4.6	0.627	33.1	LOS C	7.8	55.3	0.98	0.82	1.00	4.4
Appro	ach		183 1.7	183 1.7	0.627	40.9	LOS C	7.8	55.3	0.98	0.82	1.00	6.7
All Ve	hicles		1431 16.8	<mark>1455</mark> 16.5	0.627	22.3	LOS B	13.5	95.0	0.74	0.64	0.74	16.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	/ement	Perforr	nance								
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed	
		ned/h	200		[Ped Dist] ped m			Rate	202	m	m/sec	
ped/h sec ped m sec South: Clarence St (S) 22.2 2.7 2.7 0.00 0.00 55.8												
P1	Full	1108	39.2	LOS D	2.7	2.7	0.96	0.96	55.8	20.0	0.36	
Eas	t: Margaret St	(E)										
P2	Full	511	38.2	LOS D	1.2	1.2	0.93	0.93	54.8	20.0	0.36	
Nor	th: Clarence S	St (N)										
P3	Full	581	36.4	LOS D	1.4	1.4	0.91	0.91	53.1	20.0	0.38	
We	st: Margaret S	t (W)										
P4	Full	609	33.8	LOS D	1.4	1.4	0.88	0.88	50.5	20.0	0.40	

All Pedestrians	2809	37.3	LOS D	2.7	2.7	0.92	0.92	53.9	20.0	0.37
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Site: BGU09 [BGU09 Margaret St / York St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 3042

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total HV] veh/h %	[Total HV] veh/h %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
East: I	Marga	aret St (E)										
4	L2	All MCs	146 1.4	146 1.4	0.186	19.6	LOS B	4.0	28.3	0.64	0.69	0.64	19.8
5	T1	All MCs	106 56.4	106 56.4	0.186	16.4	LOS B	4.0	28.3	0.64	0.53	0.64	11.8
Appro	ach		253 24.6	253 24.6	0.186	18.3	LOS B	4.0	28.7	0.64	0.62	0.64	17.5
North:	York	St (N)											
8	T1	All MCs	735 22.8	735 22.8	* 0.375	20.0	LOS B	7.5	63.0	0.74	0.63	0.74	20.5
9	R2	All MCs	218 7.2	218 7.2	0.308	23.2	LOS B	6.5	48.0	0.72	0.76	0.72	9.5
Appro	ach		953 19.2	953 19.2	0.375	20.7	LOS B	7.5	63.0	0.73	0.66	0.73	18.5
West:	Marg	aret St (V	V)										
12	R2	All MCs	128 3.3	128 3.3	0.268	31.8	LOS C	5.1	37.0	0.95	0.70	0.95	14.1
Appro	ach		128 3.3	128 3.3	0.268	31.8	LOS C	5.1	37.0	0.95	0.70	0.95	14.1
All Vel	nicles		1334 18.7	1334 18.7	0.375	21.3	LOS B	7.5	63.0	0.74	0.66	0.74	17.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	ovement	Perfor	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUI	BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m		1 10.10	sec	m	m/sec
South: York St (S)									
P1 Full	1498	35.1	LOS D	3.5	3.5	0.91	0.91	51.8	20.0	0.39
East: Margaret	St (E)									
P2 Full	1457	35.1	LOS D	3.4	3.4	0.91	0.91	51.7	20.0	0.39
North: York St (V)									
P3 Full	851	32.4	LOS D	1.9	1.9	0.86	0.86	49.1	20.0	0.41
West: Margaret	St (W)									
P4 Full	809	36.8	LOS D	1.9	1.9	0.92	0.92	53.5	20.0	0.37
All Pedestrians	4615	34.9	LOS D	3.5	3.5	0.90	0.90	51.6	20.0	0.39

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Site: BGU10 [BGU10 Pedestrian Mid-block Crossing at Sussex St under Exchange PI (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 3939 (?)

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 65 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Sussex St (S)															
2	T1	All MCs	535	0.6	535	0.6	0.249	8.1	LOS A	4.4	30.8	0.55	0.47	0.55	25.0
Appro	ach		535	0.6	535	0.6	0.249	8.1	LOS A	4.4	30.8	0.55	0.47	0.55	25.0
North	Suss	sex St (N)													
8	T1	All MCs	518	6.1	518	6.1	*0.254	8.1	LOS A	4.3	31.4	0.55	0.47	0.55	23.2
Appro	ach		518	6.1	518	6.1	0.254	8.1	LOS A	4.3	31.4	0.55	0.47	0.55	23.2
All Ve	hicles	;	1053	3.3	1053	3.3	0.254	8.1	LOS A	4.4	31.4	0.55	0.47	0.55	24.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec					
South: Sussex St	(S)														
P1 Full	306	26.2	LOS C	0.5	0.5	0.90	0.90	42.9	20.0	0.47					
All Pedestrians	306	26.2	LOS C	0.5	0.5	0.90	0.90	42.9	20.0	0.47					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: BGU11 [BGU11 Pedestrian Mid-block Crossing at Kent St near Margaret St (Site Folder: Block 4 - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 4109

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South: Kent St (S)															
2	T1	All MCs	586	6.6	586	6.6	*0.352	10.2	LOS A	4.0	30.0	0.73	0.60	0.73	21.9
Appro	2 T1 All M Approach		586	6.6	586	6.6	0.352	10.2	LOS A	4.0	30.0	0.73	0.60	0.73	21.9
North:	Kent	St (N)													
8	T1	All MCs	283	0.7	283	0.7	0.229	9.4	LOS A	2.6	18.6	0.67	0.54	0.67	14.7
Appro	ach		283	0.7	283	0.7	0.229	9.4	LOS A	2.6	18.6	0.67	0.54	0.67	14.7
All Ve	hicles		869	4.7	869	4.7	0.352	9.9	LOS A	4.0	30.0	0.71	0.58	0.71	20.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ned/h	202		[Ped]	Dist]		Rate	202	m	m/sec
South: Kent St (S) j)	360		peu				360		m/sec
P1 Full	1607	15.4	LOS B	1.8	1.8	0.86	0.86	32.1	20.0	0.62
All Pedestrians	1607	15.4	LOS B	1.8	1.8	0.86	0.86	32.1	20.0	0.62

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: BGU12 [BGU12 Sussex St / Erskine St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 310

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ט ו		Class	٦ Total آ	IOWS HV 1	۲۱ Total آ	lows HV 1	Sath	Delay	Service	[Veh.	Dist 1	Que	Stop Rate	NO. OT Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- ,	km/h
South	Sus	sex St (S))												
1	L2	All MCs	49	4.3	49	4.3	0.424	37.6	LOS C	7.2	51.5	0.89	0.75	0.89	14.8
2	T1	All MCs	353	0.9	353	0.9	*0.424	30.5	LOS C	7.8	55.0	0.89	0.74	0.89	15.1
Appro	ach		402	1.3	402	1.3	0.424	31.4	LOS C	7.8	55.0	0.89	0.74	0.89	15.1
East: I	Erskir	ne St (E)													
4	L2	All MCs	446	1.4	446	1.4	0.457	11.5	LOS A	8.4	59.8	0.46	0.65	0.46	26.6
5	T1	All MCs	51	2.1	51	2.1	0.149	2.6	LOS A	0.6	4.5	0.15	0.30	0.15	24.9
6	R2	All MCs	49	0.0	49	0.0	0.149	6.9	LOS A	0.6	4.5	0.15	0.30	0.15	24.9
Approach			546	1.3	546	1.3	0.457	10.2	LOS A	8.4	59.8	0.41	0.59	0.41	26.4
North:	Suss	ex St (N)													
7	L2	All MCs	45	25.6	45	25.6	0.097	26.6	LOS B	1.4	11.9	0.73	0.68	0.73	13.8
8	T1	All MCs	496	3.8	496	3.8	0.383	24.1	LOS B	8.3	59.7	0.80	0.68	0.80	22.7
9	R2	All MCs	17	6.3	17	6.3	*0.072	32.0	LOS C	0.6	4.3	0.85	0.68	0.85	12.2
Appro	ach		558	5.7	558	5.7	0.383	24.5	LOS B	8.3	59.7	0.80	0.68	0.80	21.9
West:	Erski	ne St (W))												
10	L2	All MCs	116	1.8	116	1.8	0.338	12.9	LOS A	8.3	59.3	0.56	0.55	0.56	13.4
11	T1	All MCs	247	3.0	247	3.0	0.338	11.1	LOS A	8.3	59.3	0.56	0.55	0.56	13.4
12	R2	All MCs	223	4.7	223	4.7	*0.493	21.8	LOS B	7.0	50.7	0.74	0.76	0.74	19.9
Appro	Approach			3.4	586	3.4	0.493	15.5	LOS B	8.3	59.3	0.63	0.63	0.63	17.4
All Vel	nicles		2093	3.1	2093	3.1	0.493	19.6	LOS B	8.4	59.8	0.67	0.65	0.67	20.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ned/h	, 990		[Ped	Dist]		Rate	992	m	m/sec
South: Sussex S	t (S)	300	_	peu			_	300		11/300
P1 Full	194	38.6	LOS D	0.5	0.5	0.93	0.93	55.3	20.0	0.36

East: Erskine St (E	Ξ)									
P2 Full	137	38.5	LOS D	0.3	0.3	0.93	0.93	55.2	20.0	0.36
North: Sussex St (N)									
P3 Full	265	38.7	LOS D	0.6	0.6	0.93	0.93	55.4	20.0	0.36
West: Erskine St (W)									
P4 Full	144	38.5	LOS D	0.3	0.3	0.93	0.93	55.2	20.0	0.36
All Pedestrians	740	38.6	LOS D	0.6	0.6	0.93	0.93	55.3	20.0	0.36

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Site: BGU13 [BGU13 Kent St / Erskine St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 307

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	NIC	500		[Veh.	Dist]		Rate	Cycles	km/h
South	Ken	t St (S)	ven/m	70		70	v/C	360		VEIT	111	_	_		N111/11
1	L2	All MCs	121	1.7	121	1.7	0.230	29.8	LOS C	4.1	29.0	0.80	0.74	0.80	15.8
2	T1	All MCs	472	6.9	472	6.9	*0.362	24.0	LOS B	6.6	49.7	0.77	0.63	0.77	17.4
3	R2	All MCs	2	0.0	2	0.0	0.030	11.9	LOS A	1.9	5.2	0.49	0.38	0.49	22.5
Appro	ach		595	5.8	595	5.8	0.362	25.1	LOS B	6.6	49.7	0.77	0.65	0.77	17.1
East: I	Erskir	ne St (E)													
5	T1	All MCs	252	2.1	252	2.1	0.220	23.9	LOS B	4.3	31.0	0.77	0.63	0.77	7.3
6	R2	All MCs	9	0.0	9	0.0	0.220	31.0	LOS C	4.0	28.8	0.77	0.63	0.77	7.3
Appro	ach		261	2.0	261	2.0	0.220	24.1	LOS B	4.3	31.0	0.77	0.63	0.77	7.3
North:	Kent	St (N)													
7	L2	All MCs	1	0.0	1	0.0	0.034	11.7	LOS A	2.3	6.3	0.49	0.38	0.49	18.5
8	T1	All MCs	119	0.0	119	0.0	0.034	9.8	LOS A	2.3	6.3	0.49	0.38	0.49	23.5
9	R2	All MCs	176	1.2	176	1.2	*0.601	40.8	LOS C	7.4	52.0	0.97	0.81	0.97	7.2
Appro	ach		296	0.7	296	0.7	0.601	28.2	LOS B	7.4	52.0	0.77	0.64	0.77	13.4
West:	Erski	ne St (W))												
10	L2	All MCs	49	2.1	49	2.1	0.253	28.9	LOS C	5.0	36.7	0.81	0.69	0.81	8.8
11	T1	All MCs	245	8.2	245	8.2	*0.253	26.3	LOS B	5.2	38.8	0.83	0.69	0.83	11.8
Appro	Approach		295	7.1	295	7.1	0.253	26.7	LOS B	5.2	38.8	0.82	0.69	0.82	11.4
All Vel	nicles		1446	4.4	1446	4.4	0.601	25.9	LOS B	7.4	52.0	0.78	0.65	0.78	13.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	/ement	Perform	nance							
Mo∖ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	ith: Kent St (S)									
P1	Full	281	38.7	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
Eas	t: Erskine St (E)									

P2 Full	288	38.7	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
North: Kent St (N)	1									
P3 Full	399	38.9	LOS D	1.0	1.0	0.94	0.94	55.6	20.0	0.36
West: Erskine St ((W)									
P4 Full	289	38.7	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
All Pedestrians	1258	38.8	LOS D	1.0	1.0	0.93	0.93	55.5	20.0	0.36

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Site: BGU14 [BGU14 Sussex St / King St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 4 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 284

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows HV]	Ar Fl [Total veb/b	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
East:	King S	St (E)	VON/IT	/0	Veniin	70	V/0	000		Von					1X11/11
4a	L1	All MCs	182	0.0	182	0.0	0.429	46.5	LOS D	8.0	21.5	1.00	0.79	1.00	18.1
Appro	ach		182	0.0	182	0.0	0.429	46.5	LOS D	8.0	21.5	1.00	0.79	1.00	18.1
North:	Suss	ex St (N)													
7	L2	All MCs	117	12.6	117	12.6	*0.653	24.7	LOS B	19.4	140.7	0.81	0.74	0.81	20.0
8	T1	All MCs	1097	2.0	1097	2.0	0.653	17.9	LOS B	20.3	144.4	0.80	0.73	0.80	27.2
Appro	ach		1214	3.0	1214	3.0	0.653	18.6	LOS B	20.3	144.4	0.80	0.73	0.80	26.7
South	West:	King St ((SW)												
30a	L1	All MCs	359	0.9	359	0.9	0.437	14.4	LOS A	7.4	51.9	0.74	0.76	0.74	36.5
32a	R1	All MCs	912	1.8	912	1.8	*0.621	29.6	LOS C	15.5	110.0	0.88	0.81	0.88	27.2
32b	R3	All MCs	225	10.3	225	10.3	0.417	28.5	LOS B	7.5	57.1	0.79	0.80	0.79	30.4
Appro	ach		1496	2.9	1496	2.9	0.621	25.8	LOS B	15.5	110.0	0.84	0.80	0.84	29.9
All Ve	hicles		2892	2.8	2892	2.8	0.653	24.1	LOS B	20.3	144.4	0.83	0.77	0.83	27.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	vement	Perform	nance							
Mov ID	, Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUI	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		171			[Ped	Dist]		Rate			',
		ped/n	sec		ped	m			sec	m	m/sec
Sou	th: Sussex St	(S)									
P1	Full	222	40.5	LOS E	0.5	0.5	0.95	0.95	57.2	20.0	0.35
Eas	t: King St (E)										
P2	Full	759	39.5	LOS D	1.8	1.8	0.95	0.95	56.2	20.0	0.36
Nor	th: Sussex St	(N)									
P3	Full	222	38.6	LOS D	0.5	0.5	0.93	0.93	55.3	20.0	0.36
Sou	thWest: King	St (SW)									
P8	Full	424	39.0	LOS D	1.0	1.0	0.94	0.94	205.6	200.0	0.97

P8B Slip/ Bypass	360	40.8	LOS E	0.9	0.9	0.96	0.96	207.4	200.0	0.96
All Pedestrians	1987	39.6	LOS D	1.8	1.8	0.95	0.95	115.5	91.0	0.79

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Site: BGU15 [BGU15 Kent St / King St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 4 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 283

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	[Total	HV]	[Total	HV]	Salli	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Ken	t St (S)													
1	L2	All MCs	21	0.0	21	0.0	0.197	46.3	LOS D	3.5	9.5	0.98	0.72	0.98	13.0
2	T1	All MCs	499	6.3	499	6.3	0.442	29.6	LOS C	8.6	64.2	0.87	0.72	0.87	22.6
3	R2	All MCs	244	0.9	244	0.9	*0.442	37.0	LOS C	8.2	60.9	0.90	0.77	0.90	15.7
Appro	ach		764	4.4	764	4.4	0.442	32.4	LOS C	8.6	64.2	0.89	0.74	0.89	20.3
East:	King S	St (E)													
5	T1	All MCs	57	0.0	57	0.0	0.175	42.9	LOS D	2.6	6.9	0.98	0.70	0.98	4.6
6	R2	All MCs	4	0.0	4	0.0	0.175	49.0	LOS D	2.6	6.9	0.98	0.70	0.98	14.0
Appro	ach		61	0.0	61	0.0	0.175	43.3	LOS D	2.6	6.9	0.98	0.70	0.98	5.5
North	Kent	St (N)													
7	L2	All MCs	9	0.0	9	0.0	0.176	46.2	LOS D	3.1	8.5	0.98	0.71	0.98	10.7
8	T1	All MCs	66	0.0	66	0.0	0.176	42.8	LOS D	3.1	8.5	0.98	0.71	0.98	19.0
9	R2	All MCs	84	0.0	84	0.0	*0.369	48.8	LOS D	3.7	10.0	1.00	0.74	1.00	11.7
Appro	ach		160	0.0	160	0.0	0.369	46.1	LOS D	3.7	10.0	0.99	0.72	0.99	14.9
West:	King	St (W)													
10	L2	All MCs	179	1.2	179	1.2	*0.427	13.9	LOS A	4.6	32.8	0.39	0.49	0.39	27.2
11	T1	All MCs	848	3.5	848	3.5	*0.427	3.9	LOS A	4.6	32.8	0.23	0.22	0.23	29.1
Appro	ach		1027	3.1	1027	3.1	0.427	5.6	LOS A	4.6	32.8	0.26	0.27	0.26	28.5
All Ve	hicles		2013	3.2	2013	3.2	0.442	20.2	LOS B	8.6	64.2	0.58	0.50	0.58	21.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perform	nance							
Mo\ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Kent St (S)									
P1	Full	480	39.1	LOS D	1.2	1.2	0.94	0.94	55.7	20.0	0.36
Eas	t: King St (E)										

P2 Full	360	38.9	LOS D	0.9	0.9	0.94	0.94	55.5	20.0	0.36
North: Kent St (N)										
P3 Full	686	39.4	LOS D	1.7	1.7	0.95	0.95	56.1	20.0	0.36
West: King St (W)										
P4 Full	504	39.1	LOS D	1.2	1.2	0.94	0.94	55.8	20.0	0.36
All Pedestrians	2031	39.1	LOS D	1.7	1.7	0.94	0.94	55.8	20.0	0.36

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★ Site: BGU16 [BGU16 Pedestrian Mid-block Crossing at Hickson Rd (North of Metro) (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

NA

Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Hick	son Rd (S)												
2	T1	All MCs	397	2.9	397	2.9	0.241	2.1	LOS A	1.1	7.9	0.01	0.36	0.01	37.6
Appro	ach		397	2.9	397	2.9	0.241	2.1	LOS A	1.1	7.9	0.01	0.36	0.01	37.6
North:	Hicks	son Rd (N	1)												
8	T1	All MCs	265	4.8	265	4.8	0.162	2.1	LOS A	0.7	4.9	0.01	0.36	0.01	36.1
Appro	ach		265	4.8	265	4.8	0.162	2.1	LOS A	0.7	4.9	0.01	0.36	0.01	36.1
All Ve	hicles		662	3.7	662	3.7	0.241	2.1	NA	1.1	7.9	0.01	0.36	0.01	37.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: BGU17 [BGU17 Pedestrian Mid-block Crossing at Hickson Rd (South of Metro) (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

New Site Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehicle Movement Performance

Mov ID	Turn	Mov Class	Den F	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] [%	[Total ∣ veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Hick	son Rd (S	5)	,0	VOII/II	70	110	000		Von					NIII/11
2	T1	All MCs	397	2.9	397	2.9	0.249	2.2	LOS A	1.1	8.2	0.14	0.36	0.14	35.6
Appro	ach		397	2.9	397	2.9	0.249	2.2	LOS A	1.1	8.2	0.14	0.36	0.14	35.6
North	Hicks	son Rd (N)												
8	T1	All MCs	265	4.8	265	4.8	0.168	2.2	LOS A	0.7	5.1	0.13	0.36	0.13	37.7
Appro	ach		265	4.8	265	4.8	0.168	2.2	LOS A	0.7	5.1	0.13	0.36	0.13	37.7
All Ve	hicles		662	3.7	662	3.7	0.249	2.2	NA	1.1	8.2	0.14	0.36	0.14	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: BGU18 [BGU18 Shelley St / Erskine St (Site Folder: Block 4 - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 305

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	ince										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	F [Total	HV]	Total	ows HV]	Sain	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Shel	ley St (S))												
1	L2	All MCs	13	0.0	13	0.0	0.053	10.9	LOS A	1.0	7.2	0.39	0.36	0.39	20.3
2	T1	All MCs	53	0.0	53	0.0	0.053	5.9	LOS A	1.0	7.2	0.39	0.36	0.39	28.6
3	R2	All MCs	236	0.4	236	0.4	*0.382	14.1	LOS A	5.5	39.0	0.57	0.70	0.57	16.8
Appro	ach		301	0.3	301	0.3	0.382	12.6	LOS A	5.5	39.0	0.53	0.62	0.53	19.0
East:	Erskir	ne St (E)													
4	L2	All MCs	17	6.3	17	6.3	0.067	40.0	LOS C	0.7	4.8	0.89	0.68	0.89	10.4
5	T1	All MCs	77	4.1	77	4.1	*0.316	33.7	LOS C	3.9	28.0	0.90	0.73	0.90	8.8
6	R2	All MCs	23	0.0	23	0.0	0.316	42.0	LOS C	3.9	28.0	0.90	0.73	0.90	10.0
Appro	ach		117	3.6	117	3.6	0.316	36.3	LOS C	3.9	28.0	0.90	0.72	0.90	9.3
North	Shel	ley St (N)	1												
7	L2	All MCs	183	0.6	183	0.6	0.231	11.5	LOS A	3.5	24.9	0.47	0.64	0.47	16.6
8	T1	All MCs	6	0.0	6	0.0	0.010	6.2	LOS A	0.2	1.2	0.38	0.39	0.38	27.6
9	R2	All MCs	4	0.0	4	0.0	0.010	10.0	LOS A	0.2	1.2	0.38	0.39	0.38	16.7
Appro	ach		194	0.5	194	0.5	0.231	11.3	LOS A	3.5	24.9	0.46	0.63	0.46	17.1
West:	Erski	ne St (W))												
10	L2	All MCs	6	0.0	6	0.0	0.316	42.4	LOS C	3.1	23.6	0.89	0.71	0.89	10.7
11	T1	All MCs	167	10.7	167	10.7	0.316	33.2	LOS C	3.7	28.3	0.89	0.71	0.89	6.0
12	R2	All MCs	3	33.3	3	33.3	0.316	40.5	LOS C	3.7	28.3	0.89	0.71	0.89	12.1
Appro	ach		177	10.7	177	10.7	0.316	33.7	LOS C	3.7	28.3	0.89	0.71	0.89	6.4
All Ve	hicles		788	3.2	788	3.2	0.382	20.5	LOS B	5.5	39.0	0.65	0.66	0.65	12.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ned/h	500		[Ped	Dist]		Rate	202	m	mleec
South: Shelley S	t (S)	360	_	peu		_	_	360		m/sec
P1 Full	334	38.8	LOS D	0.8	0.8	0.94	0.94	205.5	200.0	0.97

East: Erskine St (E	E)									
P2 Full	57	38.4	LOS D	0.1	0.1	0.92	0.92	205.0	200.0	0.98
North: Shelley St (N)									
P3 Full	339	38.8	LOS D	0.8	0.8	0.94	0.94	205.5	200.0	0.97
West: Erskine St (W)									
P4 Full	137	38.5	LOS D	0.3	0.3	0.93	0.93	205.2	200.0	0.97
All Pedestrians	866	38.7	LOS D	0.8	0.8	0.93	0.93	205.4	200.0	0.97

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V Site: BUG01 [BGU01 Hickson Rd / Towns PI (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Derr Fl [Total veh/h	nand lows HV] %	Ar Fl [Total] veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
East:	Hicks	on Rd (E))												
4a	L1	All MCs	213	3.0	213	3.0	0.305	4.1	LOS A	1.5	10.8	0.41	0.52	0.41	34.4
6a	R1	All MCs	102	2.1	102	2.1	0.305	8.2	LOS A	1.5	10.8	0.41	0.52	0.41	34.4
Appro	ach		315	2.7	315	2.7	0.305	5.4	NA	1.5	10.8	0.41	0.52	0.41	34.4
North	Nest:	Towns Pl	(NW)												
27a	L1	All MCs	168	1.3	168	1.3	0.413	7.3	LOS A	1.9	13.8	0.69	0.94	0.96	32.2
29	R2	All MCs	78	4.1	78	4.1	0.413	14.2	LOS A	1.9	13.8	0.69	0.94	0.96	20.9
Appro	ach		246	2.1	246	2.1	0.413	9.5	LOS A	1.9	13.8	0.69	0.94	0.96	30.5
South	West:	Hickson	Rd (SV	V)											
30	L2	All MCs	81	3.9	81	3.9	0.351	4.7	LOS A	2.1	14.7	0.33	0.43	0.33	33.4
32a	R1	All MCs	446	1.7	446	1.7	0.351	3.0	LOS A	2.1	14.7	0.33	0.43	0.33	36.8
Appro	ach		527	2.0	527	2.0	0.351	3.2	NA	2.1	14.7	0.33	0.43	0.33	36.5
All Ve	hicles		1088	2.2	1088	2.2	0.413	5.3	NA	2.1	14.7	0.43	0.57	0.49	34.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: BGU02 [BGU02 Dalgety Rd / Towns PI (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

Site Category: (None) Roundabout

Vehic	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Dalg	ety Rd (S	3)												
30	L2	All MCs	3	0.0	3	0.0	0.156	6.0	LOS A	0.9	6.6	0.07	0.58	0.07	24.6
3b	R3	All MCs	243	2.2	243	2.2	0.156	6.2	LOS A	0.9	6.6	0.07	0.58	0.07	32.4
32u	U	All MCs	1	0.0	1	0.0	0.156	6.9	LOS A	0.9	6.6	0.07	0.58	0.07	34.8
Appro	ach		247	2.1	247	2.1	0.156	6.2	LOS A	0.9	6.6	0.07	0.58	0.07	32.3
South	East:	Towns Pl	(SE)												
21b	L3	All MCs	177	3.0	177	3.0	0.119	2.6	LOS A	0.8	5.4	0.07	0.42	0.07	35.8
21a	L1	All MCs	6	0.0	6	0.0	0.119	8.2	LOS A	0.8	5.4	0.07	0.42	0.07	18.9
23u	U	All MCs	1	0.0	1	0.0	0.119	6.9	LOS A	0.8	5.4	0.07	0.42	0.07	30.9
Appro	ach		184	2.9	184	2.9	0.119	2.8	LOS A	0.8	5.4	0.07	0.42	0.07	35.3
West:	Parki	ng Acces	s (W)												
12a	R1	All MCs	3	0.0	3	0.0	0.010	1.4	LOS A	0.1	0.4	0.42	0.21	0.42	9.6
29	R2	All MCs	6	0.0	6	0.0	0.010	1.4	LOS A	0.1	0.4	0.42	0.21	0.42	21.4
29u	U	All MCs	1	0.0	1	0.0	0.010	1.4	LOS A	0.1	0.4	0.42	0.21	0.42	9.7
Appro	ach		11	0.0	11	0.0	0.010	1.4	LOS A	0.1	0.4	0.42	0.21	0.42	17.9
All Ve	hicles	i	442	2.4	442	2.4	0.156	4.7	LOS A	0.9	6.6	0.08	0.50	0.08	33.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: BGU03 [BGU03 Kent St / Argyle St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	and	Ar	rival	Deg.	Aver.	Level of	95%	Back Of	Prop.	Eff.	Aver.	Aver.
שר		Class	FI [TotaL	ows HV L	FI [TotaL	ows HV L	Sath	Delay	Service	ر Veh	ueue Dist1	Que	Rate_	Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		, late	C y cloc	km/h
South	: Kent	t St (S)													
1	L2	All MCs	109	1.9	109	1.9	0.586	8.8	LOS A	3.5	24.9	0.76	1.12	1.31	32.6
2	T1	All MCs	27	11.5	27	11.5	0.586	11.8	LOS A	3.5	24.9	0.76	1.12	1.31	30.8
3	R2	All MCs	163	0.0	163	0.0	0.586	16.7	LOS B	3.5	24.9	0.76	1.12	1.31	31.3
Appro	ach		300	1.8	300	1.8	0.586	13.4	LOS A	3.5	24.9	0.76	1.12	1.31	31.8
East:	Argyle	e St (E)													
4	L2	All MCs	160	0.0	160	0.0	0.304	6.3	LOS A	1.5	10.9	0.48	0.47	0.48	36.3
5	T1	All MCs	114	0.9	114	0.9	0.304	1.3	LOS A	1.5	10.9	0.48	0.47	0.48	35.8
6	R2	All MCs	94	44.4	94	44.4	0.304	5.4	LOS A	1.5	10.9	0.48	0.47	0.48	30.8
Appro	ach		283	1.9	283	1.9	0.304	4.3	NA	1.5	10.9	0.48	0.47	0.48	36.1
North:	Kent	St (N)													
7	L2	All MCs	7	0.0	7	0.0	0.060	7.6	LOS A	0.2	1.4	0.56	0.94	0.56	25.4
8	T1	All MCs	22	0.0	22	0.0	0.060	13.0	LOS A	0.2	1.4	0.56	0.94	0.56	32.3
9	R2	All MCs	2	0.0	2	0.0	0.060	9.0	LOS A	0.2	1.4	0.56	0.94	0.56	29.1
Appro	ach		32	0.0	32	0.0	0.060	11.5	LOS A	0.2	1.4	0.56	0.94	0.56	31.1
West:	Argyl	e PI (W)													
10	L2	All MCs	9	0.0	9	0.0	0.153	5.5	LOS A	0.7	5.2	0.44	0.42	0.44	34.4
11	T1	All MCs	75	4.2	75	4.2	0.153	1.2	LOS A	0.7	5.2	0.44	0.42	0.44	36.1
12	R2	All MCs	63	0.0	63	0.0	0.153	6.8	LOS A	0.7	5.2	0.44	0.42	0.44	36.9
Appro	ach		147	2.1	147	2.1	0.153	3.9	NA	0.7	5.2	0.44	0.42	0.44	36.5
All Ve	hicles		762	1.8	762	1.8	0.586	8.1	NA	3.5	24.9	0.58	0.74	0.80	33.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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CCG MOVEMENT SUMMARY

□ Common Control Group: CCG1 [TCS 4272] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 70 seconds (CCG User-Given Phase Times)

Vehio	cle M	ovemen	t Perfo	orma	nce (C	CCG)									
Mov ID	Turn	Mov Class	Dem F [Total	nand Iows HV 1	Ar Fl [Total	rival lows HV 1	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
Site: E	3GU0	4 [BGU04	1 Pedes	strian	Mid-bl	ock C	crossing at	Kent St	near Gas	Ln]					
South	: Ken	t St													
2	T1	All MCs	361	2.9	361	2.9	0.342	4.9	LOS A	4.2	30.5	0.37	0.32	0.37	36.1
Appro	ach		361	2.9	361	2.9	0.342	4.9	LOS A	4.2	30.5	0.37	0.32	0.37	36.1
North	: Kent	St													
8	T1	All MCs	307	4.8	307	4.8	0.482	29.5	LOS C	5.0	36.1	0.95	0.76	0.95	23.2
Appro	ach		307	4.8	307	4.8	0.482	29.5	LOS C	5.0	36.1	0.95	0.76	0.95	23.2
All Ve	hicles	;	668	3.8	668	3.8	0.482	16.2	LOS B	5.0	36.1	0.64	0.52	0.64	29.0
Site: E	3GU0	5 [BGU05	5 Kent S	St / S	ydney	Harbo	our Bridge	(SHB) C	n-ramp]						
South	: Ken	t St (S)													
2	T1	All MCs	285	2.2	285	2.2	0.268	9.4	LOS A	5.4	38.7	0.59	0.51	0.59	27.4
3a	R1	All MCs	207	1.5	207	1.5	*0.376	26.7	LOS B	6.1	43.4	0.92	0.78	0.92	22.4
Appro	ach		493	1.9	493	1.9	0.376	16.7	LOS B	6.1	43.4	0.73	0.62	0.73	24.5
East:	Clare	nce St (E)												
4	L2	All MCs	33	0.0	33	0.0	0.072	25.7	LOS B	0.9	6.1	0.80	0.68	0.80	15.5
6	R2	All MCs	141	3.0	141	3.0	*0.323	27.5	LOS B	4.1	29.2	0.86	0.76	0.86	14.8
Appro	ach		174	2.4	174	2.4	0.323	27.2	LOS B	4.1	29.2	0.85	0.74	0.85	14.9
North	East:	SHB On-	ramp (N	IE)											
24a	L1	All MCs	4	0.0	4	0.0	0.004	25.9	LOS B	0.1	0.3	0.85	0.55	0.85	21.1
Appro	ach		4	0.0	4	0.0	0.004	25.9	LOS B	0.1	0.3	0.85	0.55	0.85	21.1
North	: Kent	St (N)													
7b	L3	All MCs	108	7.8	108	7.8	0.296	35.1	LOS C	3.8	28.2	1.00	0.82	1.00	14.2
8	T1	All MCs	142	4.4	142	4.4	*0.666	26.9	LOS B	4.6	33.1	0.91	0.74	0.94	7.3
Appro	ach		251	5.9	251	5.9	0.666	30.5	LOS C	4.6	33.1	0.95	0.77	0.97	11.0
All Ve	hicles	;	921	3.1	921	3.1	0.666	22.5	LOS B	6.1	43.4	0.81	0.69	0.82	19.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mov	vement	Perforr	nance (C	CG)					
Mov	Dem.	Aver.	Level of	AVERAGE BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUEUE	Que	Stop	Time	Dist.	Speed

				[Ped	Dist]		Rate			
	ped/h	sec		ped	m			sec	m	m/sec
Site: BGU04 [BGU	04 Pede	estrian N	/lid-block C	rossing at K	ent St near	Gas Ln]				
South: Kent St										
P1 Full	95	28.5	LOS C	0.2	0.2	0.90	0.90	195.1	200.0	1.02
All Pedestrians	95	28.5	LOS C	0.2	0.2	0.90	0.90	195.1	200.0	1.02
Site: BGU05 [BGU	05 Kent	St / Sy	dney Harbo	our Bridge (S	HB) On-rar	mp]				
South: Kent St (S)										
P1 Full	1	24.9	LOS C	0.0	0.0	0.84	0.84	41.5	20.0	0.48
All Pedestrians	1	24.9	LOS C	0.0	0.0	0.84	0.84	41.5	20.0	0.48

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Site: BGU06 [BGU06 Hickson Rd / Napoleon St / Sussex St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 4625

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 70 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Sus	sex St (S)													
2 3	T1 R2	All MCs All MCs	337 74	0.6	337 74	0.6	0.320	9.5	LOS A LOS B	6.4 1.6	44.7	0.59	0.51	0.59	30.8 22.6
Appro	acn		411	0.8	411	0.8	0.320	11.0	LUSA	0.4	44.7	0.62	0.54	0.62	29.3
East:	Napol	lean St (E)												
4	L2	All MCs	59	16.1	59	16.1	0.118	22.2	LOS B	1.5	11.6	0.74	0.69	0.74	15.2
6	R2	All MCs	218	2.9	218	2.9	*0.471	27.6	LOS B	6.5	46.4	0.89	0.78	0.89	21.8
Appro	ach		277	5.7	277	5.7	0.471	26.4	LOS B	6.5	46.4	0.86	0.76	0.86	20.9
North:	Hick	son Rd (N	1)												
7	L2	All MCs	127	0.0	127	0.0	0.171	18.1	LOS B	2.8	19.7	0.68	0.69	0.68	25.4
8	T1	All MCs	328	4.2	328	4.2	*0.423	15.9	LOS B	8.0	58.3	0.76	0.65	0.76	23.1
Appro	ach		456	3.0	456	3.0	0.423	16.5	LOS B	8.0	58.3	0.73	0.66	0.73	23.9
West:	Car F	Park Acce	ss (W)												
10	L2	All MCs	1	0.0	1	0.0	0.041	43.0	LOS D	0.0	0.3	1.00	0.57	1.00	11.8
11	T1	All MCs	1	0.0	1	0.0	*0.195	44.6	LOS D	0.2	1.5	1.00	0.63	1.00	8.4
12	R2	All MCs	4	0.0	4	0.0	0.195	44.6	LOS D	0.2	1.5	1.00	0.63	1.00	2.2
Appro	ach		6	0.0	6	0.0	0.195	44.3	LOS D	0.2	1.5	1.00	0.62	1.00	5.4
All Ve	hicles		1149	2.8	1149	2.8	0.471	17.1	LOS B	8.0	58.3	0.73	0.64	0.73	24.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Ped	estrian Mov	vement	Perforr	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	EUE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sout	h: Sussex St	(S)									
P1	Full	57	28.4	LOS C	0.1	0.1	0.90	0.90	45.1	20.0	0.44
East	: Napolean S	st (E)									
P2	Full	38	28.4	LOS C	0.1	0.1	0.90	0.90	45.1	20.0	0.44

North: Hickson Rd	(N)									
P3 Full	23	28.4	LOS C	0.0	0.0	0.90	0.90	45.0	20.0	0.44
West: Car Park Acc	cess (W))								
P4 Full	75	28.4	LOS C	0.1	0.1	0.90	0.90	45.1	20.0	0.44
All Pedestrians	193	28.4	LOS C	0.1	0.1	0.90	0.90	45.1	20.0	0.44

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Site: BGU07 [BGU07 Margaret St / Kent St / Napoleon St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 308

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 70 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	hand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
UI		Class	⊢ Total]	IOWS HV 1	FI [Total	lows HV 1	Sath	Delay	Service	[Veh	Dist 1	Que	Stop Rate	NO. OF Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		1 10.110	0,000	km/h
South	: Ken	t St (S)													
1a	L1	All MCs	37	8.6	37	8.6	0.329	17.3	LOS B	6.2	44.5	0.65	0.58	0.65	23.0
2	T1	All MCs	360	2.6	360	2.6	0.329	14.6	LOS B	6.2	44.5	0.72	0.61	0.72	10.7
3	R2	All MCs	20	0.0	20	0.0	0.329	54.6	LOS D	3.3	23.7	0.90	0.72	0.90	7.4
Appro	ach		417	3.0	417	3.0	0.329	16.7	LOS B	6.2	44.5	0.72	0.61	0.72	12.0
East:	Marga	aret St (E)												
4	L2	All MCs	19	0.0	19	0.0	0.051	18.0	LOS B	0.3	2.3	0.51	0.58	0.51	13.7
6a	R1	All MCs	200	5.3	200	5.3	0.416	17.5	LOS B	5.3	38.4	0.72	0.66	0.72	20.2
6	R2	All MCs	23	0.0	23	0.0	0.416	20.2	LOS B	5.3	38.4	0.72	0.66	0.72	10.2
Appro	ach		242	4.3	242	4.3	0.416	17.8	LOS B	5.3	38.4	0.70	0.65	0.70	19.1
North	Kent	St (N)													
7	L2	All MCs	42	2.5	42	2.5	*0.347	40.7	LOS C	4.9	34.9	0.86	0.72	0.86	18.3
8	T1	All MCs	160	1.3	160	1.3	*0.347	20.7	LOS B	4.9	34.9	0.88	0.71	0.88	19.4
9b	R3	All MCs	45	4.7	45	4.7	0.196	12.7	LOS A	0.5	3.6	0.32	0.59	0.32	28.1
Appro	ach		247	2.1	247	2.1	0.347	22.7	LOS B	4.9	34.9	0.77	0.69	0.77	20.7
North	West:	Napoleo	n St (N	W)											
27b	L3	All MCs	97	0.0	97	0.0	0.415	10.0	LOS A	4.6	32.7	0.77	0.74	0.77	20.8
27a	L1	All MCs	133	1.6	133	1.6	*0.415	17.8	LOS B	4.6	32.7	0.77	0.74	0.77	20.8
Appro	ach		229	0.9	229	0.9	0.415	14.5	LOS A	4.6	32.7	0.77	0.74	0.77	20.8
All Ve	hicles		1136	2.7	1136	2.7	0.416	17.8	LOS B	6.2	44.5	0.74	0.66	0.74	18.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUI [Ped	BACK OF EUE Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Kent St (S	5)									

P1 Full	325	22.7	LOS C	0.5	0.5	0.81	0.81	39.4	20.0	0.51		
East: Margaret St (E)												
P2 Full	44	26.6	LOS C	0.1	0.1	0.87	0.87	43.3	20.0	0.46		
North: Kent St (N)												
P3 Full	84	21.7	LOS C	0.1	0.1	0.79	0.79	38.3	20.0	0.52		
NorthWest: Napoleon St (NW)												
P7 Full	122	21.7	LOS C	0.2	0.2	0.79	0.79	188.4	200.0	1.06		
All Pedestrians	576	22.6	LOS C	0.5	0.5	0.81	0.81	71.1	58.2	0.82		

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Site: BGU08 [BGU08 Margaret St / Clarence St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 319

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 70 seconds (Network Site User-Given Phase Times)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar F [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South: Clarence St (S)															
1	L2	All MCs	33	0.0	33	0.0	0.226	22.2	LOS B	3.8	26.6	0.72	0.62	0.72	18.5
2	T1	All MCs	460	9.6	460	9.6	*0.226	15.6	LOS B	3.9	27.5	0.71	0.60	0.71	22.0
3	R2	All MCs	41	0.0	41	0.0	0.226	22.1	LOS B	3.7	26.2	0.72	0.63	0.72	18.3
Appro	ach		534	8.3	534	8.3	0.226	16.5	LOS B	3.9	27.5	0.72	0.60	0.72	21.6
East: Margaret St (E)															
5	T1	All MCs	209	5.0	209	5.0	0.191	8.2	LOS A	2.1	15.0	0.46	0.40	0.46	15.4
6	R2	All MCs	46	18.2	46	18.2	*0.191	15.4	LOS B	1.8	13.6	0.63	0.59	0.63	16.8
Appro	ach		256	7.4	256	7.4	0.191	9.5	LOS A	2.1	15.0	0.49	0.43	0.49	15.8
West: Margaret St (W)															
10	L2	All MCs	97	1.1	97	1.1	*0.572	34.1	LOS C	6.2	44.0	0.94	0.78	0.94	10.2
11	T1	All MCs	98	2.2	98	2.2	0.572	25.0	LOS B	6.2	44.0	0.94	0.78	0.94	5.8
Appro	ach		195	1.6	195	1.6	0.572	29.5	LOS C	6.2	44.0	0.94	0.78	0.94	8.2
All Ve	hicles		984	6.7	984	6.7	0.572	17.3	LOS B	6.2	44.0	0.70	0.59	0.70	17.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Movement Performance											
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE BACK OF		Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		nod/h	600		[Ped	Dist J		Rate	500	m	m/soc
South: Clarence St (S)											111/300
P1	Full	268	27.8	LOS C	0.5	0.5	0.90	0.90	44.4	20.0	0.45
East: Margaret St (E)											
P2	Full	136	27.6	LOS C	0.2	0.2	0.89	0.89	44.3	20.0	0.45
North: Clarence St (N)											
P3	Full	332	26.1	LOS C	0.6	0.6	0.87	0.87	42.7	20.0	0.47
West: Margaret St (W)											
P4	Full	104	23.3	LOS C	0.2	0.2	0.82	0.82	40.0	20.0	0.50
All Pedestrians	840	26.5	LOS C	0.6	0.6	0.87	0.87	43.2	20.0	0.46	
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Site: BGU09 [BGU09 Margaret St / York St (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

■ Network: BGU-N2 [BGU Network 2 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 3042

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 70 seconds (Network User-Given Cycle Time)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	hand lows HV]	Ar Fl [Total]	rival ows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
East:	Marga	aret St (E)	/0	ven/m	/0	V/C	360	_	ven		_	_	_	K111/11
4	L2	All MCs	60	1.8	60	1.8	0.092	19.6	LOS B	1.4	9.7	0.70	0.67	0.70	19.8
5	T1	All MCs	52	16.3	52	16.3	0.086	16.2	LOS B	1.2	9.4	0.69	0.53	0.69	12.0
Appro	ach		112	8.5	112	8.5	0.092	18.0	LOS B	1.4	9.7	0.69	0.61	0.69	17.3
North:	York	St (N)													
8	T1	All MCs	733	6.9	733	6.9	*0.293	12.9	LOS A	5.3	38.9	0.67	0.56	0.67	24.8
9	R2	All MCs	204	5.2	204	5.2	0.254	16.5	LOS B	4.3	31.5	0.65	0.73	0.65	12.1
Appro	ach		937	6.5	937	6.5	0.293	13.7	LOS A	5.3	38.9	0.66	0.60	0.66	22.7
West:	Marg	aret St (V	∨)												
12	R2	All MCs	139	1.5	139	1.5	*0.289	29.4	LOS C	4.6	32.7	1.00	0.73	1.00	14.8
Appro	ach		139	1.5	139	1.5	0.289	29.4	LOS C	4.6	32.7	1.00	0.73	1.00	14.8
All Ve	nicles		1187	6.1	1187	6.1	0.293	15.9	LOS B	5.3	38.9	0.70	0.61	0.70	20.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m Dist j		Rale	sec	m	m/sec
South: York St (S))									
P1 Full	363	23.6	LOS C	0.6	0.6	0.83	0.83	40.2	20.0	0.50
East: Margaret St	(E)									
P2 Full	462	23.7	LOS C	0.8	0.8	0.83	0.83	40.3	20.0	0.50
North: York St (N)										
P3 Full	238	21.8	LOS C	0.4	0.4	0.79	0.79	38.5	20.0	0.52
West: Margaret S	t (W)									
P4 Full	246	26.0	LOS C	0.4	0.4	0.87	0.87	42.6	20.0	0.47
All Pedestrians	1309	23.7	LOS C	0.8	0.8	0.83	0.83	40.4	20.0	0.50

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Site: BGU10 [BGU10 Pedestrian Mid-block Crossing at Sussex St under Exchange PI (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 3939 (?)

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 70 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Sus	sex St (S))												
2	T1	All MCs	482	0.4	482	0.4	*0.217	7.9	LOS A	4.0	28.1	0.52	0.44	0.52	25.3
Appro	ach		482	0.4	482	0.4	0.217	7.9	LOS A	4.0	28.1	0.52	0.44	0.52	25.3
North:	Suss	ex St (N)	1												
8	T1	All MCs	437	5.1	437	5.1	0.206	7.8	LOS A	3.6	26.3	0.52	0.43	0.52	23.6
Appro	ach		437	5.1	437	5.1	0.206	7.8	LOS A	3.6	26.3	0.52	0.43	0.52	23.6
All Ve	hicles		919	2.6	919	2.6	0.217	7.8	LOS A	4.0	28.1	0.52	0.44	0.52	24.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		[Ped ped	Dist J m		Rate	sec	m	m/sec
South: Sussex St	(S)									
P1 Full	141	28.5	LOS C	0.3	0.3	0.91	0.91	45.2	20.0	0.44
All Pedestrians	141	28.5	LOS C	0.3	0.3	0.91	0.91	45.2	20.0	0.44

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: BGU11 [BGU11 Pedestrian Mid-block Crossing at Kent St near Margaret St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 4109

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Ken	t St (S)													
2	T1	All MCs	408	3.9	408	3.9	*0.265	10.0	LOS A	3.0	21.8	0.71	0.58	0.71	22.6
Appro	ach		408	3.9	408	3.9	0.265	10.0	LOS A	3.0	21.8	0.71	0.58	0.71	22.6
North:	Kent	St (N)													
8	T1	All MCs	176	0.6	176	0.6	0.192	9.5	LOS A	2.2	15.2	0.67	0.53	0.67	15.0
Appro	ach		176	0.6	176	0.6	0.192	9.5	LOS A	2.2	15.2	0.67	0.53	0.67	15.0
All Ve	hicles		584	2.9	584	2.9	0.265	9.8	LOS A	3.0	21.8	0.70	0.56	0.70	20.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		red و ا	Dist j m		Rate	sec	m	m/sec					
South: Kent St (S)														
P1 Full	13	14.4	LOS B	0.0	0.0	0.80	0.80	31.1	20.0	0.64					
All Pedestrians	13	14.4	LOS B	0.0	0.0	0.80	0.80	31.1	20.0	0.64					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: BGU12 [BGU12 Sussex St / Erskine St (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 310

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ט ו		Class	٦ Total آ	IOWS HV 1	۲۱ Total آ	ows HV 1	Sath	Delay	Service	[Veh.	Dist 1	Que	Stop Rate	NO. OT Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- ,	km/h
South	: Sus	sex St (S))												
1	L2	All MCs	89	2.4	89	2.4	*0.494	36.8	LOS C	5.7	40.8	0.90	0.77	0.90	14.3
2	T1	All MCs	323	1.0	323	1.0	0.494	29.6	LOS C	9.9	69.6	0.89	0.75	0.89	15.4
Appro	ach		413	1.3	413	1.3	0.494	31.2	LOS C	9.9	69.6	0.89	0.76	0.89	15.2
East:	Erskir	ne St (E)													
4	L2	All MCs	296	1.1	296	1.1	0.330	13.8	LOS A	6.0	42.3	0.49	0.65	0.49	25.0
5	T1	All MCs	201	0.0	201	0.0	0.481	3.3	LOS A	2.4	16.7	0.24	0.26	0.24	26.4
6	R2	All MCs	38	0.0	38	0.0	0.481	8.1	LOS A	2.4	16.7	0.24	0.26	0.24	26.4
Appro	ach		535	0.6	535	0.6	0.481	9.4	LOS A	6.0	42.3	0.38	0.48	0.38	25.3
North:	Suss	ex St (N)													
7	L2	All MCs	40	18.4	40	18.4	0.075	25.1	LOS B	1.2	9.6	0.70	0.67	0.70	14.5
8	T1	All MCs	332	4.4	332	4.4	0.242	21.3	LOS B	5.1	36.8	0.73	0.61	0.73	23.9
9	R2	All MCs	24	4.3	24	4.3	*0.199	35.3	LOS C	0.9	6.7	0.89	0.72	0.89	11.4
Appro	ach		396	5.9	396	5.9	0.242	22.5	LOS B	5.1	36.8	0.74	0.62	0.74	22.4
West:	Erski	ne St (W))												
10	L2	All MCs	54	0.0	54	0.0	0.110	12.4	LOS A	2.3	16.4	0.50	0.52	0.50	13.3
11	T1	All MCs	244	2.6	244	2.6	0.531	14.2	LOS A	10.2	73.4	0.68	0.66	0.68	10.1
12	R2	All MCs	158	3.3	158	3.3	*0.531	23.2	LOS B	10.2	73.4	0.74	0.70	0.74	21.5
Appro	ach		456	2.5	456	2.5	0.531	17.1	LOS B	10.2	73.4	0.68	0.66	0.68	16.2
All Ve	hicles		1799	2.4	1799	2.4	0.531	19.3	LOS B	10.2	73.4	0.65	0.62	0.65	19.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delav	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
				[Ped	Dist]		Rate			
	ped/h	sec		ped	m			sec	m	m/sec
South: Sussex S	t (S)									
P1 Full	389	38.9	LOS D	0.9	0.9	0.94	0.94	55.6	20.0	0.36

East: Erskine St (E)									
P2 Full	136	38.5	LOS D	0.3	0.3	0.93	0.93	55.2	20.0	0.36
North: Sussex St	(N)									
P3 Full	548	39.2	LOS D	1.3	1.3	0.94	0.94	55.8	20.0	0.36
West: Erskine St	(W)									
P4 Full	84	38.4	LOS D	0.2	0.2	0.93	0.93	55.1	20.0	0.36
All Pedestrians	1158	38.9	LOS D	1.3	1.3	0.94	0.94	55.6	20.0	0.36

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Site: BGU13 [BGU13 Kent St / Erskine St (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 307

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	N/O			[Veh.	Dist]		Rate	Cycles	km/b
South	Ken	t St (S)	ven/m	70	VEII/II	70	v/C	360		Ven		_	_		NIII/11
1	L2	All MCs	133	1.6	133	1.6	0.163	19.4	LOS B	3.5	24.6	0.63	0.68	0.63	20.0
2	T1	All MCs	340	4.0	340	4.0	*0.184	15.2	LOS B	4.0	29.3	0.61	0.50	0.61	22.1
3	R2	All MCs	1	0.0	1	0.0	0.010	9.0	LOS A	0.6	1.5	0.40	0.29	0.40	24.3
Appro	ach		474	3.3	474	3.3	0.184	16.4	LOS B	4.0	29.3	0.62	0.55	0.62	21.4
East: I	Erskir	ne St (E)													
5	T1	All MCs	260	2.0	260	2.0	0.479	47.8	LOS D	5.7	40.5	0.91	0.73	0.91	5.7
6	R2	All MCs	13	0.0	13	0.0	* 0.479	60.6	LOS E	4.8	34.1	0.92	0.74	0.92	5.5
Appro	ach		273	1.9	273	1.9	0.479	48.4	LOS D	5.7	40.5	0.91	0.73	0.91	4.0
North:	Kent	St (N)													
7	L2	All MCs	1	0.0	1	0.0	0.007	8.9	LOS A	0.5	1.3	0.39	0.29	0.39	21.0
8	T1	All MCs	28	0.0	28	0.0	0.007	6.6	LOS A	0.5	1.3	0.39	0.29	0.39	25.3
9	R2	All MCs	147	0.7	147	0.7	*0.762	50.2	LOS D	6.9	48.9	1.00	0.94	1.20	6.0
Appro	ach		177	0.6	177	0.6	0.762	42.9	LOS D	6.9	48.9	0.90	0.83	1.07	8.4
West:	Erski	ne St (W))												
10	L2	All MCs	80	2.6	80	2.6	0.181	43.2	LOS D	2.5	17.9	0.74	0.70	0.74	8.1
11	T1	All MCs	206	6.6	206	6.6	0.556	43.0	LOS D	7.5	55.3	0.86	0.71	0.86	11.3
Appro	ach		286	5.5	286	5.5	0.556	43.1	LOS D	7.5	55.3	0.82	0.71	0.82	7.6
All Vel	nicles		1209	3.1	1209	3.1	0.762	33.8	LOS C	7.5	55.3	0.77	0.67	0.80	11.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	/ement	Perform	nance							
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	ith: Kent St (S)									
P1	Full	293	38.7	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
Eas	t: Erskine St (E)									

P2 Full	42	38.3	LOS D	0.1	0.1	0.92	0.92	55.0	20.0	0.36
North: Kent St (N)										
P3 Full	238	38.7	LOS D	0.6	0.6	0.93	0.93	55.3	20.0	0.36
West: Erskine St (\	N)									
P4 Full	72	38.4	LOS D	0.2	0.2	0.92	0.92	55.1	20.0	0.36
All Pedestrians	644	38.6	LOS D	0.7	0.7	0.93	0.93	55.3	20.0	0.36

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Site: BGU14 [BGU14 Sussex St / King St (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 4 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 284

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total veb/b	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
East:	King S	St (E)	VON/IT	/0	Veniin	70	0,0	000		Von					1X11/11
4a	L1	All MCs	31	0.0	31	0.0	0.072	44.9	LOS D	1.3	3.5	1.00	0.69	1.00	18.3
Appro	ach		31	0.0	31	0.0	0.072	44.9	LOS D	1.3	3.5	1.00	0.69	1.00	18.3
North	Suss	ex St (N)													
7	L2	All MCs	74	11.4	74	11.4	0.442	23.2	LOS B	11.0	79.5	0.73	0.66	0.73	20.3
8	T1	All MCs	679	2.0	679	2.0	0.442	17.7	LOS B	11.5	81.9	0.73	0.64	0.73	27.3
Appro	ach		753	2.9	753	2.9	0.442	18.3	LOS B	11.5	81.9	0.73	0.65	0.73	26.9
South	West:	King St (SW)												
30a	L1	All MCs	614	0.3	614	0.3	*0.674	16.2	LOS B	13.8	96.7	0.82	0.84	0.82	36.4
32a	R1	All MCs	1180	0.7	1180	0.7	*0.761	30.3	LOS C	23.0	162.1	0.92	0.87	0.95	27.6
32b	R3	All MCs	274	2.3	274	2.3	0.419	25.6	LOS B	8.6	61.4	0.76	0.80	0.76	31.5
Appro	ach		2067	0.8	2067	0.8	0.761	25.5	LOS B	23.0	162.1	0.87	0.85	0.89	30.4
All Ve	hicles		2851	1.4	2851	1.4	0.761	23.8	LOS B	23.0	162.1	0.84	0.80	0.85	29.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	vement	Perform	nance							
Mov ID	, Crossing	Dem. Flow	Aver. Delav	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/n	sec		ped	m			sec	m	m/sec
Sou	th: Sussex St	(S)									
P1	Full	188	40.5	LOS E	0.5	0.5	0.95	0.95	57.1	20.0	0.35
Eas	t: King St (E)										
P2	Full	172	38.5	LOS D	0.4	0.4	0.93	0.93	55.2	20.0	0.36
Nor	th: Sussex St	(N)									
P3	Full	386	38.9	LOS D	0.9	0.9	0.94	0.94	55.6	20.0	0.36
Sou	thWest: King	St (SW)									
P8	Full	355	38.8	LOS D	0.8	0.8	0.94	0.94	205.5	200.0	0.97

P8B Slip/ Bypass	311	40.7	LOS E	0.8	0.8	0.96	0.96	207.3	200.0	0.96
All Pedestrians	1412	39.4	LOS D	0.9	0.9	0.94	0.94	126.8	104.8	0.83

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Site: BGU15 [BGU15 Kent St / King St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: BGU-N2 [BGU Network 4 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 283

Site Category: NA

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar Fl	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
0 11			veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
South	: Ken	st (S)													
1	L2	All MCs	13	0.0	13	0.0	0.079	45.7	LOS D	1.4	3.7	0.97	0.67	0.97	13.0
2	T1	All MCs	303	3.1	303	3.1	*0.424	35.3	LOS C	6.3	45.3	0.93	0.75	0.93	20.8
3	R2	All MCs	178	0.0	178	0.0	*0.424	42.7	LOS D	6.0	42.7	0.95	0.76	0.95	14.3
Appro	ach		494	1.9	494	1.9	0.424	38.2	LOS C	6.3	45.3	0.94	0.75	0.94	18.5
East:	King S	St (E)													
5	T1	All MCs	7	0.0	7	0.0	0.076	43.9	LOS D	0.6	1.7	0.98	0.65	0.98	4.3
6	R2	All MCs	7	0.0	7	0.0	0.076	50.1	LOS D	0.6	1.7	0.98	0.65	0.98	13.2
Appro	ach		15	0.0	15	0.0	0.076	47.0	LOS D	0.6	1.7	0.98	0.65	0.98	9.4
North:	Kent	St (N)													
7	L2	All MCs	3	0.0	3	0.0	0.100	45.8	LOS D	1.8	4.8	0.97	0.68	0.97	10.8
8	T1	All MCs	40	0.0	40	0.0	0.100	42.4	LOS C	1.8	4.8	0.97	0.68	0.97	19.1
9	R2	All MCs	13	0.0	13	0.0	0.051	46.1	LOS D	0.5	1.4	0.96	0.66	0.96	12.2
Appro	ach		56	0.0	56	0.0	0.100	43.4	LOS D	1.8	4.8	0.97	0.67	0.97	17.3
West:	King	St (W)													
10	L2	All MCs	160	1.3	160	1.3	*0.418	23.2	LOS B	8.8	62.6	0.64	0.62	0.64	23.5
11	T1	All MCs	1088	1.4	1088	1.4	*0.418	6.0	LOS A	8.8	62.6	0.34	0.31	0.34	25.4
Appro	ach		1248	1.3	1248	1.3	0.418	8.2	LOS A	8.8	62.6	0.38	0.35	0.38	24.9
All Ve	hicles		1813	1.5	1813	1.5	0.424	17.8	LOS B	8.8	62.6	0.55	0.47	0.55	20.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	lestrian Mov	/ement	Perform	nance							
Mo∖ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Kent St (S)									
P1	Full	204	38.6	LOS D	0.5	0.5	0.93	0.93	55.3	20.0	0.36
Eas	t: King St (E)										

P2 Full	85	38.4	LOS D	0.2	0.2	0.93	0.93	55.1	20.0	0.36
North: Kent St (N)										
P3 Full	187	38.6	LOS D	0.4	0.4	0.93	0.93	55.2	20.0	0.36
West: King St (W)										
P4 Full	83	38.4	LOS D	0.2	0.2	0.93	0.93	55.1	20.0	0.36
All Pedestrians	560	38.5	LOS D	0.5	0.5	0.93	0.93	55.2	20.0	0.36

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Site: BGU16 [BGU16 Pedestrian Mid-block Crossing at Hickson Rd (North of Metro) (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA

Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Derr Fl	nand	Ar Fl	rival	Deg. Satn	Aver. Delav	Level of Service	95% Back	Of Queue	Prop.	Eff. Stop	Aver.	Aver. Speed
		01000	[Total	HV]	[Total	HV]	via	Doidy	0011100	[Veh.	Dist]	Quo	Rate	Cycles	km/h
South	: Hick	son Rd (S)	70	ven/n	70	V/C	Sec	_	ven	111	_	_	_	KIII/II
2	T1	All MCs	545	1.5	545	1.5	0.328	2.1	LOS A	1.7	12.1	0.02	0.36	0.02	37.6
Appro	ach		545	1.5	545	1.5	0.328	2.1	LOS A	1.7	12.1	0.02	0.36	0.02	37.6
North:	Hick	son Rd (N	V)												
8	T1	All MCs	298	3.2	298	3.2	0.181	2.1	LOS A	0.8	5.5	0.01	0.36	0.01	36.1
Appro	ach		298	3.2	298	3.2	0.181	2.1	LOS A	0.8	5.5	0.01	0.36	0.01	36.1
All Ve	hicles	i.	843	2.1	843	2.1	0.328	2.1	NA	1.7	12.1	0.01	0.36	0.01	37.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: BGU17 [BGU17 Pedestrian Mid-block Crossing at Hickson Rd (South of Metro) (Site Folder: Block 4 - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: BGU-N1 [BGU Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

New Site Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehicle Movement Performance

Mov	Turn	Mov	Dem	nand	Ar	rival	Dea.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
סו		Class	F	lows	F	ows	Satn	Delav	Service			Que	Ston	No of	Sneed
		01033	[Total		Totol		Oaui	Delay		[\/ab	Diet 1	Que	Data		Opecu
			liotai	ΠVJ	[IOtal	ΠVJ				[ven.	Dist J		Rale	Cycles	
			veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
South	: Hick	son Rd (S)												
2	T1	All MCs	545	1.5	545	1.5	0.343	2.3	LOS A	1.8	12.6	0.18	0.36	0.18	35.3
Appro	ach		545	15	545	15	0 343	23		1.8	12.6	0.18	0.36	0.18	35.3
Арріс	acri		545	1.5	545	1.5	0.040	2.5	LOOA	1.0	12.0	0.10	0.50	0.10	55.5
North	Hick	son Rd (N	۷)												
0	T 4		000	~ ~	000	0.0	0.400	0.0	1004	0.0	F 0	0.45	0.00	0.45	077
8	11	All MCs	298	3.2	298	3.2	0.189	2.3	LOSA	0.8	5.8	0.15	0.36	0.15	37.7
Annro	ach		298	32	298	32	0 189	23	LOSA	0.8	58	0 15	0.36	0 15	37.7
, uppic	uon		200	0.2	200	0.2	0.100	2.0	LOOM	0.0	0.0	0.10	0.00	0.10	01.1
A II \ /-			040	0.4	040	0.4	0.040	0.0	NIA	4.0	40.0	0.47	0.00	0.47	007
All Ve	nicles		843	2.1	ö43	2.1	0.343	2.3	NA	1.8	12.0	0.17	0.36	0.17	36.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: BGU18 [BGU18 Shelley St / Erskine St (Site Folder: Block 4 - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: BGU-N2 [BGU Network 3 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 305

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	ince										
Mov	Turn	Mov	Dem	hand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	[Total	HV]	۲۱ Total I	ows HV]	Sain	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			, i i i i i i i i i i i i i i i i i i i	km/h
South	: She	ley St (S)													
1	L2	All MCs	5	0.0	5	0.0	0.082	12.2	LOS A	1.7	11.9	0.43	0.36	0.43	19.7
2	T1	All MCs	92	0.0	92	0.0	0.082	7.5	LOS A	1.7	11.9	0.43	0.36	0.43	27.6
3	R2	All MCs	166	0.0	166	0.0	*0.437	15.4	LOS B	4.3	29.9	0.60	0.71	0.60	15.9
Appro	ach		263	0.0	263	0.0	0.437	12.6	LOS A	4.3	29.9	0.54	0.58	0.54	20.0
East:	Erskir	ne St (E)													
4	L2	All MCs	66	3.2	66	3.2	0.185	46.8	LOS D	2.5	17.7	0.87	0.73	0.87	11.2
5	T1	All MCs	204	0.5	204	0.5	0.637	43.3	LOS D	10.0	70.3	0.95	0.80	0.95	9.1
6	R2	All MCs	44	0.0	44	0.0	*0.637	51.8	LOS D	10.0	70.3	0.95	0.80	0.95	10.3
Appro	ach		315	1.0	315	1.0	0.637	45.2	LOS D	10.0	70.3	0.93	0.79	0.94	7.9
North:	Shel	ley St (N)													
7	L2	All MCs	115	0.9	115	0.9	0.106	12.0	LOS A	2.2	15.3	0.46	0.62	0.46	16.2
8	T1	All MCs	7	0.0	7	0.0	0.013	7.4	LOS A	0.2	1.5	0.42	0.41	0.42	26.2
9	R2	All MCs	5	0.0	5	0.0	0.013	11.3	LOS A	0.2	1.5	0.42	0.41	0.42	16.0
Appro	ach		127	0.8	127	0.8	0.106	11.7	LOS A	2.2	15.3	0.46	0.60	0.46	16.9
West:	Erski	ne St (W))												
10	L2	All MCs	22	0.0	22	0.0	0.300	37.5	LOS C	4.8	35.3	0.86	0.70	0.86	11.4
11	T1	All MCs	175	6.0	175	6.0	0.300	30.0	LOS C	4.8	35.3	0.86	0.70	0.86	6.4
12	R2	All MCs	3	33.3	33	33.3	0.300	43.3	LOS D	2.5	18.7	0.86	0.68	0.86	12.6
Appro	ach		200	5.8	200	5.8	0.300	31.1	LOS C	4.8	35.3	0.86	0.70	0.86	7.2
All Ve	hicles		905	1.7	905	1.7	0.637	27.9	LOS B	10.0	70.3	0.73	0.68	0.74	10.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ned/h	500		[Ped	Dist]		Rate	202	m	mleec
South: Shelley S	t (S)	360	_	peu		_	_	360		m/sec
P1 Full	293	38.7	LOS D	0.7	0.7	0.93	0.93	205.4	200.0	0.97

East: Erskine St (E	Ξ)									
P2 Full	42	38.3	LOS D	0.1	0.1	0.92	0.92	205.0	200.0	0.98
North: Shelley St (N)									
P3 Full	238	38.7	LOS D	0.6	0.6	0.93	0.93	205.3	200.0	0.97
West: Erskine St (W)									
P4 Full	72	38.4	LOS D	0.2	0.2	0.92	0.92	205.1	200.0	0.98
All Pedestrians	644	38.6	LOS D	0.7	0.7	0.93	0.93	205.3	200.0	0.97

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Site: MPL01 [MPL01 Hunter St / Castlereagh St / Bligh St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 244

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop Rate	Aver. No. of	Aver. Speed
			veh/h %	veh/h %	v/c	sec		veh	m		Trate	Oycics	km/h
East:	Hunte	er St (E)											
4	L2	All MCs	212 11.4	212 11.4	*0.293	15.9	LOS B	5.0	38.6	0.55	0.66	0.55	20.2
6a	R1	All MCs	289 4.4	289 4.4	0.293	8.1	LOS A	5.0	38.6	0.34	0.48	0.34	25.2
Appro	ach		501 7.4	501 7.4	0.293	11.4	LOS A	5.0	38.6	0.43	0.56	0.43	22.7
North:	Bligh	n St (N)											
7	L2	All MCs	96 33.0	96 33.0	*0.638	66.0	LOS E	4.5	33.4	1.00	0.86	1.08	10.0
8	T1	All MCs	117 5.4	117 5.4	0.177	45.5	LOS D	2.7	14.3	0.96	0.74	0.96	17.5
9b	R3	All MCs	6 50.0	6 50.0	0.177	47.7	LOS D	2.4	14.0	0.96	0.74	0.96	15.9
Appro	ach		219 18.8	219 18.8	0.638	54.5	LOS D	4.5	33.4	0.98	0.79	1.01	12.1
North\	Nest:	Hunter S	t (NW)										
27a	L1	All MCs	293 18.3	293 18.3	0.271	10.8	LOS A	4.3	34.3	0.49	0.60	0.49	20.6
29a	R1	All MCs	63 10.0	63 10.0	*0.271	12.7	LOS A	4.3	34.3	0.53	0.62	0.53	26.4
Appro	ach		356 16.9	356 16.9	0.271	11.1	LOS A	4.3	34.3	0.50	0.60	0.50	21.2
All Ve	hicles		1076 12.8	1076 12.8	0.638	20.1	LOS B	5.0	38.6	0.56	0.62	0.57	17.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestriar	Pedestrian Movement Performance														
Mov ID Crossir	Dem. ng Flow	Aver. Delay	Level of Service	AVERAGE BACK OF QUEUE [Ped Dist]		Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		ped	m Dist		Rate	sec	m	m/sec					
South: Castlereagh St (S)															
P1 Full	1346	40.5	LOS E	3.4	3.4	0.98	0.98	207.2	200.0	0.97					
East: Hunte	r St (E)														
P2 Full	1075	40.1	LOS E	2.7	2.7	0.97	0.97	206.7	200.0	0.97					
North: Bligh	St (N)														
P3 Full	1045	40.0	LOS E	2.6	2.6	0.96	0.96	206.7	200.0	0.97					
NorthWest:	Hunter St (NV	V)													
P7 Full	817	39.6	LOS D	2.0	2.0	0.95	0.95	206.3	200.0	0.97					

All Pedestrians	4283	40.1	LOS E	3.4	3.4	0.97	0.97	206.8	200.0	0.97
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Site: MPL02 [MPL02 Hunter St / Elizabeth St / Chifley Square (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 302

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total HV]	[Total HV]				[Veh.	Dist]		Rate	Cycles	
Ocuth	. 		veh/h %	veh/h %	V/C	sec	_	veh	m	_	_	_	km/h
South	: Eliza	abeth St (5)										
1	L2	All MCs	199 12.7	199 12.7	0.199	19.3	LOS B	3.8	22.5	0.47	0.64	0.47	21.2
3a	R1	All MCs	580 21.2	580 21.2	*0.850	21.2	LOS B	15.3	126.5	0.56	0.67	0.64	20.1
3	R2	All MCs	160 3.9	160 3.9	0.375	24.0	LOS B	5.0	26.1	0.81	0.76	0.81	19.7
Appro	ach		939 16.5	939 16.5	0.850	21.2	LOS B	15.3	126.5	0.58	0.68	0.63	16.7
East:	Hunte	er St (E)											
4	L2	All MCs	143 3.7	143 3.7	0.484	28.6	LOS C	8.7	45.4	0.88	0.77	0.88	16.9
5	T1	All MCs	302 3.8	302 3.8	0.484	34.0	LOS C	8.7	45.4	0.91	0.76	0.91	11.3
Approach			445 3.8	445 3.8	0.484	32.2	LOS C	8.7	45.4	0.90	0.77	0.90	13.4
North	East:	Chifley So	quare (NE)										
24b	L3	All MCs	33 9.7	33 9.7	0.253	27.7	LOS B	2.4	25.7	0.67	0.67	0.67	19.8
24a	L1	All MCs	257 32.0	257 32.0	0.381	13.3	LOS A	3.4	26.9	0.44	0.54	0.44	25.7
Appro	ach		289 29.5	289 29.5	0.381	14.9	LOS B	3.4	26.9	0.47	0.55	0.47	24.9
West:	Hunte	er St (W)											
10a	L1	All MCs	128 15.6	128 15.6	0.466	34.8	LOS C	9.7	65.3	0.91	0.79	0.91	5.8
11	T1	All MCs	137 9.2	137 9.2	0.466	26.5	LOS B	9.7	65.3	0.91	0.79	0.91	13.7
12	R2	All MCs	124 43.2	124 43.2	*0.537	35.2	LOS C	4.9	40.9	0.96	0.80	0.96	12.7
Appro	ach		389 22.2	389 22.2	0.537	32.0	LOS C	9.7	65.3	0.93	0.79	0.93	11.3
All Ve	hicles		2063 16.6	2063 16.6	0.850	24.8	LOS B	15.3	126.5	0.70	0.70	0.72	15.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	Pedestrian Movement Performance														
Mo∖ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE BACK OF QUEUE		Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
					[Ped Dist]			Rate							
		ped/h	sec		ped	m			sec	m	m/sec				
Sou	th: Elizabeth	St (S)													
P1	Full	1887	41.5	LOS E	4.8	4.8	1.00	1.00	208.2	200.0	0.96				
Eas	East: Hunter St (E)														

P2 Full	1840	41.5	LOS E	4.7	4.7	1.00	1.00	208.1	200.0	0.96
NorthEast: Chifle	y Square	(NE)								
P6 Full	1083	40.1	LOS E	2.7	2.7	0.97	0.97	206.7	200.0	0.97
West: Hunter St (W)									
P4 Full	1220	40.3	LOS E	3.0	3.0	0.97	0.97	207.0	200.0	0.97
All Pedestrians	6031	41.0	LOS E	4.8	4.8	0.99	0.99	207.7	200.0	0.96

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Site: MPL03 [MPL03 Bent St / Bligh St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 1412

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehio	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
SouthEast: Bent St (SE)													
21	L2	All MCs	177 20.2	177 20.2	0.285	7.9	LOS A	4.9	39.1	0.37	0.48	0.37	16.0
22	T1	All MCs	500 10.3	500 10.3	*0.285	2.7	LOS A	4.9	39.1	0.23	0.24	0.23	28.8
Appro	ach		677 12.9	677 12.9	0.285	4.1	LOS A	4.9	39.1	0.27	0.31	0.27	25.6
North	West:	Bent St ((NW)										
28	T1	All MCs	156 4.1	156 4.1	0.101	2.5	LOS A	1.5	11.0	0.26	0.23	0.26	25.6
29	R2	All MCs	42 12.5	42 12.5	*0.101	8.1	LOS A	0.7	5.1	0.37	0.55	0.37	16.6
Appro	ach		198 5.9	198 5.9	0.101	3.7	LOS A	1.5	11.0	0.29	0.30	0.29	22.9
All Ve	hicles		875 11.3	875 11.3	0.285	4.0	LOS A	4.9	39.1	0.27	0.30	0.27	25.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pede	Pedestrian Movement Performance														
Mov	Crossing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.				
שו	orecomig	FIOW	Delay	Service	QUEUE [Ped Dist]		Que	Rate	Time	Dist.	Speed				
		ped/h	sec		ped	m			sec	m	m/sec				
SouthEast: Bent St (SE)															
P5	Full	465	39.0	LOS D	1.1	1.1	0.94	0.94	205.7	200.0	0.97				
North	West: Bent	St (NW)													
P7	Full	783	39.6	LOS D	1.9	1.9	0.95	0.95	206.2	200.0	0.97				
South	hWest: Bligh	St (SW)													
P8	Full	699	39.4	LOS D	1.7	1.7	0.95	0.95	206.1	200.0	0.97				
All Pe	edestrians	1947	39.4	LOS D	1.9	1.9	0.95	0.95	206.1	200.0	0.97				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL04 [MPL04 Bent St / Phillip St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 242

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	[Total HV]	[Total HV]	Sain	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	East:	Bent St (SE)										
21	L2	All MCs	89 54.1	89 54.1	0.596	35.5	LOS C	1.1	10.9	0.05	0.45	0.12	25.3
22	T1	All MCs	435 5.1	435 5.1	0.429	19.8	LOS B	11.7	85.4	0.72	0.62	0.72	12.7
23a	R1	All MCs	133 3.2	133 3.2	*0.429	27.3	LOS B	5.6	40.5	0.78	0.70	0.78	18.8
Appro	ach		657 11.4	657 11.4	0.596	23.5	LOS B	11.7	85.4	0.64	0.61	0.65	15.5
North	Philli	p St (N)											
7a	L1	All MCs	102 10.3	102 10.3	* 0.212	23.0	LOS B	3.6	30.6	0.70	0.68	0.70	20.1
9a	R1	All MCs	185 18.8	185 18.8	0.212	18.4	LOS B	4.4	33.3	0.66	0.64	0.66	18.4
Appro	ach		287 15.8	287 15.8	0.212	20.0	LOS B	4.4	33.3	0.67	0.66	0.67	19.1
North	Nest:	Bent St (NW)										
27b	L3	All MCs	7 14.3	7 14.3	0.142	26.0	LOS B	2.3	16.8	0.63	0.52	0.63	19.6
28	T1	All MCs	135 2.3	135 2.3	0.142	18.5	LOS B	2.3	16.8	0.63	0.52	0.63	15.6
29	R2	All MCs	15 14.3	15 14.3	0.142	27.0	LOS B	1.8	13.4	0.63	0.53	0.63	7.8
Appro	ach		157 4.0	157 4.0	0.142	19.7	LOS B	2.3	16.8	0.63	0.52	0.63	15.3
South	West:	Phillip S	t (SW)										
30	L2	All MCs	243 26.8	243 26.8	0.416	21.1	LOS B	7.3	63.0	0.72	0.75	0.72	15.7
30a	L1	All MCs	362 20.3	362 20.3	0.337	14.9	LOS B	7.4	60.7	0.64	0.66	0.64	25.4
32	R2	All MCs	103 4.1	103 4.1	* 0.337	22.6	LOS B	4.9	37.6	0.72	0.71	0.72	20.5
Appro	Approach 708 20.2 708 20.2					18.2	LOS B	7.4	63.0	0.68	0.70	0.68	21.8
All Ve	hicles	i.	1809 14.9	1809 14.9	0.596	20.5	LOS B	11.7	85.4	0.66	0.65	0.67	18.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec				
SouthEast: Bent	St (SE)													
P5 Full	1233	40.3	LOS E	3.1	3.1	0.97	0.97	207.0	200.0	0.97				

North: Phillip St (N	North: Phillip St (N)													
P3 Full	868	39.7	LOS D	2.1	2.1	0.96	0.96	206.4	200.0	0.97				
NorthWest: Bent St (NW)														
P7 Full	1183	40.3	LOS E	2.9	2.9	0.97	0.97	206.9	200.0	0.97				
SouthWest: Phillip	o St (SW)													
P8 Full	1024	40.0	LOS D	2.5	2.5	0.96	0.96	206.6	200.0	0.97				
All Pedestrians	4308	40.1	LOS E	3.1	3.1	0.97	0.97	206.8	200.0	0.97				

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Site: MPL05 [MPL05 Pedestrian Mid-block Crossing at Castlereagh St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 245

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue [Veh. Dist]		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed		
			veh/h %	veh/h %	v/c	sec		veh	m				km/h		
North: Castlereagh St (N)															
8	T1	All MCs	405 10.1	405 10.1	*0.429	15.5	LOS B	3.9	29.5	0.87	0.71	0.87	24.4		
Appro	ach		405 10.1	405 10.1	0.429	15.5	LOS B	3.9	29.5	0.87	0.71	0.87	24.4		
All Vel	hicles		405 10.1	405 10.1	0.429	15.5	LOS B	3.9	29.5	0.87	0.71	0.87	24.4		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian I	Pedestrian Movement Performance														
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.				
ID Crossing	Vol.	Flow	Delay	Service	QUE [Ped	EUE Dist]	Que	Stop Rate	Time	Dist.	Speed				
	ped/h	ped/h	sec		ped	m			sec	m	m/sec				
South: Castlereagh St (S)															
P1 Full	6966	7333	23.1	LOS C	11.4	11.4	1.22	1.22	189.8	200.0	1.05				
All Pedestrians	6966	7333	23.1	LOS C	11.4	11.4	1.22	1.22	189.8	200.0	1.05				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL06 [MPL06 Pedestrian Mid-block Crossing at Elizabeth St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 287

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% B Que [Veh.	ack Of eue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South: Elizabeth St (S)													
2	T1	All MCs	939 16.5	939 16.5	*0.424	10.9	LOS A	9.7	77.9	0.51	0.45	0.51	29.9
Appro	ach		939 16.5	939 16.5	0.424	8.8	LOS A	9.7	77.9	0.51	0.45	0.51	27.7
North:	Eliza	beth St (N	۷)										
8	T1	All MCs	524 26.9	524 26.9	0.338	7.6	LOS A	8.0	58.3	0.47	0.41	0.47	30.5
Appro	ach		524 26.9	524 26.9	0.338	7.6	LOS A	8.0	58.3	0.47	0.41	0.47	30.5
All Ve	hicles		1463 20.2	1463 20.2	0.424	9.7	LOS A	9.7	77.9	0.50	0.44	0.50	28.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	Pedestrian Movement Performance														
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.				
ID Crossing	Vol.	Flow	Delay	Service	QUE	EUE	Que	Stop	Time	Dist.	Speed				
					[Ped	Dist]		Rate							
	ped/h	ped/h	sec		ped	m			sec	m	m/sec				
South: Elizabe	th St (S)														
P1 Full	5118	5387	49.3	LOS E	16.4	16.4	1.19	1.19	216.0	200.0	0.93				
All	5118	5387	49.3	LOS E	16.4	16.4	1.19	1.19	216.0	200.0	0.93				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL01 [MPL01 Hunter St / Castlereagh St / Bligh St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 244

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand Iows HV 1	Ar Fl [Total	rival lows HV 1	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist 1	Prop. Que	Eff. Stop Rate	Aver. No. of Cvcles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- ,	km/h
East:	Hunte	er St (E)													
4	L2	All MCs	173	14.0	173	14.0	*0.256	15.7	LOS B	4.0	31.5	0.53	0.65	0.53	20.4
6a	R1	All MCs	261	0.8	261	0.8	0.256	9.8	LOS A	4.0	31.5	0.41	0.51	0.41	23.5
Appro	ach		434	6.1	434	6.1	0.256	12.1	LOS A	4.0	31.5	0.45	0.57	0.45	22.1
North:	Bligh	n St (N)													
7	L2	All MCs	132	18.4	132	18.4	*0.704	62.1	LOS E	5.8	37.2	0.98	0.84	1.05	11.1
8	T1	All MCs	57	3.7	57	3.7	0.091	42.5	LOS D	1.2	6.5	0.84	0.63	0.84	18.9
9b	R3	All MCs	6	0.0	6	0.0	0.091	41.2	LOS C	1.1	6.0	0.84	0.64	0.84	17.1
Appro	ach		195	13.5	195	13.5	0.704	55.7	LOS D	5.8	37.2	0.93	0.77	0.98	10.8
North	West:	Hunter S	st (NW)												
27a	L1	All MCs	363	6.1	363	6.1	0.330	9.9	LOS A	5.2	37.9	0.51	0.61	0.51	20.4
29a	R1	All MCs	92	2.3	92	2.3	*0.330	11.5	LOS A	5.2	37.9	0.55	0.63	0.55	26.4
Appro	ach		455	5.3	455	5.3	0.330	10.2	LOS A	5.2	37.9	0.52	0.62	0.52	22.2
All Ve	hicles		1083	7.1	1083	7.1	0.704	19.2	LOS B	5.8	37.9	0.57	0.62	0.57	17.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	lestrian Mov	ement	Perforn	nance							
Mov ID	Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		ped/h	sec		ped	m m		Nale	sec	m	m/sec
Sou	th: Castlereag	h St (S)									
P1	Full	1129	40.2	LOS E	2.8	2.8	0.97	0.97	206.8	200.0	0.97
Eas	t: Hunter St (E)									
P2	Full	1295	40.5	LOS E	3.2	3.2	0.97	0.97	207.1	200.0	0.97
Nor	th: Bligh St (N))									
P3	Full	809	39.6	LOS D	2.0	2.0	0.95	0.95	206.3	200.0	0.97
Nor	thWest: Hunte	r St (NW	')								
P7	Full	774	39.5	LOS D	1.9	1.9	0.95	0.95	206.2	200.0	0.97

All Pedestrians	4007	40.0	LOS E	3.2	3.2	0.96	0.96	206.7	200.0	0.97
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Site: MPL02 [MPL02 Hunter St / Elizabeth St / Chifley Square (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 302

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovement	t Performa	ince									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total HV]	[Total HV]	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	Eliza	abeth St (S)		110	000		Von					
1	L2	All MCs	187 1.1	187 1.1	0.164	23.7	LOS B	3.4	17.3	0.45	0.63	0.45	21.8
3a	R1	All MCs	705 12.8	705 12.8	*0.947	38.6	LOS C	27.0	209.6	0.68	0.88	0.91	13.9
3	R2	All MCs	166 0.0	166 0.0	0.395	25.5	LOS B	5.4	26.9	0.84	0.76	0.84	19.1
Appro	ach		1059 8.7	1059 8.7	0.947	33.9	LOS C	27.0	209.6	0.66	0.81	0.82	12.4
East: I	Hunte	er St (E)											
4	L2	All MCs	105 4.0	105 4.0	0.419	29.0	LOS C	6.9	38.1	0.88	0.76	0.88	16.7
5	T1	All MCs	246 9.8	246 9.8	0.419	34.4	LOS C	6.9	38.1	0.90	0.75	0.90	11.2
Appro	ach		352 8.1	352 8.1	0.419	32.8	LOS C	6.9	38.1	0.89	0.75	0.89	13.1
North	East:	Chifley So	quare (NE)										
24b	L3	All MCs	55 0.0	55 0.0	0.233	32.8	LOS C	2.9	26.3	0.80	0.73	0.80	17.7
24a	L1	All MCs	229 22.5	229 22.5	0.351	27.5	LOS B	6.9	52.9	0.82	0.74	0.82	18.9
Appro	ach		284 18.1	284 18.1	0.351	28.5	LOS B	6.9	52.9	0.82	0.74	0.82	18.6
West:	Hunte	er St (W)											
10a	L1	All MCs	231 0.5	231 0.5	0.531	35.5	LOS C	10.1	65.3	0.94	0.82	0.94	5.1
11	T1	All MCs	140 1.5	140 1.5	0.531	26.3	LOS B	10.1	65.3	0.94	0.81	0.94	13.0
12	R2	All MCs	124 34.7	124 34.7	*0.531	35.0	LOS C	6.6	46.0	0.94	0.79	0.94	13.6
Appro	ach		495 9.4	495 9.4	0.531	32.8	LOS C	10.1	65.3	0.94	0.81	0.94	10.0
All Vel	nicles		2189 10.0	2189 10.0	0.947	32.8	LOS C	27.0	209.6	0.78	0.79	0.86	12.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mo	vement	Perform	nance							
Mo∖ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Elizabeth	St (S)									
P1	Full	1193	40.3	LOS E	3.0	3.0	0.97	0.97	206.9	200.0	0.97
Eas	t: Hunter St (I	E)									

P2 Full	1332	40.5	LOS E	3.3	3.3	0.98	0.98	207.2	200.0	0.97
NorthEast: Chifle	y Square	(NE)								
P6 Full	656	39.3	LOS D	1.6	1.6	0.95	0.95	206.0	200.0	0.97
West: Hunter St (W)									
P4 Full	764	39.5	LOS D	1.9	1.9	0.95	0.95	206.2	200.0	0.97
All Pedestrians	3944	40.1	LOS E	3.3	3.3	0.97	0.97	206.7	200.0	0.97

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Site: MPL03 [MPL03 Bent St / Bligh St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 1412

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehi	cle M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows HV]	Ar Fl [Total]	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Bacl [Veh.	< Of Queu Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	East:	Bent St (SE)												
21	L2	All MCs	141	17.2	141	17.2	0.246	6.3	LOS A	3.2	24.3	0.27	0.39	0.27	19.3
22	T1	All MCs	472	5.1	472	5.1	*0.246	2.7	LOS A	3.2	24.3	0.23	0.25	0.23	28.8
Appro	ach		613	7.9	613	7.9	0.246	3.6	LOS A	3.2	24.3	0.24	0.28	0.24	27.0
North	West:	Bent St	(NW)												
28	T1	All MCs	152	0.0	152	0.0	0.092	2.5	LOS A	1.4	10.0	0.26	0.24	0.26	25.5
29	R2	All MCs	44	4.8	44	4.8	*0.092	7.0	LOS A	0.7	4.7	0.34	0.52	0.34	18.3
Appro	ach		196	1.1	196	1.1	0.092	3.5	LOS A	1.4	10.0	0.28	0.30	0.28	23.4
All Ve	hicles		808	6.3	808	6.3	0.246	3.6	LOS A	3.2	24.3	0.25	0.29	0.25	26.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedes	strian Mov	ement	Perform	nance							
Mov	Proposing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
D C	Jossing	Flow	Delay	Service	QUE [Ped	:UE Dist]	Que	Stop Rate	lime	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
South	East: Bent S	St (SE)									
P5 F	ull	298	38.8	LOS D	0.7	0.7	0.93	0.93	205.4	200.0	0.97
North	West: Bent S	St (NW)									
P7 F	ull	589	39.2	LOS D	1.4	1.4	0.95	0.95	205.9	200.0	0.97
South	West: Bligh	St (SW)									
P8 F	ull	367	38.9	LOS D	0.9	0.9	0.94	0.94	205.5	200.0	0.97
All Peo	destrians	1255	39.0	LOS D	1.4	1.4	0.94	0.94	205.7	200.0	0.97

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL04 [MPL04 Bent St / Phillip St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 242

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	ince									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ט ו		Class	Flows [Total HV]	Flows [Total HV]	Sath	Delay	Service	[Veh.	Dist]	Que	Stop Rate	NO. OT Cycles	Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	East:	Bent St (SE)										
21	L2	All MCs	52 8.2	52 8.2	0.060	6.7	LOS A	0.3	2.4	0.18	0.47	0.18	27.9
22	T1	All MCs	355 1.8	355 1.8	0.667	40.2	LOS C	9.2	65.4	0.99	0.85	1.04	7.5
23a	R1	All MCs	32 10.0	32 10.0	*0.667	47.5	LOS D	7.6	55.0	1.00	0.86	1.06	14.3
Appro	ach		438 3.1	438 3.1	0.667	36.8	LOS C	9.2	65.4	0.90	0.81	0.94	8.9
North:	Philli	p St (N)											
7a	L1	All MCs	158 2.7	158 2.7	*0.189	12.8	LOS A	3.7	29.2	0.52	0.61	0.52	25.4
9a	R1	All MCs	215 22.1	215 22.1	0.189	9.7	LOS A	3.7	29.2	0.46	0.55	0.46	24.7
Appro	ach		373 13.8	373 13.8	0.189	11.0	LOS A	3.7	29.2	0.48	0.57	0.48	25.1
North	Nest:	Bent St (NW)										
27b	L3	All MCs	19 0.0	19 0.0	0.329	41.2	LOS C	3.2	22.6	0.85	0.68	0.85	14.2
28	T1	All MCs	115 0.0	115 0.0	0.329	32.8	LOS C	3.2	22.6	0.86	0.68	0.86	10.5
29	R2	All MCs	18 0.0	18 0.0	0.329	45.0	LOS D	2.5	17.2	0.87	0.68	0.87	4.7
Appro	ach		152 0.0	152 0.0	0.329	35.3	LOS C	3.2	22.6	0.86	0.68	0.86	10.5
South	West:	Phillip St	t (SW)										
30	L2	All MCs	258 16.3	258 16.3	0.237	9.5	LOS A	4.5	35.8	0.43	0.63	0.43	23.6
30a	L1	All MCs	438 10.3	438 10.3	0.352	7.8	LOS A	8.1	61.8	0.46	0.53	0.46	31.0
32	R2	All MCs	240 1.8	240 1.8	*0.400	17.5	LOS B	7.0	49.6	0.82	0.65	0.82	21.5
Appro	ach		936 9.8	936 9.8	0.400	10.7	LOS A	8.1	61.8	0.54	0.59	0.54	26.7
All Ve	hicles		1898 8.3	1898 8.3	0.667	18.8	LOS B	9.2	65.4	0.64	0.64	0.65	19.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Movement Performance													
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE BACK OF QUEUE		Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed			
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec			
SouthEast: Bent	St (SE)												
P5 Full	684	39.4	LOS D	1.7	1.7	0.95	0.95	206.1	200.0	0.97			

North: Phillip St (N)											
P3 Full	619	39.3	LOS D	1.5	1.5	0.95	0.95	206.0	200.0	0.97	
NorthWest: Bent St (NW)											
P7 Full	238	38.7	LOS D	0.6	0.6	0.93	0.93	205.3	200.0	0.97	
SouthWest: Phillip St (SW)											
P8 Full	398	38.9	LOS D	1.0	1.0	0.94	0.94	205.6	200.0	0.97	
All Pedestrians	1939	39.2	LOS D	1.7	1.7	0.94	0.94	205.8	200.0	0.97	

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Site: MPL05 [MPL05 Pedestrian Mid-block Crossing at Castlereagh St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 245

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Dem Fl [Total]	nand Iows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Qı [Veh.	Back Of ueue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North: Castlereagh St (N)															
8	T1	All MCs	347	5.2	347	5.2	*0.281	12.3	LOS A	2.9	21.2	0.77	0.63	0.77	26.6
Appro	ach		347	5.2	347	5.2	0.281	12.3	LOS A	2.9	21.2	0.77	0.63	0.77	26.6
All Vel	hicles		347	5.2	347	5.2	0.281	12.3	LOS A	2.9	21.2	0.77	0.63	0.77	26.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance												
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.	
ID Crossing	Vol.	Flow	Delay	Service	QUI	Que	Stop	Time	Dist.	Speed		
	1/1	1/1			[Ped	Dist J		Rate				
	ped/h	ped/h	sec		ped	m			sec	m	m/sec	
South: Castler	eagh St	(S)										
P1 Full	2018	2124	17.6	LOS B	2.5	2.5	0.93	0.93	184.3	200.0	1.09	
All	2018	2124	17.6	LOS B	2.5	2.5	0.93	0.93	184.3	200.0	1.09	
Pedestrians												

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL06 [MPL06 Pedestrian Mid-block Crossing at Elizabeth St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 287

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% B Que [Veh.	ack Of eue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: Eliza	abeth St (S	S)										
2	T1	All MCs	1022 8.8	1022 8.8	*0.431	11.0	LOS A	10.7	80.3	0.52	0.46	0.52	29.9
Appro	ach		1022 8.8	1022 8.8	0.431	8.9	LOS A	10.7	80.3	0.52	0.46	0.52	27.6
North:	Eliza	beth St (N	1)										
8	T1	All MCs	515 23.5	515 23.5	0.323	7.5	LOS A	7.8	55.2	0.47	0.41	0.47	30.6
Appro	ach		515 23.5	515 23.5	0.323	7.5	LOS A	7.8	55.2	0.47	0.41	0.47	30.6
All Ve	hicles		1537 13.7	1537 13.7	0.431	9.9	LOS A	10.7	80.3	0.50	0.44	0.50	28.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian I	Pedestrian Movement Performance														
Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.				
ID Crossing	Vol.	Flow	Delay	Service	QUI	EUE	Que	Stop	Time	Dist.	Speed				
					[Ped	Dist]		Rate							
	ped/h	ped/h	sec		ped	m			sec	m	m/sec				
South: Elizabe	eth St (S)														
P1 Full	5891	6201	51.6	LOS E	19.7	19.7	1.24	1.24	218.3	200.0	0.92				
All Pedestrians	5891	6201	51.6	LOS E	19.7	19.7	1.24	1.24	218.3	200.0	0.92				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL01 [MPL01 Hunter St / Castlereagh St / Bligh St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 244

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows H\/ 1	Ar Fl [Total]	rival ows HV/ 1	Deg. Satn	Aver. Delay	Level of Service	95% Back	COf Queue	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		T tato	Cycles	km/h
East:	Hunte	er St (E)													
4	L2	All MCs	79	0.0	79	0.0	0.100	22.1	LOS B	1.4	9.9	0.48	0.61	0.48	21.2
6a	R1	All MCs	360	0.3	360	0.3	*0.447	20.8	LOS B	7.7	53.9	0.58	0.62	0.58	21.1
Appro	ach		439	0.2	439	0.2	0.447	21.0	LOS B	7.7	53.9	0.57	0.62	0.57	16.6
North:	Bligh	n St (N)													
7	L2	All MCs	66	25.4	66 2	25.4	*0.599	47.9	LOS D	2.8	20.0	1.00	0.84	1.08	10.4
8	T1	All MCs	73	2.9	73	2.9	0.125	33.5	LOS C	1.5	7.9	0.96	0.71	0.96	18.4
9b	R3	All MCs	6	0.0	6	0.0	0.125	41.5	LOS C	1.4	7.7	0.96	0.71	0.96	16.8
Appro	ach		145	13.0	145	13.0	0.599	40.4	LOS C	2.8	20.0	0.98	0.77	1.01	14.6
North\	Nest:	Hunter S	t (NW)												
27a	L1	All MCs	284	3.0	284	3.0	0.173	8.1	LOS A	2.7	19.1	0.44	0.57	0.44	22.6
29a	R1	All MCs	51	2.1	51	2.1	*0.173	9.3	LOS A	2.7	19.1	0.49	0.59	0.49	28.0
Appro	ach		335	2.8	335	2.8	0.173	8.3	LOS A	2.7	19.1	0.45	0.57	0.45	23.8
All Ve	hicles		919	3.2	919	3.2	0.599	19.4	LOS B	7.7	53.9	0.59	0.63	0.59	17.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mov	ement l	Perforn	nance							
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Castlereag	h St (S)									
P1	Full	332	33.8	LOS D	0.7	0.7	0.93	0.93	200.4	200.0	1.00
Eas	t: Hunter St (E)									
P2	Full	342	33.8	LOS D	0.7	0.7	0.93	0.93	200.5	200.0	1.00
Nor	th: Bligh St (N)										
P3	Full	233	33.6	LOS D	0.5	0.5	0.92	0.92	200.3	200.0	1.00
Nor	thWest: Hunter	r St (NW)								
P7	Full	129	33.5	LOS D	0.3	0.3	0.92	0.92	200.2	200.0	1.00

All Pedestrians	1036	33.7	LOS D	0.7	0.7	0.92	0.92	200.4	200.0	1.00
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Site: MPL02 [MPL02 Hunter St / Elizabeth St / Chifley Square (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 302

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	ince									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total HV]	[Total HV]				[Veh.	Dist]		Rate	Cycles	
0 11	E.		veh/h %	veh/h %	V/C	sec		veh	m				km/h
South	: Eliza	abeth St (5)										
1	L2	All MCs	196 0.5	196 0.5	0.291	25.4	LOS B	4.2	21.3	0.58	0.69	0.58	19.1
3a	R1	All MCs	537 11.8	537 11.8	*0.903	51.9	LOS D	25.9	199.9	0.99	1.15	1.32	10.0
3	R2	All MCs	139 1.5	139 1.5	0.304	21.1	LOS B	3.7	19.0	0.79	0.74	0.79	20.9
Appro	ach		872 7.6	872 7.6	0.903	41.0	LOS C	25.9	199.9	0.87	0.98	1.07	10.8
East:	Hunte	er St (E)											
4	L2	All MCs	55 1.9	55 1.9	0.130	32.0	LOS C	2.2	11.2	0.73	0.66	0.73	20.4
5	T1	All MCs	243 0.0	243 0.0	*0.652	40.5	LOS C	7.9	39.3	0.91	0.80	0.95	12.6
Appro	ach		298 0.4	298 0.4	0.652	38.9	LOS C	7.9	39.3	0.87	0.77	0.91	11.1
North	East:	Chifley So	quare (NE)										
24b	L3	All MCs	13 0.0	13 0.0	0.137	20.2	LOS B	0.8	9.0	0.51	0.57	0.51	22.8
24a	L1	All MCs	193 15.3	193 15.3	0.288	17.3	LOS B	3.6	25.6	0.58	0.60	0.58	23.4
Appro	ach		205 14.4	205 14.4	0.288	17.5	LOS B	3.6	25.6	0.58	0.60	0.58	23.4
West:	Hunte	er St (W)											
10a	L1	All MCs	126 0.0	126 0.0	0.354	24.2	LOS B	8.4	50.1	0.84	0.72	0.84	7.6
11	T1	All MCs	149 0.7	149 0.7	0.354	20.1	LOS B	8.4	50.1	0.84	0.72	0.84	16.7
12	R2	All MCs	75 32.4	75 32.4	*0.354	33.4	LOS C	2.7	20.5	0.97	0.77	0.97	13.3
Appro	ach		351 7.2	351 7.2	0.354	24.4	LOS B	8.4	50.1	0.87	0.73	0.87	13.3
All Ve	hicles		1725 7.1	1725 7.1	0.903	34.5	LOS C	25.9	199.9	0.83	0.85	0.94	12.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	Pedestrian Movement Performance														
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
					[Ped	Dist]		Rate							
		ped/h	sec		ped	m			sec	m	m/sec				
Sou	th: Elizabeth	St (S)													
P1	Full	104	33.5	LOS D	0.2	0.2	0.92	0.92	200.1	200.0	1.00				
Eas	t: Hunter St (E	E)													

P2 Full	114	33.5	LOS D	0.2	0.2	0.92	0.92	200.1	200.0	1.00
NorthEast: Chifley	Square	(NE)								
P6 Full	61	33.4	LOS D	0.1	0.1	0.91	0.91	200.1	200.0	1.00
West: Hunter St (W	/)									
P4 Full	246	33.7	LOS D	0.5	0.5	0.92	0.92	200.3	200.0	1.00
All Pedestrians	525	33.5	LOS D	0.5	0.5	0.92	0.92	200.2	200.0	1.00

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Site: MPL03 [MPL03 Bent St / Bligh St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 1412

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total]	nand Iows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Bac [Veh.	k Of Queu Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
SouthEast: Bent St (SE)															
21	L2	All MCs	111	17.1	111	17.1	0.106	7.9	LOS A	1.0	7.9	0.24	0.53	0.24	17.0
22	T1	All MCs	426	2.2	426	2.2	*0.386	4.9	LOS A	4.4	31.4	0.29	0.25	0.29	28.5
Appro	ach		537	5.3	537	5.3	0.386	5.5	LOS A	4.4	31.4	0.28	0.31	0.28	22.9
North	West:	Bent St ((NW)												
28	T1	All MCs	159	0.0	159	0.0	0.086	2.8	LOS A	1.3	9.0	0.30	0.28	0.30	23.6
29	R2	All MCs	35	0.0	35	0.0	* 0.086	8.3	LOS A	0.8	5.7	0.38	0.45	0.38	18.6
Appro	ach		194	0.0	194	0.0	0.086	3.8	LOS A	1.3	9.0	0.32	0.31	0.32	22.5
All Ve	hicles		731	3.9	731	3.9	0.386	5.1	LOS A	4.4	31.4	0.29	0.31	0.29	22.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedes	strian Move	ement F	Perforn	nance							
Mov	[Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
	rossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	lime	Dist.	Speed
	p	oed/h	sec		ped	m			sec	m	m/sec
SouthE	East: Bent St	(SE)									
P5 Fi	ull	83	33.4	LOS D	0.2	0.2	0.92	0.92	200.1	200.0	1.00
NorthW	Vest: Bent St	(NW)									
P7 Fu	ull	173	33.5	LOS D	0.4	0.4	0.92	0.92	200.2	200.0	1.00
SouthV	Nest: Bligh S	st (SW)									
P8 Fu	ull	92	33.4	LOS D	0.2	0.2	0.92	0.92	200.1	200.0	1.00
All Ped	destrians	347	33.5	LOS D	0.4	0.4	0.92	0.92	200.2	200.0	1.00

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL04 [MPL04 Bent St / Phillip St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: MPL-N1 [MPL Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 242

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 80 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	Flows [Total HV]	Flows [Total HV]	Sath	Delay	Service	[Veh.	Dist]	Que	Stop Rate	NO. OF Cycles	Speed
South	East:	Bent St (SE)		V/C	360		VCII		_	_		K111/11
21	L2	All MCs	17 12.5	17 12.5	0.015	5.6	LOS A	0.1	0.7	0.14	0.45	0.14	28.4
22	T1	All MCs	326 0.6	326 0.6	*0.390	26.7	LOS B	6.6	46.8	0.86	0.72	0.86	10.5
23a	R1	All MCs	52 0.0	52 0.0	0.390	30.7	LOS C	5.6	39.6	0.87	0.73	0.87	18.4
Appro	ach		395 1.1	395 1.1	0.390	26.3	LOS B	6.6	46.8	0.83	0.71	0.83	12.0
North:	Philli	p St (N)											
7a	L1	All MCs	100 1.1	100 1.1	0.158	15.3	LOS B	3.0	21.8	0.59	0.63	0.59	24.2
9a	R1	All MCs	175 15.7	175 15.7	*0.158	12.5	LOS A	3.0	21.8	0.56	0.59	0.56	22.1
Appro	ach		275 10.3	275 10.3	0.158	13.5	LOS A	3.0	21.8	0.57	0.60	0.57	23.0
North	West:	Bent St (NW)										
27b	L3	All MCs	8 0.0	8 0.0	0.167	26.1	LOS B	2.3	15.9	0.70	0.57	0.70	18.7
28	T1	All MCs	137 0.0	137 0.0	0.167	20.4	LOS B	2.3	15.9	0.70	0.57	0.70	14.9
29	R2	All MCs	14 0.0	14 0.0	0.167	27.8	LOS B	1.9	13.4	0.70	0.57	0.70	7.3
Appro	ach		159 0.0	159 0.0	0.167	21.3	LOS B	2.3	15.9	0.70	0.57	0.70	14.6
South	West	Phillip S	t (SW)										
30	L2	All MCs	211 12.5	211 12.5	0.219	14.1	LOS A	4.7	36.3	0.61	0.65	0.61	19.8
30a	L1	All MCs	287 12.1	287 12.1	0.271	10.7	LOS A	5.9	45.6	0.57	0.58	0.57	28.8
32	R2	All MCs	165 1.3	165 1.3	*0.293	17.9	LOS B	4.3	30.8	0.83	0.69	0.83	21.3
Appro	ach		663 9.5	663 9.5	0.293	13.6	LOS A	5.9	45.6	0.65	0.63	0.65	24.5
All Ve	hicles		1492 6.4	1492 6.4	0.390	17.7	LOS B	6.6	46.8	0.69	0.64	0.69	19.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec
SouthEast: Bent	St (SE)									
P5 Full	175	33.6	LOS D	0.4	0.4	0.92	0.92	200.2	200.0	1.00

North: Phillip St (N))									
P3 Full	68	33.4	LOS D	0.1	0.1	0.92	0.92	200.1	200.0	1.00
NorthWest: Bent St	t (NW)									
P7 Full	64	33.4	LOS D	0.1	0.1	0.91	0.91	200.1	200.0	1.00
SouthWest: Phillip	St (SW)									
P8 Full	144	33.5	LOS D	0.3	0.3	0.92	0.92	200.2	200.0	1.00
All Pedestrians	452	33.5	LOS D	0.4	0.4	0.92	0.92	200.2	200.0	1.00

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Site: MPL05 [MPL05 Pedestrian Mid-block Crossing at Castlereagh St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 245

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovemen	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	and	Ar	rival	Deg.	Aver.	Level of	95% E	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI	ows	F	lows	Satn	Delay	Service	Qu	eue	Que	Stop	No. of	Speed
			[Total I	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North: Castlereagh St (N)															
8	T1	All MCs	216	1.5	216	1.5	*0.180	12.6	LOS A	1.8	12.6	0.76	0.60	0.76	26.4
Appro	ach		216	1.5	216	1.5	0.180	12.6	LOS A	1.8	12.6	0.76	0.60	0.76	26.4
All Vel	nicles		216	1.5	216	1.5	0.180	12.6	LOS A	1.8	12.6	0.76	0.60	0.76	26.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian N	loveme	ent Perf	ormand	e:							
Mov Crossing	Input	Dem.	Aver.	Level of	AVERAGE		Prop.	Eff. Stop	Travel	Travel	Aver.
	voi.	11000	Delay	Oervice	[Ped	Dist]	Que	Rate	TITIC	Dist.	opeeu
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Castler	eagh St	(S)									
P1 Full	907	955	16.7	LOS B	1.1	1.1	0.88	0.88	183.4	200.0	1.09
All Pedestrians	907	955	16.7	LOS B	1.1	1.1	0.88	0.88	183.4	200.0	1.09

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: MPL06 [MPL06 Pedestrian Mid-block Crossing at Elizabeth St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 287

Site Category: (None)

Pedestrian Crossing (Signalised) - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	95% B Qu	ack Of eue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total HV] veh/h %	[Total HV] veh/h %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Eliza	abeth St (S	S)										
2	T1	All MCs	901 7.5	901 7.5	* 0.374	10.4	LOS A	8.9	66.2	0.49	0.43	0.49	30.3
Appro	ach		901 7.5	901 7.5	0.374	8.4	LOS A	8.9	66.2	0.49	0.43	0.49	28.1
North:	Eliza	beth St (N	1)										
8	T1	All MCs	297 20.6	297 20.6	0.191	6.8	LOS A	4.2	29.4	0.43	0.36	0.43	31.3
Appro	ach		297 20.6	297 20.6	0.191	6.8	LOS A	4.2	29.4	0.43	0.36	0.43	31.3
All Ve	hicles		1198 10.7	1198 10.7	0.374	9.5	LOS A	8.9	66.2	0.48	0.41	0.48	28.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	loveme	nt Perf	ormand	e:							
Mov	Input	Dem.	Aver.	Level of	AVERAGE	E BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QU	EUE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Elizabe	eth St (S)										
P1 Full	798	840	39.7	LOS D	2.1	2.1	0.96	0.96	206.3	200.0	0.97
All	798	840	39.7	LOS D	2.1	2.1	0.96	0.96	206.3	200.0	0.97
Pedestrians											

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT01 [PIT01 Pitt St / Bathurst St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 2312

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 60 seconds (Network Site User-Given Phase Times)

Vehio	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total I veb/b	and ows HV] %	Ar Fl [Total veb/b	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	k Of Queu Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	: Pitt s	St (S)	VG1/11	70	VCH/H	70	V/C	300		Ven					KIT/TT
2	T1	All MCs	249	10.5	249	10.5	0.334	25.2	LOS B	3.7	27.9	0.86	0.69	0.86	20.4
3	R2	All MCs	123	9.4	123	9.4	*0.618	42.8	LOS D	3.6	27.5	0.97	0.83	1.08	16.0
Appro	ach		373	10.2	373	10.2	0.618	31.0	LOS C	3.7	27.9	0.90	0.74	0.93	16.2
West:	Bath	urst St (N	/)												
10	L2	All MCs	293	5.4	293	5.4	*0.437	16.8	LOS B	5.5	40.6	0.68	0.73	0.68	15.3
11	T1	All MCs	952	5.5	952	5.5	0.332	8.9	LOS A	5.5	40.1	0.59	0.50	0.59	20.4
Appro	ach		1244	5.5	1244	5.5	0.437	10.7	LOS A	5.5	40.6	0.61	0.56	0.61	17.6
All Ve	hicles		1617	6.6	1617	6.6	0.618	15.4	LOS B	5.5	40.6	0.68	0.60	0.68	17.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m		rtato	sec	m	m/sec
South: Pitt St (S)										
P1 Full	1554	26.0	LOS C	2.5	2.5	0.96	0.96	42.6	20.0	0.47
East: Bathurst St	(E)									
P2 Full	526	24.8	LOS C	0.8	0.8	0.92	0.92	41.5	20.0	0.48
North: Pitt St (N)										
P3 Full	765	25.1	LOS C	1.2	1.2	0.93	0.93	41.8	20.0	0.48
West: Bathurst St	(W)									
P4 Full	980	24.4	LOS C	1.5	1.5	0.92	0.92	41.1	20.0	0.49
All Pedestrians	3825	25.2	LOS C	2.5	2.5	0.94	0.94	41.9	20.0	0.48

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT02 [PIT02 Castlereagh St / Bathurst St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 2281

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 60 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total	hand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
North	Cast	lereadh S	ven/n St (N)	%	ven/h	%	V/C	sec	_	ven	m			_	Km/h
7	1.0		10	<u></u>	10	<u></u>	0.072	21.1		0.6	E	0.74	0.62	0.74	11 E
1		AII MCS	19	22.2	19	ZZ.Z	0.072	21.1	LUSB	0.6	5.5	0.74	0.03	0.74	11.5
8	T1	All MCs	246	9.0	246	9.0	*0.389	16.8	LOS B	5.5	40.3	0.81	0.67	0.81	26.4
Appro	ach		265	9.9	265	9.9	0.389	17.1	LOS B	5.5	40.3	0.80	0.67	0.80	25.2
West:	Bath	urst St (N	/)												
11	T1	All MCs	958	5.9	958	5.9	0.324	4.0	LOS A	3.5	25.9	0.29	0.27	0.29	28.7
12	R2	All MCs	117	6.3	117	6.3	*0.324	12.2	LOS A	3.1	22.8	0.46	0.53	0.46	28.8
Appro	ach		1075	6.0	1075	6.0	0.324	4.9	LOS A	3.5	25.9	0.31	0.30	0.31	28.8
All Ve	hicles		1340	6.8	1340	6.8	0.389	7.3	LOS A	5.5	40.3	0.41	0.37	0.41	27.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance								
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed	
	ped/h	sec		ped	m		Trate	sec	m	m/sec	
South: Castlereag	gh St (S)										
P1 Full	417	23.8	LOS C	0.6	0.6	0.90	0.90	40.5	20.0	0.49	
East: Bathurst St (E)											
P2 Full	273	23.7	LOS C	0.4	0.4	0.89	0.89	40.3	20.0	0.50	
North: Castlereag	h St (N)										
P3 Full	763	24.2	LOS C	1.2	1.2	0.91	0.91	40.8	20.0	0.49	
West: Bathurst St	(W)										
P4 Full	367	23.8	LOS C	0.6	0.6	0.90	0.90	40.4	20.0	0.49	
All Pedestrians	1820	23.9	LOS C	1.2	1.2	0.90	0.90	40.6	20.0	0.49	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT03 [PIT03 Park St / Castlereagh St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 250

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfo	rma	nce									
Mov ID	Turn	Mov Class	Dema Flo [Total H veh/h	and ows IV] %	Arrival Flows [Total HV] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
East:	Park \$	St (E)												
4 5 Appro	L2 T1 ach	All MCs All MCs	120 389 1 509 1	4.4 8.1 4.9	120 4.4 389 18.1 509 14.9	0.169 * 0.491 0.491	21.9 17.6 18.6	LOS B LOS B LOS B	3.4 10.5 10.5	24.5 77.4 77.4	0.68 0.72 0.71	0.69 0.62 0.64	0.68 0.72 0.71	7.0 8.3 8.0
North	Cast	lereagh S	St (N)											
7	L2	All MCs	104	3.0	104 3.0	0.201	29.6	LOS C	3.5	25.0	0.79	0.73	0.79	18.8
8	T1	All MCs	106	9.9	106 9.9	*0.644	58.3	LOS E	7.8	59.7	0.97	0.83	1.01	15.1
9	R2	All MCs	78 1	3.5	78 13.5	0.644	68.5	LOS E	7.8	59.7	0.98	0.84	1.01	15.1
Appro	ach		288	8.4	288 8.4	0.644	50.7	LOS D	7.8	59.7	0.91	0.79	0.93	13.2
West:	Park	St (W)												
11	T1	All MCs	142 3	87.8	142 37.8	0.307	14.1	LOS A	4.8	43.6	0.65	0.57	0.65	16.5
12	R2	All MCs	39 2	27.0	39 27.0	*0.307	20.0	LOS B	4.8	43.6	0.65	0.57	0.65	13.3
Appro	ach		1813	85.5	181 35.5	0.307	15.4	LOS B	4.8	43.6	0.65	0.57	0.65	15.9
All Ve	hicles		979 1	6.8	979 16.8	0.644	27.4	LOS B	10.5	77.4	0.76	0.67	0.77	11.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mov	vement	Perform	nance							
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		1.0			[Ped	Dist]		Rate			
		ped/n	sec		ped	m			sec	m	m/sec
Sou	th: Castlereag	gh St (S)									
P1	Full	372	38.9	LOS D	0.9	0.9	0.94	0.94	55.5	20.0	0.36
Eas	st: Park St (E)										
P2	Full	354	38.8	LOS D	0.8	0.8	0.94	0.94	55.5	20.0	0.36
Nor	th: Castlereag	h St (N)									
P3	Full	1217	40.3	LOS E	3.0	3.0	0.97	0.97	57.0	20.0	0.35
We	st: Park St (W)									
P4	Full	482	39.1	LOS D	1.2	1.2	0.94	0.94	55.7	20.0	0.36

All Pedestrians	2424	39.6	LOS D	3.0	3.0	0.95	0.95	56.3	20.0	0.36
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Site: PIT04 [PIT04 Park St / Pitt St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 235

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	: Pitt s	St (S)	ven/n %	ven/n %	V/C	sec	_	ven	m	_	_	_	Km/n
1	L2	All MCs	146 15.1	146 15.1	0.458	21.8	LOS B	3.0	24.1	0.92	0.78	0.92	18.1
2	T1	All MCs	232 12.3	232 12.3	*0.458	18.2	LOS B	4.2	32.2	0.83	0.69	0.83	28.7
3	R2	All MCs	73 13.0	73 13.0	0.223	25.7	LOS B	1.4	11.1	0.87	0.73	0.87	15.1
Appro	ach		451 13.3	451 13.3	0.458	20.6	LOS B	4.2	32.2	0.87	0.72	0.87	22.0
East:	Park	St (E)											
5	T1	All MCs	403 16.2	403 16.2	0.796	17.2	LOS B	11.0	81.1	0.93	0.87	1.02	17.2
6	R2	All MCs	100 6.3	100 6.3	*0.796	23.7	LOS B	11.0	81.1	0.96	0.91	1.06	23.1
Appro	ach		503 14.2	503 14.2	0.796	18.5	LOS B	11.0	81.1	0.93	0.88	1.03	18.8
West:	Park	St (W)											
10	L2	All MCs	1 100. 0	1 ^{100.} 0	0.194	18.6	LOS B	1.4	14.5	0.70	0.56	0.70	27.9
11	T1	All MCs	86 62.2	86 62.2	0.194	10.6	LOS A	1.4	14.5	0.70	0.56	0.70	16.4
Appro	ach		87 62.7	87 62.7	0.194	10.7	LOS A	1.4	14.5	0.70	0.56	0.70	16.7
All Ve	hicles		1041 17.9	1041 17.9	0.796	18.8	LOS B	11.0	81.1	0.89	0.78	0.93	20.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	vement	Perform	nance							
Mov Crossing	Dem. Flow	Aver.	Level of	AVERAGE		Prop.	Eff. Stop	Travel Time	Travel Dist	Aver.
	110 10	Belay		[Ped	Dist]	Que	Rate	mile	Dist.	opeeu
	ped/h	sec		ped	m			sec	m	m/sec
South: Pitt St (S)										
P1 Full	957	16.7	LOS B	1.1	1.1	0.88	0.88	33.4	20.0	0.60
East: Park St (E)										
P2 Full	529	16.4	LOS B	0.6	0.6	0.86	0.86	33.1	20.0	0.60
North: Pitt St (N)										
P3 Full	1292	18.8	LOS B	1.6	1.6	0.94	0.94	35.5	20.0	0.56
West: Park St (W)									
P4 Full	786	16.6	LOS B	0.9	0.9	0.87	0.87	33.3	20.0	0.60

All Pedestrians	3564	17.4	LOS B	1.6	1.6	0.90	0.90	34.1	20.0	0.59
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Site: PIT01 [PIT01 Pitt St / Bathurst St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 2312

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 85 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total]	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pitt \$	St (S)													
2	T1	All MCs	293	0.4	293	0.4	0.305	28.4	LOS B	5.0	35.2	0.86	0.69	0.86	17.1
3	R2	All MCs	145	2.2	145	2.2	*0.427	37.7	LOS C	5.5	39.1	0.93	0.79	0.93	14.2
Appro	ach		438	1.0	438	1.0	0.427	31.5	LOS C	5.5	39.1	0.88	0.72	0.88	16.0
West:	Bathu	urst St (N	/)												
10	L2	All MCs	198	2.1	198	2.1	0.206	15.7	LOS B	3.9	27.8	0.51	0.65	0.51	16.2
11	T1	All MCs	1108	1.8	1108	1.8	*0.341	9.5	LOS A	8.0	57.1	0.52	0.46	0.52	19.7
Appro	ach		1306	1.9	1306	1.9	0.341	10.4	LOS A	8.0	57.1	0.52	0.49	0.52	17.9
All Ve	hicles		1744	1.6	1744	1.6	0.427	15.7	LOS B	8.0	57.1	0.61	0.55	0.61	17.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		red و ا	DIST J m		Rate	sec	m	m/sec
South: Pitt St (S)										
P1 Full	1935	39.9	LOS D	4.7	4.7	1.01	1.01	56.6	20.0	0.35
East: Bathurst St	(E)									
P2 Full	625	37.7	LOS D	1.4	1.4	0.95	0.95	54.4	20.0	0.37
North: Pitt St (N)										
P3 Full	1015	38.3	LOS D	2.4	2.4	0.97	0.97	55.0	20.0	0.36
West: Bathurst St	(W)									
P4 Full	1293	37.8	LOS D	3.0	3.0	0.97	0.97	54.5	20.0	0.37
All Pedestrians	4867	38.8	LOS D	4.7	4.7	0.98	0.98	55.4	20.0	0.36

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT02 [PIT02 Castlereagh St / Bathurst St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 2281

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 85 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total I	and ows HV]	Ar Fl [Total]	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
North: Castlereagh St (N)											_	_	Km/n		
7	L2	All MCs	28	0.0	28	0.0	0.150	23.6	LOS B	1.9	19.7	0.67	0.60	0.67	11.5
8	T1	All MCs	251	18.9	251	18.9	*0.260	16.9	LOS B	5.6	40.1	0.68	0.58	0.68	26.2
Appro	ach		279	17.0	279	17.0	0.260	17.6	LOS B	5.6	40.1	0.68	0.58	0.68	24.5
West:	Bathu	urst St (W	')												
11	T1	All MCs	1164	1.9	1164	1.9	0.397	18.3	LOS B	9.3	66.5	0.74	0.64	0.74	15.1
12	R2	All MCs	89	1.2	89	1.2	*0.397	32.0	LOS C	9.3	65.7	0.85	0.75	0.85	20.7
Appro	ach		1254	1.8	1254	1.8	0.397	19.3	LOS B	9.3	66.5	0.74	0.64	0.74	15.5
All Ve	hicles		1533	4.6	1533	4.6	0.397	19.0	LOS B	9.3	66.5	0.73	0.63	0.73	17.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Castlereag	gh St (S)									
P1 Full	1377	38.0	LOS D	3.2	3.2	0.97	0.97	54.6	20.0	0.37
East: Bathurst St	(E)									
P2 Full	320	36.3	LOS D	0.7	0.7	0.93	0.93	52.9	20.0	0.38
North: Castlereag	h St (N)									
P3 Full	934	37.2	LOS D	2.2	2.2	0.95	0.95	53.9	20.0	0.37
West: Bathurst St	: (W)									
P4 Full	555	36.6	LOS D	1.3	1.3	0.94	0.94	53.3	20.0	0.38
All Pedestrians	3185	37.4	LOS D	3.2	3.2	0.96	0.96	54.0	20.0	0.37

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT03 [PIT03 Park St / Castlereagh St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 250

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	cle M	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [Total HV] veh/h %	Arrival Flows [Total HV] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
East:	Park \$	St (E)											
4 5 Appro	L2 T1 ach	All MCs All MCs	32 3.3 329 20.8 361 19.2	32 3.3 329 20.8 361 19.2	0.095 * 0.308 0.308	22.0 16.3 16.8	LOS B LOS B LOS B	1.4 7.4 7.4	13.2 52.8 52.8	0.64 0.66 0.66	0.60 0.56 0.56	0.64 0.66 0.66	7.7 8.7 8.6
North	Cast	lereagh S	St (N)										
7	L2	All MCs	157 0.0	157 0.0	0.262	27.8	LOS B	5.1	35.8	0.78	0.74	0.78	19.3
8	T1	All MCs	205 21.5	205 21.5	0.753	56.8	LOS E	12.4	90.3	0.96	0.86	1.05	15.4
9	R2	All MCs	113 6.5	113 6.5	*0.753	72.9	LOS F	12.4	90.3	0.99	0.91	1.10	14.9
Appro	ach		475 10.9	475 10.9	0.753	51.1	LOS D	12.4	90.3	0.91	0.83	0.97	13.1
West:	Park	St (W)											
11	T1	All MCs	149 33.1	149 33.1	0.213	15.8	LOS B	3.8	27.1	0.65	0.55	0.65	15.9
12	R2	All MCs	42 5.0	42 5.0	*0.213	19.8	LOS B	3.8	27.1	0.67	0.58	0.67	12.4
Appro	ach		192 26.9	192 26.9	0.213	16.6	LOS B	3.8	27.1	0.65	0.55	0.65	15.2
All Ve	hicles		1027 16.8	1027 16.8	0.753	32.6	LOS C	12.4	90.3	0.77	0.69	0.80	12.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perform	nance							
Mo∖ ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		1.0			[Ped	Dist]		Rate			-
		ped/n	sec		ped	m			sec	m	m/sec
Sou	th: Castlereag	h St (S)									
P1	Full	304	38.8	LOS D	0.7	0.7	0.93	0.93	55.4	20.0	0.36
Eas	t: Park St (E)										
P2	Full	641	39.3	LOS D	1.6	1.6	0.95	0.95	56.0	20.0	0.36
Nor	th: Castlereag	h St (N)									
P3	Full	2415	42.6	LOS E	6.3	6.3	1.03	1.03	59.2	20.0	0.34
We	st: Park St (W))									
P4	Full	857	39.7	LOS D	2.1	2.1	0.96	0.96	56.4	20.0	0.35

All Pedestrians	4217	41.2	LOS E	6.3	6.3	0.99	0.99	57.9	20.0	0.35
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Site: PIT04 [PIT04 Park St / Pitt St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 235

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	Vehicle Movement Performance Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Queue P <u>rop. Eff. Aver. Aver.</u>														
Mov ID	Turn	Mov Class	Demand Flows [Total HV]	Arrival Flows [Total HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed		
South	· Pitt 9	St (S)	veh/h %	veh/h %	v/c	sec	_	veh	m	_	_	_	km/h		
Coun		01(0)													
1	L2	All MCs	151 2.8	151 2.8	0.417	21.5	LOS B	3.2	22.6	0.92	0.77	0.92	18.2		
2	T1	All MCs	219 0.5	219 0.5	0.379	16.7	LOS B	3.9	27.6	0.81	0.66	0.81	29.0		
3	R2	All MCs	79 1.3	79 1.3	0.216	24.5	LOS B	1.6	11.1	0.87	0.73	0.87	15.2		
Appro	ach		448 1.4	448 1.4	0.417	19.7	LOS B	3.9	27.6	0.85	0.71	0.85	22.2		
East:	Park \$	St (E)													
5	T1	All MCs	452 16.3	452 16.3	0.657	12.5	LOS A	8.2	59.0	0.86	0.74	0.86	20.6		
6	R2	All MCs	37 0.0	37 0.0	*0.657	18.2	LOS B	8.2	59.0	0.88	0.77	0.88	26.3		
Appro	ach		488 15.1	488 15.1	0.657	13.0	LOS A	8.2	59.0	0.86	0.74	0.86	21.3		
West:	Park	St (W)													
10	L2	All MCs	1 100. 0	1 ^{100.} 0	0.238	18.9	LOS B	1.7	18.3	0.72	0.58	0.72	27.8		
11	T1	All MCs	109 57.7	109 57.7	0.238	10.8	LOS A	1.7	18.3	0.72	0.58	0.72	16.3		
Appro	ach		111 58.1	111 58.1	0.238	10.9	LOS A	1.7	18.3	0.72	0.58	0.72	16.5		
All Ve	hicles		1047 13.8	1047 13.8	0.657	15.6	LOS B	8.2	59.0	0.84	0.71	0.84	21.5		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pede	estrian Mov	vement	Perform	nance							
Mov ID	Crossing	Dem. Aver. Level of Flow Delay Service		AVERAGE QUE	BACK OF UE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed	
		ped/h	sec		ped	m			sec	m	m/sec
Sout	h: Pitt St (S)										
P1	Full	1979	17.5	LOS B	2.4	2.4	0.92	0.92	34.2	20.0	0.59
East:	Park St (E)										
P2	Full	1314	17.0	LOS B	1.5	1.5	0.89	0.89	33.7	20.0	0.59
North	n: Pitt St (N)										
P3	Full	2243	19.6	LOS B	2.9	2.9	0.98	0.98	36.3	20.0	0.55
West	:: Park St (W))									
P4	Full	1768	17.3	LOS B	2.1	2.1	0.91	0.91	34.0	20.0	0.59

All Pedestrians	7304	18.0	LOS B	2.9	2.9	0.93	0.93	34.7	20.0	0.58
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Site: PIT01 [PIT01 Pitt St / Bathurst St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 2312

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 45 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	hand lows HV]	Ar Fl [Total]	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	· Ditt (St (S)	ven/h	%	ven/n	%	V/C	sec	_	ven	m	_	_	_	Km/h
South	. F III •	51 (3)													
2	T1	All MCs	1	0.0	1	0.0	0.001	13.0	LOS A	0.0	0.0	0.75	0.43	0.75	20.6
3	R2	All MCs	1	0.0	1	0.0	0.001	18.6	LOS B	0.0	0.0	0.84	0.54	0.84	17.9
Appro	ach		2	0.0	2	0.0	0.001	15.8	LOS B	0.0	0.0	0.79	0.49	0.79	19.2
West:	Bath	urst St (N	/)												
10	L2	All MCs	252	1.7	252	1.7	*0.360	14.6	LOS B	4.0	28.3	0.74	0.74	0.74	15.3
11	T1	All MCs	938	2.1	938	2.1	0.361	8.7	LOS A	4.6	33.0	0.68	0.58	0.68	19.9
Appro	ach		1189	2.0	1189	2.0	0.361	9.9	LOS A	4.6	33.0	0.69	0.61	0.69	18.4
All Ve	hicles		1192	2.0	1192	2.0	0.361	10.0	LOS A	4.6	33.0	0.69	0.61	0.69	18.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m		Rate	sec	m	m/sec
South: Pitt St (S)										
P1 Full	1213	17.8	LOS B	1.4	1.4	0.91	0.91	34.5	20.0	0.58
East: Bathurst St	(E)									
P2 Full	265	17.1	LOS B	0.3	0.3	0.88	0.88	33.8	20.0	0.59
North: Pitt St (N)										
P3 Full	481	17.2	LOS B	0.5	0.5	0.88	0.88	33.9	20.0	0.59
West: Bathurst St	: (W)									
P4 Full	918	16.7	LOS B	1.0	1.0	0.88	0.88	33.3	20.0	0.60
All Pedestrians	2877	17.3	LOS B	1.4	1.4	0.89	0.89	34.0	20.0	0.59

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT02 [PIT02 Castlereagh St / Bathurst St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 2281

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 45 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total veb/b	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
North	Cast	lereagh S	St (N)	70	VCH/H	70	V/C	300		VCII					KIII/II
7	L2	All MCs	8	12.5	8	12.5	0.027	16.5	LOS B	0.2	1.5	0.74	0.61	0.74	12.3
8	T1	All MCs	143	2.2	143	2.2	*0.233	12.8	LOS A	2.4	16.6	0.78	0.62	0.78	28.8
Appro	ach		152	2.8	152	2.8	0.233	13.0	LOS A	2.4	16.6	0.78	0.62	0.78	27.7
West:	Bath	urst St (N	/)												
11	T1	All MCs	866	1.6	866	1.6	*0.306	2.7	LOS A	1.3	9.4	0.25	0.23	0.25	31.7
12	R2	All MCs	72	8.8	72	8.8	0.306	7.8	LOS A	1.0	7.6	0.25	0.36	0.25	32.8
Appro	ach		938	2.1	938	2.1	0.306	3.0	LOS A	1.3	9.4	0.25	0.24	0.25	31.9
All Ve	hicles		1089	2.2	1089	2.2	0.306	4.4	LOS A	2.4	16.6	0.32	0.29	0.32	30.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		ped	m		Trate	sec	m	m/sec
South: Castlereag	gh St (S)									
P1 Full	623	16.5	LOS B	0.7	0.7	0.87	0.87	33.1	20.0	0.60
East: Bathurst St	(E)									
P2 Full	99	16.1	LOS B	0.1	0.1	0.85	0.85	32.8	20.0	0.61
North: Castlereag	h St (N)									
P3 Full	225	16.2	LOS B	0.2	0.2	0.85	0.85	32.9	20.0	0.61
West: Bathurst St	: (W)									
P4 Full	235	16.2	LOS B	0.3	0.3	0.85	0.85	32.9	20.0	0.61
All Pedestrians	1182	16.3	LOS B	0.7	0.7	0.86	0.86	33.0	20.0	0.61

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: PIT03 [PIT03 Park St / Castlereagh St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 250

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total] veh/h	rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
East:	Park \$	St (E)													
4 5 Appro	L2 T1 ach	All MCs All MCs	46 486 533	2.3 7.1 6.7	46 486 533	2.3 7.1 6.7	0.045 * 0.448 0.448	12.1 9.4 9.6	LOS A LOS A LOS A	0.9 10.5 10.5	6.3 75.0 75.0	0.47 0.55 0.55	0.60 0.49 0.50	0.47 0.55 0.55	11.0 13.2 13.0
North:	Cast	lereagh S	St (N)												
7	L2	All MCs	126	1.7	126	1.7	0.249	30.8	LOS C	4.3	30.8	0.82	0.74	0.82	18.4
8	T1	All MCs	88	3.6	88	3.6	*0.560	56.6	LOS E	6.9	49.7	0.96	0.79	0.96	15.2
9	R2	All MCs	79	5.3	79	5.3	0.560	64.6	LOS E	6.9	49.7	0.96	0.79	0.96	15.2
Appro	ach		294	3.2	294	3.2	0.560	47.7	LOS D	6.9	49.7	0.90	0.77	0.90	13.8
West:	Park	St (W)													
11	T1	All MCs	80	36.8	80	36.8	0.090	9.4	LOS A	1.4	10.2	0.50	0.42	0.50	20.8
12	R2	All MCs	18	5.9	18	5.9	*0.090	13.3	LOS A	1.4	10.2	0.51	0.45	0.51	17.0
Appro	ach		98	31.2	98	31.2	0.090	10.1	LOS A	1.4	10.2	0.50	0.43	0.50	20.1
All Ve	hicles		924	8.2	924	8.2	0.560	21.8	LOS B	10.5	75.0	0.65	0.58	0.65	14.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	rement	Perform	nance							
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		nod/h			[Ped	Dist]		Rate			m/200
Car	the Castlerage		sec	_	peu	m	_	_	sec	111	m/sec
500	ith: Castlereag	n 51 (5)									
P1	Full	258	38.7	LOS D	0.6	0.6	0.93	0.93	55.4	20.0	0.36
Eas	t: Park St (E)										
P2	Full	327	38.8	LOS D	0.8	0.8	0.93	0.93	55.5	20.0	0.36
Nor	th: Castlereag	h St (N)									
P3	Full	961	39.9	LOS D	2.4	2.4	0.96	0.96	56.5	20.0	0.35
We	st: Park St (W)										
P4	Full	81	38.4	LOS D	0.2	0.2	0.93	0.93	55.1	20.0	0.36

All Pedestrians	1627	39.4	LOS D	2.4	2.4	0.95	0.95	56.1	20.0	0.36
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Site: PIT04 [PIT04 Park St / Pitt St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: PIT-N1 [PIT Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 235

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 90 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Derr Fl	hand lows มหาวา	Ar Fl	rival ows HV/ 1	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop Rate	Aver. No. of	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		Trate	Cycles	km/h
South	: Pitt \$	St (S)													
1	L2	All MCs	85	1.2	85	1.2	0.262	21.9	LOS B	1.9	13.5	0.90	0.74	0.90	18.1
2	T1	All MCs	126	2.5	126	2.5	0.213	12.9	LOS A	2.4	17.1	0.77	0.62	0.77	28.9
3	R2	All MCs	40	0.0	40	0.0	0.121	21.3	LOS B	0.9	6.0	0.87	0.70	0.87	14.9
Appro	ach		252	1.7	252	1.7	0.262	17.3	LOS B	2.4	17.1	0.83	0.67	0.83	23.6
East:	Park \$	St (E)													
5	T1	All MCs	505	7.3	505	7.3	0.732	12.3	LOS A	10.7	76.8	0.88	0.78	0.89	20.8
6	R2	All MCs	60	3.5	60	3.5	*0.732	17.0	LOS B	10.7	76.8	0.90	0.80	0.91	26.5
Appro	ach		565	6.9	565	6.9	0.732	12.8	LOS A	10.7	76.8	0.88	0.78	0.89	21.7
West:	Park	St (W)													
10	L2	All MCs	1	100. 0	1	100. 0	0.111	15.9	LOS B	0.8	8.4	0.63	0.50	0.63	29.2
11	T1	All MCs	58	52.7	58	52.7	0.111	8.7	LOS A	0.8	8.4	0.63	0.50	0.63	18.2
Appro	ach		59	53.6	59	53.6	0.111	8.9	LOS A	0.8	8.4	0.63	0.50	0.63	18.7
All Ve	hicles		876	8.5	876	8.5	0.732	13.8	LOS A	10.7	76.8	0.85	0.73	0.86	22.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mov	/ement	Perform	nance							
Mo	/ Crossing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
שו	orecomig	FIOW	Delay	Service	[Ped	Dist]	Que	Rate	nne	DISI.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Soι	th: Pitt St (S)										
P1	Full	865	16.8	LOS B	1.1	1.1	0.88	0.88	33.5	20.0	0.60
Eas	st: Park St (E)										
P2	Full	582	16.6	LOS B	0.7	0.7	0.87	0.87	33.3	20.0	0.60
Nor	th: Pitt St (N)										
Р3	Full	1477	19.1	LOS B	2.0	2.0	0.95	0.95	35.8	20.0	0.56
We	st: Park St (W)									
P4	Full	1898	17.6	LOS B	2.5	2.5	0.92	0.92	34.3	20.0	0.58

All Pedestrians	4822	17.8	LOS B	2.5	2.5	0.91	0.91	34.5	20.0	0.58
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CCG MOVEMENT SUMMARY

□□ Common Control Group: CCG1 [CEN-N1] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: CEN-N1 [CEN Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (CCG User-Given Phase Times)

Vehic	le M	ovement	t Perfo	orma	nce (C	CCG)									
Mov ID	Turn	Mov Class	Dem Fl [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
Site: 0	CEN0	1 [CEN01	Elizab	eth S	st / Edd	y Ave]	360	_	Ven		_	_	_	N111/11
South	: Eliza	abeth St (S)				-								_
1a	L1	All MCs	245	6.0	245	6.0	0.229	5.5	LOS A	1.0	7.8	0.10	0.36	0.10	30.2
2	T1	All MCs	1244	5.3	1244	5.3	*0.728	13.6	LOS A	8.0	57.1	0.69	0.70	0.69	21.5
Appro	ach		1489	5.4	1489	5.4	0.728	12.3	LOS A	8.0	57.1	0.59	0.64	0.59	20.7
North:	Eliza	beth St (N	۷)												
8	T1	All MCs	485	6.1	485	6.1	* 0.733	29.6	LOS C	21.5	155.1	0.90	0.80	0.90	10.9
9b	R3	All MCs	223	15.6	223	15.6	0.485	50.8	LOS D	5.6	44.8	0.96	0.79	0.96	9.9
Appro	ach		708	9.1	708	9.1	0.733	36.3	LOS C	21.5	155.1	0.92	0.80	0.92	10.5
North	Nest:	Eddy Ave	e (NW)												
27b	L3	All MCs	599	7.2	599	7.2	*0.749	24.0	LOS B	10.5	78.1	0.89	0.83	0.92	16.6
29a	R1	All MCs	137	15.4	137	15.4	*0.832	56.2	LOS D	7.8	61.6	1.00	0.92	1.16	4.0
Appro	ach		736	8.7	736	8.7	0.832	30.0	LOS C	10.5	78.1	0.91	0.84	0.97	13.4
All Ve	hicles		2934	7.1	2934	7.1	0.832	22.5	LOS B	21.5	155.1	0.75	0.73	0.77	15.1
Site: 0	EN0	2 [CEN02	Elizab	eth S	st / Fov	eaux	St]								
South	: Eliza	abeth St (S)												
2	T1	All MCs	953	6.5	953	6.5	0.636	27.9	LOS B	20.4	150.5	0.86	0.76	0.86	13.1
Appro	ach		953	6.5	953	6.5	0.636	27.9	LOS B	20.4	150.5	0.86	0.76	0.86	13.1
South	East:	Foveaux	St (SE))											
21b	L3	All MCs	218	7.7	218	7.7	0.370	29.8	LOS C	8.3	62.2	0.76	0.76	0.76	18.1
23a	R1	All MCs	537	3.5	537	3.5	0.628	26.9	LOS B	11.1	80.2	0.81	0.78	0.81	12.3
Appro	ach		755	4.7	755	4.7	0.628	27.8	LOS B	11.1	80.2	0.79	0.78	0.79	14.4
North:	Eliza	beth St (N	۷)												
8	T1	All MCs	622	8.1	622	8.1	0.385	10.4	LOS A	7.9	57.1	0.36	0.30	0.36	24.7
Appro	ach		622	8.1	622	8.1	0.385	10.4	LOS A	7.9	57.1	0.36	0.30	0.36	24.7
All Ve	hicles		2329	6.4	2329	6.4	0.636	23.2	LOS B	20.4	150.5	0.70	0.64	0.70	15.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

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V Site: CEN03 [CEN03 Elizabeth St / Cooper St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehio	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar F	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Bac	k Of Queu	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	East:	Cooper S	St (SE)												
21b	L3	All MCs	78	4.1	78	4.1	0.091	6.9	LOS A	0.4	2.6	0.54	0.70	0.54	33.2
Appro	ach		78	4.1	78	4.1	0.091	6.9	LOS A	0.4	2.6	0.54	0.70	0.54	33.2
North	: Eliza	ibeth St (N)												
7a	L1	All MCs	78	4.1	78	4.1	0.224	3.6	LOS A	0.7	5.3	0.22	0.23	0.22	36.7
8	T1	All MCs	747	9.4	747	9.4	0.224	0.2	LOS A	0.7	5.3	0.08	0.08	0.08	38.4
Appro	ach		825	8.9	825	8.9	0.224	0.5	NA	0.7	5.3	0.09	0.10	0.09	38.1
All Ve	hicles	;	903	8.5	903	8.5	0.224	1.1	NA	0.7	5.3	0.13	0.15	0.13	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CEN05 [CEN05 Elizabeth St / Randle St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: CEN-N2 [CEN Network 2 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 2916

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site User-Given Phase Times)

Vehio	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar F	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
North	Eliza	beth St (I	N)												
8	T1	All MCs	757	8.8	757	8.8	0.258	2.4	LOS A	4.6	34.4	0.25	0.22	0.25	33.9
Appro	ach		757	8.8	757	8.8	0.258	2.4	LOS A	4.6	34.4	0.25	0.22	0.25	33.9
South	West:	Randle S	St (SW))											
30a	L1	All MCs	958	6.5	958	6.5	*0.404	7.3	LOS A	7.0	51.8	0.24	0.54	0.24	29.5
32b	R3	All MCs	68	10.8	68	10.8	0.404	4.2	LOS A	0.0	0.0	0.00	0.43	0.00	31.9
Appro	ach		1026	6.8	1026	6.8	0.404	7.1	LOS A	7.0	51.8	0.22	0.53	0.22	29.5
All Ve	hicles		1783	7.6	1783	7.6	0.404	5.1	LOS A	7.0	51.8	0.23	0.40	0.23	30.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Elizabeth	St (S)									
P1 Full	217	48.7	LOS E	0.6	0.6	0.94	0.94	215.3	200.0	0.93
SouthWest: Rand	lle St (S\	N)								
P8 Full	248	22.4	LOS C	0.5	0.5	0.85	0.85	189.1	200.0	1.06
All Pedestrians	465	34.6	LOS D	0.6	0.6	0.89	0.89	201.3	200.0	0.99

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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CCG MOVEMENT SUMMARY

□□ Common Control Group: CCG1 [CCGName] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: CEN-N1 [CEN Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (CCG User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce (0	CCG)									
Mov ID	Turn	Mov Class	Dem F	nand lows H\/ 1	Ar Fl [Total	rival lows HV/ 1	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop Rate	Aver. No. of	Aver. Speed
			veh/h	%	veh/h	<u>%</u>	v/c	sec		veh	m		110		km/h
Site: 0	CEN0	1 [CEN01	Elizab	eth S	st / Edd	y Ave]								
South	: Eliza	abeth St (S)												
1a	L1	All MCs	433	2.4	433	2.4	0.409	4.3	LOS A	5.4	41.1	0.29	0.49	0.29	25.6
2	T1	All MCs	964	4.1	964	4.1	*0.618	13.1	LOS A	8.1	57.1	0.69	0.67	0.69	20.9
Appro	ach		1397	3.6	1397	3.6	0.618	10.4	LOS A	8.1	57.1	0.57	0.61	0.57	21.8
North:	Eliza	beth St (I	N)												
8	T1	All MCs	546	4.0	546	4.0	*0.822	39.7	LOS C	28.0	198.2	0.98	0.92	1.05	8.7
9b	R3	All MCs	504	6.7	504	6.7	*0.864	63.7	LOS E	15.2	112.5	1.00	1.03	1.27	8.7
Appro	ach		1051	5.3	1051	5.3	0.864	51.2	LOS D	28.0	198.2	0.99	0.97	1.16	8.5
North	Nest:	Eddy Ave	e (NW)												
27b	L3	All MCs	633	5.0	633	5.0	0.624	16.9	LOS B	8.3	60.6	0.73	0.75	0.73	20.0
29a	R1	All MCs	136	14.0	136	14.0	*0.627	49.4	LOS D	7.0	54.6	0.96	0.79	0.98	4.5
Appro	ach		768	6.6	768	6.6	0.627	22.6	LOS B	8.3	60.6	0.77	0.76	0.78	16.1
All Ve	hicles		3216	4.9	3216	4.9	0.864	26.6	LOS B	28.0	198.2	0.75	0.76	0.81	13.5
Site: 0	CEN0	2 [CEN02	l Elizab	eth S	st / Fov	eaux	St]								
South	: Eliza	abeth St (S)												
2	T1	All MCs	694	4.9	694	4.9	0.554	32.1	LOS C	15.2	110.8	0.87	0.75	0.87	11.8
Appro	ach		694	4.9	694	4.9	0.554	32.1	LOS C	15.2	110.8	0.87	0.75	0.87	11.8
South	East:	Foveaux	St (SE)											
21b	L3	All MCs	176	2.4	176	2.4	0.239	23.1	LOS B	5.6	40.3	0.64	0.72	0.64	20.5
23a	R1	All MCs	703	2.4	703	2.4	0.890	50.5	LOS D	22.5	160.7	0.95	1.07	1.25	7.6
Appro	ach		879	2.4	879	2.4	0.890	45.0	LOS D	22.5	160.7	0.89	1.00	1.13	9.8
North	Eliza	beth St (I	N)												
8	T1	All MCs	682	6.0	682	6.0	0.500	10.8	LOS A	8.1	57.1	0.37	0.32	0.37	24.4
Appro	ach		682	6.0	682	6.0	0.500	10.8	LOS A	8.1	57.1	0.37	0.32	0.37	24.4
All Ve	hicles		2255	4.2	2255	4.2	0.890	30.7	LOS C	22.5	160.7	0.73	0.72	0.82	13.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

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V Site: CEN03 [CEN03 Elizabeth St / Cooper St (Site Folder: Block 4 Model - 2024 PM Peak)] **Output produced by SIDRA INTERSECTION Version: 9.1.6.228**

Stop

Rate

Que

Dist]

Aver

Speed

km/h

Aver.

Cycles

NA Site Category: (None) Give-Way (Two-Way)

Vehicle Movement Performance Deg. Mov Turn Mov Level of 95% Back Of Queue Prop. Demand Arrival ID Class Flows Satn Delay Service Flows [Total HV] [Total HV] [Veh. veh/h SouthEast: Cooper St (SE)

		•	· · /												
21b	L3	All MCs	93	2.3	93	2.3	0.135	8.4	LOS A	0.5	3.7	0.61	0.80	0.61	32.2
Appro	ach		93	2.3	93	2.3	0.135	8.4	LOS A	0.5	3.7	0.61	0.80	0.61	32.2
North:	Eliza	ibeth St (N	1)												
7a	L1	All MCs	57	0.0	57	0.0	0.298	3.5	LOS A	0.6	4.0	0.12	0.12	0.12	37.5
8	T1	All MCs	1068	4.7	1068	4.7	0.298	0.1	LOS A	0.6	4.0	0.05	0.05	0.05	39.0
Appro	ach		1125	4.5	1125	4.5	0.298	0.3	NA	0.6	4.0	0.05	0.06	0.05	38.8
All Ve	hicles		1218	43	1218	43	0 298	0.9	NA	0.6	40	0 10	0 11	0 10	37.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CEN05 [CEN05 Elizabeth St / Randle St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 2916

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site User-Given Phase Times)

Vehio	cle M	ovement	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand lows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
North	: Eliza	beth St (N	۷)												
8	T1	All MCs	1044	4.6	1044	4.6	0.346	2.9	LOS A	7.3	52.9	0.29	0.26	0.29	32.9
Appro	ach		1044	4.6	1044	4.6	0.346	2.9	LOS A	7.3	52.9	0.29	0.26	0.29	32.9
South	West:	Randle S	St (SW)												
30a	L1	All MCs	721	5.7	721	5.7	* 0.313	6.4	LOS A	4.8	35.5	0.23	0.53	0.23	30.3
32b	R3	All MCs	80	2.6	80	2.6	0.313	4.2	LOS A	0.0	0.0	0.00	0.44	0.00	31.7
Appro	ach		801	5.4	801	5.4	0.313	6.2	LOS A	4.8	35.5	0.21	0.52	0.21	30.4
All Ve	hicles		1845	5.0	1845	5.0	0.346	4.3	LOS A	7.3	52.9	0.25	0.37	0.25	31.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Elizabeth	St (S)									
P1 Full	348	48.9	LOS E	1.0	1.0	0.95	0.95	215.6	200.0	0.93
SouthWest: Rand	lle St (S\	N)								
P8 Full	366	22.9	LOS C	0.7	0.7	0.85	0.85	189.5	200.0	1.06
All Pedestrians	715	35.6	LOS D	1.0	1.0	0.90	0.90	202.2	200.0	0.99

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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CCG MOVEMENT SUMMARY

□□ Common Control Group: CCG1 [CCGName] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: CEN-N1 [CEN Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 105 seconds (CCG User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce (C	CCG)									
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total]	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
Site: (veh/h	% oth S	veh/h	% v Ave	v/c	sec	-	veh	m	-	-	-	km/h
South		beth St (S)	euro		улче	.1								
19	. בוובנ 1		201	16	201	16	0 405	3.1		62	11 1	0.46	0.49	0.46	24.3
2	T1	All MCs	965	3.7	965	3.7	* 0.405	6.9	LOSA	6.2	44.4	0.51	0.46	0.51	27.5
Appro	ach		1166	3.3	1166	3.3	0.405	6.2	LOSA	6.2	44.4	0.50	0.46	0.50	27.1
North:	Eliza	beth St (I	(N												
8	T1	All MCs	425	5.7	425	5.7	0.288	22.8	LOS B	7.3	53.9	0.72	0.61	0.72	13.0
9b	R3	All MCs	224	7.5	224	7.5	0.458	48.5	LOS D	5.4	40.1	0.95	0.78	0.95	10.3
Appro	ach		649	6.3	649	6.3	0.458	31.7	LOS C	7.3	53.9	0.80	0.67	0.80	11.6
North	Nest:	Eddy Ave	e (NW)												
27b	L3	All MCs	462	3.6	462	3.6	*0.573	21.1	LOS B	6.7	48.4	0.80	0.76	0.80	17.8
29a	R1	All MCs	106	5.0	106	5.0	*0.615	50.4	LOS D	5.4	39.2	0.98	0.79	1.00	4.5
Appro	ach		568	3.9	568	3.9	0.615	26.6	LOS B	6.7	48.4	0.83	0.76	0.84	14.5
All Ve	hicles		2384	4.3	2384	4.3	0.615	18.0	LOS B	7.3	53.9	0.66	0.59	0.66	17.2
Site: 0	CEN0	2 [CEN02	Elizab	eth S	st / Fove	eaux	St]								
South	: Eliza	abeth St (S)												
2	T1	All MCs	749	3.7	749	3.7	*0.498	25.3	LOS B	14.3	103.5	0.80	0.70	0.80	13.9
Appro	ach		749	3.7	749	3.7	0.498	25.3	LOS B	14.3	103.5	0.80	0.70	0.80	13.9
South	East:	Foveaux	St (SE)											
21b	L3	All MCs	143	2.9	143	2.9	0.232	27.2	LOS B	4.9	35.2	0.71	0.73	0.71	19.0
23a	R1	All MCs	417	2.8	417	2.8	0.310	21.1	LOS B	7.0	50.2	0.66	0.69	0.66	14.4
Appro	ach		560	2.8	560	2.8	0.310	22.7	LOS B	7.0	50.2	0.68	0.70	0.68	16.1
North:	Eliza	beth St (I	V)												
8	T1	All MCs	532	5.5	532	5.5	0.307	9.2	LOS A	6.6	47.3	0.32	0.27	0.32	25.9
Appro	ach		532	5.5	532	5.5	0.307	9.2	LOS A	6.6	47.3	0.32	0.27	0.32	25.9
All Ve	hicles		1841	3.9	1841	3.9	0.498	19.9	LOS B	14.3	103.5	0.62	0.57	0.62	17.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: CEN03 [CEN03 Elizabeth St / Cooper St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar F	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Bac	k Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	East:	Cooper S	St (SE)												
21b	L3	All MCs	71	0.0	71	0.0	0.068	5.9	LOS A	0.3	1.9	0.47	0.62	0.47	33.9
Appro	bach		71	0.0	71	0.0	0.068	5.9	LOS A	0.3	1.9	0.47	0.62	0.47	33.9
North	: Eliza	beth St (l	N)												
7a	L1	All MCs	41	2.6	41	2.6	0.180	2.8	LOS A	0.3	2.5	0.10	0.11	0.10	37.6
8	T1	All MCs	668	5.7	668	5.7	0.180	0.1	LOS A	0.3	2.5	0.04	0.04	0.04	39.2
Appro	bach		709	5.5	709	5.5	0.180	0.2	NA	0.3	2.5	0.04	0.05	0.04	39.0
All Ve	hicles		780	5.0	780	5.0	0.180	0.7	NA	0.3	2.5	0.08	0.10	0.08	37.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: CEN05 [CEN05 Elizabeth St / Randle St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: CEN-N2 [CEN Network 2 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 2916

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 105 seconds (Site User-Given Phase Times)

Vehio	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Derr Fl	nand lows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
North	: Eliza	beth St (N)												
8	T1	All MCs	665	5.5	665	5.5	0.220	2.2	LOS A	3.7	26.9	0.24	0.21	0.24	34.4
Appro	ach		665	5.5	665	5.5	0.220	2.2	LOS A	3.7	26.9	0.24	0.21	0.24	34.4
South	West:	Randle	St (SW)												
30a	L1	All MCs	744	5.0	744	5.0	*0.310	6.2	LOS A	4.5	32.7	0.22	0.51	0.22	30.6
32b	R3	All MCs	44	4.8	44	4.8	0.310	4.2	LOS A	0.0	0.0	0.00	0.43	0.00	31.9
Appro	ach		788	4.9	788	4.9	0.310	6.1	LOS A	4.5	32.7	0.21	0.51	0.21	30.6
All Ve	hicles		1454	5.2	1454	5.2	0.310	4.3	LOS A	4.5	32.7	0.22	0.37	0.22	31.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Elizabeth	St (S)									
P1 Full	93	45.9	LOS E	0.3	0.3	0.94	0.94	212.6	200.0	0.94
SouthWest: Rand	lle St (S\	N)								
P8 Full	85	20.4	LOS C	0.1	0.1	0.83	0.83	187.0	200.0	1.07
All Pedestrians	178	33.7	LOS D	0.3	0.3	0.89	0.89	200.3	200.0	1.00

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: WLO01 [WLO01 Botany Rd / Raglan St / Henderson Rd (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 47

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Den F [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	. Doto	my Dd (S	veh/h	%	veh/h	%	V/C	sec	_	veh	m			_	km/h
South	. DOla)												
1	L2	All MCs	828	6.9	828	6.9	*0.689	30.9	LOS C	18.0	133.5	0.82	0.81	0.82	17.8
Appro	ach		828	6.9	828	6.9	0.689	30.9	LOS C	18.0	133.5	0.82	0.81	0.82	17.8
East:	Ragla	n St (E)													
4	L2	All MCs	24	8.7	24	8.7	0.432	58.6	LOS E	5.3	39.2	0.97	0.77	0.97	4.8
5	T1	All MCs	183	7.5	183	7.5	0.432	50.2	LOS D	5.8	42.9	0.96	0.76	0.96	4.9
Appro	ach		207	7.6	207	7.6	0.432	51.2	LOS D	5.8	42.9	0.96	0.76	0.96	4.9
North:	Bota	ny Rd (N))												
7	L2	All MCs	48	17.4	48	17.4	0.320	11.4	LOS A	7.7	58.8	0.37	0.37	0.37	35.6
8	T1	All MCs	783	9.5	783	9.5	0.320	5.3	LOS A	7.9	59.8	0.37	0.35	0.37	36.1
9	R2	All MCs	480	5.0	480	5.0	*0.681	51.8	LOS D	12.8	93.7	0.98	0.84	1.00	10.8
Appro	ach		1312	8.2	1312	8.2	0.681	22.6	LOS B	12.8	93.7	0.59	0.53	0.60	19.5
West:	Hend	lerson Rd	(W)												
11	T1	All MCs	220	2.4	220	2.4	*0.622	13.0	LOS A	3.8	28.0	0.44	0.38	0.45	15.0
12	R2	All MCs	45	11.6	45	11.6	0.622	30.5	LOS C	3.8	28.0	0.67	0.61	0.68	9.2
Appro	ach		265	4.0	265	4.0	0.622	16.0	LOS B	3.8	28.0	0.48	0.42	0.49	13.5
All Ve	hicles		2613	7.3	2613	7.3	0.689	26.8	LOS B	18.0	133.5	0.68	0.63	0.69	16.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pede	estrian Mov	/ement	Perforr	nance							
Mov ID	Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
		ped/h	sec		ped	m		rtato	sec	m	m/sec
South	n: Botany Rd	(S)									
P1	Full	454	51.7	LOS E	1.4	1.4	0.96	0.96	68.4	20.0	0.29
East:	Raglan St (I	E)									
P2	Full	99	50.9	LOS E	0.3	0.3	0.94	0.94	67.6	20.0	0.30

North: Botany Rd	(N)										
P3 Full	154	51.0	LOS E	0.5	0.5	0.95	0.95	67.7	20.0	0.30	
West: Henderson Rd (W)											
P4 Full	55	50.8	LOS E	0.2	0.2	0.94	0.94	67.5	20.0	0.30	
All Pedestrians	761	51.4	LOS E	1.4	1.4	0.95	0.95	68.1	20.0	0.29	

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Site: WLO02 [WLO02 Raglan St / Cope St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 5057

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 35 seconds (Site User-Given Phase Times)

Vehio	cle M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	hand	Ar	rival	Deg.	Aver.	Level of	95% Back	COf Queue	Prop.	Eff.	Aver.	Aver.
שו		Class	٦ Total آ	HV 1	اح Total آ	HV 1	Sam	Delay	Service	[Veh.	Dist 1	Que	Rate	Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m			- 5	km/h
South	: Cop	e St (S)													
1	L2	All MCs	36	2.9	36	2.9	0.141	15.7	LOS B	0.8	6.0	0.80	0.68	0.80	16.3
2	T1	All MCs	17	0.0	17	0.0	0.141	10.4	LOS A	0.8	6.0	0.80	0.68	0.80	30.9
3	R2	All MCs	7	0.0	7	0.0	0.141	13.8	LOS A	0.8	6.0	0.80	0.68	0.80	30.8
Appro	ach		60	1.8	60	1.8	0.141	14.0	LOS A	0.8	6.0	0.80	0.68	0.80	24.4
East:	Ragla	n St (E)													
4	L2	All MCs	12	0.0	12	0.0	0.185	15.8	LOS B	1.4	10.3	0.74	0.60	0.74	34.8
5	T1	All MCs	192	7.7	192	7.7	0.185	8.9	LOS A	1.4	10.3	0.74	0.60	0.74	34.8
6	R2	All MCs	13	8.3	13	8.3	0.185	15.2	LOS B	1.3	9.8	0.74	0.60	0.74	38.9
Appro	ach		216	7.3	216	7.3	0.185	9.6	LOS A	1.4	10.3	0.74	0.60	0.74	35.2
North	Cope	e St (N)													
7	L2	All MCs	12	9.1	12	9.1	0.042	14.4	LOS A	0.3	2.1	0.72	0.62	0.72	36.9
8	T1	All MCs	11	10.0	11	10.0	0.042	10.2	LOS A	0.3	2.1	0.72	0.62	0.72	30.7
9	R2	All MCs	42	2.5	42	2.5	*0.302	23.2	LOS B	0.8	5.6	0.97	0.72	0.97	23.0
Appro	ach		64	4.9	64	4.9	0.302	19.5	LOS B	0.8	5.6	0.88	0.69	0.88	27.1
West:	Ragla	an St (W))												
10	L2	All MCs	54	5.9	54	5.9	0.108	14.7	LOS B	0.7	5.2	0.77	0.70	0.77	29.4
11	T1	All MCs	212	5.5	212	5.5	*0.334	9.6	LOS A	2.8	20.6	0.78	0.64	0.78	36.6
Appro	ach		265	5.6	265	5.6	0.334	10.7	LOS A	2.8	20.6	0.78	0.65	0.78	35.0
All Ve	hicles		605	5.7	605	5.7	0.334	11.5	LOS A	2.8	20.6	0.78	0.64	0.78	33.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perforr	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Cope St (S	S)									
P1	Full	198	12.1	LOS B	0.2	0.2	0.84	0.84	178.8	200.0	1.12

East: Raglan St (E)									
P2 Full	41	12.0	LOS B	0.0	0.0	0.83	0.83	28.7	20.0	0.70
North: Cope St (N)										
P3 Full	86	12.1	LOS B	0.1	0.1	0.83	0.83	28.7	20.0	0.70
West: Raglan St (V	V)									
P4 Full	103	12.1	LOS B	0.1	0.1	0.83	0.83	28.7	20.0	0.70
All Pedestrians	428	12.1	LOS B	0.2	0.2	0.83	0.83	98.0	103.1	1.05

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Site: WLO03 [WLO03 Botany Rd / Wellington St / Buckland St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 137

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	FI Total	IOWS HV/1	Fl [Total	lows HV/1	Satn	Delay	Service	[Veh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		rtato	Cycles	km/h
South	: Bota	ny Rd (S)												
2	T1	All MCs	797	7.1	797	7.1	0.361	4.7	LOS A	8.9	66.4	0.36	0.34	0.36	40.6
3	R2	All MCs	56	9.4	56	9.4	*0.361	12.4	LOS A	6.7	49.9	0.37	0.39	0.37	39.3
Appro	ach		853	7.3	853	7.3	0.361	5.2	LOS A	8.9	66.4	0.36	0.34	0.36	40.5
East:	Wellir	ngton St (E)												
4	L2	All MCs	35	3.0	35	3.0	0.222	59.3	LOS E	1.9	13.6	0.97	0.73	0.97	15.3
6	R2	All MCs	11	0.0	11	0.0	0.069	57.1	LOS E	0.6	3.9	0.94	0.68	0.94	4.5
Appro	Approach 45 2			2.3	45	2.3	0.222	58.8	LOS E	1.9	13.6	0.96	0.72	0.96	13.4
North:	Bota	ny Rd (N)												
7	L2	All MCs	18	29.4	18	29.4	0.314	8.7	LOS A	6.2	46.9	0.29	0.27	0.29	40.9
8	T1	All MCs	835	9.2	835	9.2	0.314	3.6	LOS A	6.3	47.6	0.29	0.26	0.29	45.6
Appro	ach		853	9.6	853	9.6	0.314	3.7	LOS A	6.3	47.6	0.29	0.26	0.29	45.5
West:	Buck	land St (\	V)												
10	L2	All MCs	21	0.0	21	0.0	0.340	54.4	LOS D	4.4	30.8	0.95	0.75	0.95	5.3
11	T1	All MCs	62	1.7	62	1.7	*0.340	50.0	LOS D	4.4	30.8	0.95	0.75	0.95	5.3
12	R2	All MCs	26	4.0	26	4.0	0.173	58.0	LOS E	1.4	10.3	0.96	0.71	0.96	15.3
Appro	ach		109	1.9	109	1.9	0.340	52.8	LOS D	4.4	30.8	0.95	0.74	0.95	8.5
All Ve	hicles		1860	7.9	1860	7.9	0.361	8.6	LOS A	8.9	66.4	0.38	0.34	0.38	38.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

 ${\rm HV}$ (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perforr	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	ith: Botany Ro	l (S)									
P1	Full	184	51.1	LOS E	0.6	0.6	0.95	0.95	67.8	20.0	0.30
Eas	t: Wellington	St (E)									

P2 Full	23	50.8	LOS E	0.1	0.1	0.94	0.94	67.4	20.0	0.30
North: Botany Rd (N)									
P3 Full	29	50.8	LOS E	0.1	0.1	0.94	0.94	67.4	20.0	0.30
West: Buckland St	(W)									
P4 Full	60	50.8	LOS E	0.2	0.2	0.94	0.94	67.5	20.0	0.30
All Pedestrians	297	51.0	LOS E	0.6	0.6	0.94	0.94	67.7	20.0	0.30

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Site: WLO04 [WLO04 Cope St / Wellington St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

NA Site Category: (None) Stop (Two-Way)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Total	lows HV 1	H Total آ	lows HV 1	Satn	Delay	Service	[Veh	Dist 1	Que	Stop Rate	No. of Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cop	e St (S)													
1	L2	All MCs	17	6.3	17	6.3	0.038	7.8	LOS A	0.1	1.0	0.15	0.93	0.15	29.8
2	T1	All MCs	12	0.0	12	0.0	0.038	8.3	LOS A	0.1	1.0	0.15	0.93	0.15	29.8
3	R2	All MCs	11	0.0	11	0.0	0.038	8.5	LOS A	0.1	1.0	0.15	0.93	0.15	33.4
Appro	ach		39	2.7	39	2.7	0.038	8.1	LOS A	0.1	1.0	0.15	0.93	0.15	31.1
East:	Wellir	igton St (I	E)												
4	L2	All MCs	12	9.1	12	9.1	0.043	5.1	LOS A	0.2	1.4	0.29	0.34	0.29	38.5
5	T1	All MCs	24	0.0	24	0.0	0.043	0.8	LOS A	0.2	1.4	0.29	0.34	0.29	37.0
6	R2	All MCs	13	8.3	13	8.3	0.043	5.8	LOS A	0.2	1.4	0.29	0.34	0.29	37.0
Appro	Approach		48	4.3	48	4.3	0.043	3.1	NA	0.2	1.4	0.29	0.34	0.29	37.6
North:	Cope	e St (N)													
7	L2	All MCs	2	0.0	2	0.0	0.011	7.8	LOS A	0.0	0.3	0.33	0.88	0.33	30.5
8	T1	All MCs	5	0.0	5	0.0	0.011	7.6	LOS A	0.0	0.3	0.33	0.88	0.33	31.7
9	R2	All MCs	3	0.0	3	0.0	0.011	7.7	LOS A	0.0	0.3	0.33	0.88	0.33	24.1
Appro	ach		11	0.0	11	0.0	0.011	7.6	LOS A	0.0	0.3	0.33	0.88	0.33	29.9
West:	Welli	ngton St ((W)												
10	L2	All MCs	22	0.0	22	0.0	0.074	4.8	LOS A	0.2	1.5	0.08	0.20	0.08	36.6
11	T1	All MCs	81	7.8	81	7.8	0.074	0.1	LOS A	0.2	1.5	0.08	0.20	0.08	43.8
12	R2	All MCs	22	0.0	22	0.0	0.074	4.7	LOS A	0.2	1.5	0.08	0.20	0.08	39.6
Appro	ach		125	5.0	125	5.0	0.074	1.7	NA	0.2	1.5	0.08	0.20	0.08	42.2
All Ve	hicles		223	4.2	223	4.2	0.074	3.4	NA	0.2	1.5	0.15	0.39	0.15	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: WLO05 [WLO05 Wyndham St / Henderson Rd (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 55

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Wyn	dham St	(S)												
1	L2	All MCs	12	9.1	12	9.1	0.633	61.4	LOS E	9.8	73.6	0.97	0.81	0.98	12.3
2	T1	All MCs	356	8.6	356	8.6	*0.633	52.7	LOS D	9.8	74.1	0.97	0.81	0.98	19.8
3	R2	All MCs	4	50.0	4	50.0	0.633	60.8	LOS E	9.8	74.1	0.97	0.80	0.98	13.2
Appro	ach		372	9.1	372	9.1	0.633	53.1	LOS D	9.8	74.1	0.97	0.81	0.98	18.1
East:	Hende	erson Rd	(E)												
4	L2	All MCs	113	2.8	113	2.8	0.289	6.4	LOS A	2.0	14.2	0.11	0.24	0.11	40.6
5	T1	All MCs	623	5.4	623	5.4	0.289	1.3	LOS A	2.0	14.2	0.10	0.15	0.10	41.7
6	R2	All MCs	756	7.7	756	7.7	*0.618	13.5	LOS A	6.9	51.5	0.63	0.74	0.63	27.8
Appro	ach		1492	6.4	1492	6.4	0.618	7.9	LOS A	6.9	51.5	0.37	0.46	0.37	31.6
West:	Hend	lerson Rd	(W)												
10	L2	All MCs	349	6.0	349	6.0	*0.742	74.0	LOS F	9.8	72.2	0.99	0.88	1.11	12.7
11	T1	All MCs	262	3.6	262	3.6	0.572	41.5	LOS C	13.0	94.1	0.93	0.79	0.93	5.5
Appro	ach		612	5.0	612	5.0	0.742	60.1	LOS E	13.0	94.1	0.97	0.84	1.04	8.8
All Ve	hicles		2475	6.4	2475	6.4	0.742	27.6	LOS B	13.0	94.1	0.61	0.60	0.63	18.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	destrian Mov	vement	Perform	nance									
Mo	′ <u> </u>	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.		
ID	Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed		
		ped/h	sec		ped	m			sec	m	m/sec		
South: Wyndham St (S)													
P1	Full	247	51.2	LOS E	0.8	0.8	0.95	0.95	67.9	20.0	0.29		
Eas	t: Henderson	Rd (E)											
P2	Full	127	51.0	LOS E	0.4	0.4	0.94	0.94	67.7	20.0	0.30		
Nor	th: Wyndham	St (N)											
P3	Full	194	51.1	LOS E	0.6	0.6	0.95	0.95	67.8	20.0	0.30		
Wes	st: Henderson	Rd (W)											

P4 Full	143	51.0	LOS E	0.4	0.4	0.94	0.94	67.7	20.0	0.30
All Pedestrians	712	51.1	LOS E	0.8	0.8	0.95	0.95	67.8	20.0	0.30

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Site: WLO06 [WLO06 Pedestrian Mid-block Crossing at Cope St (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Cop	e St (S)													
2	T1	All MCs	46	2.3	46	2.3	0.030	2.2	LOS A	0.1	0.8	0.14	0.36	0.14	32.0
Appro	ach		46	2.3	46	2.3	0.030	2.2	LOS A	0.1	0.8	0.14	0.36	0.14	32.0
North:	Cope	e St (N)													
8	T1	All MCs	11	0.0	11	0.0	0.007	2.2	LOS A	0.0	0.2	0.13	0.35	0.13	31.1
Appro	ach		11	0.0	11	0.0	0.007	2.2	LOS A	0.0	0.2	0.13	0.35	0.13	31.1
All Ve	hicles		57	1.9	57	1.9	0.030	2.2	NA	0.1	0.8	0.14	0.35	0.14	31.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: WLO01 [WLO01 Botany Rd / Raglan St / Henderson Rd (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 47

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar F	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Bota	ny Rd (S)												
1	L2	All MCs	655	5.5	655	5.5	*0.916	72.7	LOS F	23.2	169.9	1.00	1.04	1.27	9.5
Appro	ach		655	5.5	655	5.5	0.916	72.7	LOS F	23.2	169.9	1.00	1.04	1.27	9.5
East:	Ragla	n St (E)													
4	L2	All MCs	24	0.0	24	0.0	0.803	70.2	LOS E	9.3	67.0	1.00	0.96	1.21	4.1
5	T1	All MCs	267	3.9	267	3.9	0.803	62.0	LOS E	9.3	67.0	1.00	0.96	1.22	4.1
Appro	ach		292	3.6	292	3.6	0.803	62.7	LOS E	9.3	67.0	1.00	0.96	1.22	4.1
North:	Bota	ny Rd (N)												
7	L2	All MCs	49	10.6	49	10.6	0.358	11.3	LOS A	9.4	68.8	0.36	0.36	0.36	36.3
8	T1	All MCs	952	4.6	952	4.6	0.358	5.0	LOS A	9.6	69.8	0.36	0.34	0.36	36.7
9	R2	All MCs	634	1.3	634	1.3	*0.849	59.5	LOS E	20.5	145.3	1.00	0.96	1.18	9.6
Appro	ach		1635	3.5	1635	3.5	0.849	26.3	LOS B	20.5	145.3	0.61	0.58	0.68	17.7
West:	Henc	lerson Rd	(W)												
11	T1	All MCs	223	0.9	223	0.9	* 0.904	15.2	LOS B	9.6	68.1	0.74	0.68	0.86	13.9
12	R2	All MCs	57	0.0	57	0.0	0.904	57.1	LOS E	3.9	27.4	1.00	0.83	1.21	4.7
Appro	ach		280	0.8	280	0.8	0.904	23.7	LOS B	9.6	68.1	0.79	0.71	0.93	10.0
All Ve	hicles		2861	3.7	2861	3.7	0.916	40.4	LOS C	23.2	169.9	0.75	0.73	0.89	12.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mo	vement	Perform	nance							
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		ped/h	sec		ped	m		Naic	sec	m	m/sec
Sou	ith: Botany Ro	d (S)									
P1	Full	451	54.2	LOS E	1.5	1.5	0.96	0.96	70.9	20.0	0.28
Eas	t: Raglan St (E)									
P2	Full	157	53.6	LOS E	0.5	0.5	0.95	0.95	70.2	20.0	0.28

North: Botany Rd	(N)									
P3 Full	148	53.5	LOS E	0.5	0.5	0.95	0.95	70.2	20.0	0.28
West: Henderson	Rd (W)									
P4 Full	137	53.5	LOS E	0.4	0.4	0.95	0.95	70.2	20.0	0.28
All Pedestrians	893	53.9	LOS E	1.5	1.5	0.95	0.95	70.5	20.0	0.28

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Site: WLO02 [WLO02 Raglan St / Cope St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 5057

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 45 seconds (Site User-Given Phase Times)

Vehio	le M	ovemen	t Perfo	rma	nce									
Mov	Turn	Mov	Dem	and	Arriva	I Deg.	Aver.	Level of	95% Back	c Of Queue	e Prop.	Eff.	Aver.	Aver.
ID		Class	H Total I	OWS	Flow: \/H Hotal H	s Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h %	v/c	sec		veh	m		Rate	Cycles	km/h
South	: Cop	e St (S)												
1	L2	All MCs	22	0.0	22 0.0	0.114	19.7	LOS B	0.8	5.7	0.81	0.67	0.81	14.1
2	T1	All MCs	12	0.0	12 0.0	0.114	13.5	LOS A	0.8	5.7	0.81	0.67	0.81	28.5
3	R2	All MCs	12	0.0	12 0.0	0.114	17.0	LOS B	0.8	5.7	0.81	0.67	0.81	28.6
Appro	ach		45	0.0	45 0.0	0.114	17.4	LOS B	0.8	5.7	0.81	0.67	0.81	23.5
East:	Ragla	n St (E)												
4	L2	All MCs	13	0.0	13 0.0	0.218	17.2	LOS B	2.1	15.2	0.71	0.59	0.71	33.4
5	T1	All MCs	254	4.6	254 4.6	0.218	10.4	LOS A	2.1	15.2	0.71	0.58	0.71	33.6
6	R2	All MCs	33	33.3	3 33.3	0.218	17.7	LOS B	2.1	15.0	0.71	0.58	0.71	38.1
Appro	ach		269	4.7	269 4.1	0.218	10.8	LOS A	2.1	15.2	0.71	0.58	0.71	33.6
North	Cope	e St (N)												
7	L2	All MCs	11 ⁻	10.0	11 10.0	0.023	15.9	LOS B	0.2	1.4	0.69	0.63	0.69	35.1
8	T1	All MCs	2	0.0	2 0.0	0.023	11.7	LOS A	0.2	1.4	0.69	0.63	0.69	28.3
9	R2	All MCs	57	1.9	57 1.9	* 0.252	24.1	LOS B	1.2	8.5	0.91	0.74	0.91	22.4
Appro	ach		69	3.0	69 3.0	0.252	22.5	LOS B	1.2	8.5	0.87	0.72	0.87	25.0
West:	Ragla	an St (W)												
10	L2	All MCs	43	0.0	43 0.0	0.070	15.5	LOS B	0.7	4.6	0.71	0.68	0.71	28.9
11	T1	All MCs	189	4.4	189 4.4	* 0.269	10.7	LOS A	3.0	21.5	0.73	0.60	0.73	35.5
Appro	ach		233	3.6	233 3.0	0.269	11.6	LOS A	3.0	21.5	0.73	0.61	0.73	34.2
All Ve	hicles		617	3.8	617 3.8	0.269	12.9	LOS A	3.0	21.5	0.74	0.62	0.74	31.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perforr	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Cope St (S	S)									
P1	Full	124	17.0	LOS B	0.1	0.1	0.87	0.87	183.7	200.0	1.09

East: Raglan St (E)									
P2 Full	44	16.9	LOS B	0.0	0.0	0.87	0.87	33.6	20.0	0.60
North: Cope St (N)										
P3 Full	107	17.0	LOS B	0.1	0.1	0.87	0.87	33.6	20.0	0.59
West: Raglan St (V	V)									
P4 Full	156	17.0	LOS B	0.2	0.2	0.87	0.87	33.7	20.0	0.59
All Pedestrians	432	17.0	LOS B	0.2	0.2	0.87	0.87	76.8	71.8	0.93

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Site: WLO03 [WLO03 Botany Rd / Wellington St / Buckland St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 137

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Den F	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]	vic	800		[Veh.	Dist]		Rate	Cycles	km/h
South	: Bota	iny Rd (S)	70	VCH/H	70	V/C	300		VCIT		_	_		NIT/TT
2	T1	All MCs	616	5.8	616	5.8	0.363	5.2	LOS A	9.7	71.0	0.37	0.36	0.37	39.7
3	R2	All MCs	95	0.0	95	0.0	*0.363	12.6	LOS A	4.5	32.1	0.40	0.50	0.40	35.8
Appro	ach		711	5.0	711	5.0	0.363	6.1	LOS A	9.7	71.0	0.37	0.38	0.37	39.1
East:	Wellir	igton St (I	E)												
4	L2	All MCs	64	0.0	64	0.0	0.319	59.4	LOS E	3.6	25.1	0.96	0.76	0.96	15.3
6	R2	All MCs	20	0.0	20	0.0	0.120	58.2	LOS E	1.1	7.6	0.93	0.71	0.93	4.4
Appro	ach		84	0.0	84	0.0	0.319	59.1	LOS E	3.6	25.1	0.96	0.74	0.96	13.3
North	Bota	ny Rd (N))												
7	L2	All MCs	18	0.0	18	0.0	0.365	7.6	LOS A	5.8	41.7	0.21	0.20	0.21	43.2
8	T1	All MCs	1015	4.4	1015	4.4	0.365	2.4	LOS A	5.8	41.7	0.21	0.19	0.21	46.9
Appro	ach		1033	4.3	1033	4.3	0.365	2.5	LOS A	5.8	41.7	0.21	0.19	0.21	46.9
West:	Buck	land St (V	V)												
10	L2	All MCs	19	0.0	19	0.0	0.306	55.7	LOS D	4.2	30.3	0.94	0.74	0.94	5.2
11	T1	All MCs	60	3.5	60	3.5	*0.306	50.3	LOS D	4.2	30.3	0.94	0.74	0.94	5.2
12	R2	All MCs	19	0.0	19	0.0	0.107	56.9	LOS E	1.0	7.2	0.93	0.70	0.93	15.5
Appro	ach		98	2.2	98	2.2	0.306	52.6	LOS D	4.2	30.3	0.94	0.73	0.94	7.9
All Ve	hicles		1925	4.3	1925	4.3	0.365	8.9	LOS A	9.7	71.0	0.34	0.31	0.34	38.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

 ${\rm HV}$ (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perform	nance								
Mo		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.	
ID Crossing Flow Delay Service QUEUE Que Stop Time Dist. S												
					[Ped	Dist]		Rate				
		ped/h	sec		ped	m			sec	m	m/sec	
Sou	th: Botany Ro	I (S)										
P1	Full	87	53.4	LOS E	0.3	0.3	0.95	0.95	70.1	20.0	0.29	
Eas	t: Wellington	St (E)										

P2 Full	46	53.3	LOS E	0.1	0.1	0.94	0.94	70.0	20.0	0.29
North: Botany Rd (N)									
P3 Full	71	53.4	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
West: Buckland St	(W)									
P4 Full	73	53.4	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
All Pedestrians	277	53.4	LOS E	0.3	0.3	0.94	0.94	70.0	20.0	0.29

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Site: WLO04 [WLO04 Cope St / Wellington St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

NA Site Category: (None) Stop (Two-Way)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	H Total	lows HV 1	Fl [Total	lows HV 1	Satn	Delay	Service	[Veh	Dist 1	Que	Stop Rate	No. of Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		11010		km/h
South	Сор	e St (S)													
1	L2	All MCs	33	0.0	33	0.0	0.040	7.6	LOS A	0.2	1.1	0.16	0.91	0.16	30.2
2	T1	All MCs	7	0.0	7	0.0	0.040	8.5	LOS A	0.2	1.1	0.16	0.91	0.16	30.2
3	R2	All MCs	6	0.0	6	0.0	0.040	8.6	LOS A	0.2	1.1	0.16	0.91	0.16	33.7
Appro	ach		46	0.0	46	0.0	0.040	7.9	LOS A	0.2	1.1	0.16	0.91	0.16	30.9
East:	Wellir	igton St (l	E)												
4	L2	All MCs	9	0.0	9	0.0	0.052	4.9	LOS A	0.2	1.6	0.23	0.20	0.23	41.8
5	T1	All MCs	44	0.0	44	0.0	0.052	0.5	LOS A	0.2	1.6	0.23	0.20	0.23	41.0
6	R2	All MCs	5	0.0	5	0.0	0.052	5.5	LOS A	0.2	1.6	0.23	0.20	0.23	41.0
Appro	ach		59	0.0	59	0.0	0.052	1.7	NA	0.2	1.6	0.23	0.20	0.23	41.2
North:	Cope	e St (N)													
7	L2	All MCs	3	0.0	3	0.0	0.011	7.7	LOS A	0.0	0.3	0.35	0.85	0.35	30.3
8	T1	All MCs	1	0.0	1	0.0	0.011	7.7	LOS A	0.0	0.3	0.35	0.85	0.35	31.5
9	R2	All MCs	5	0.0	5	0.0	0.011	7.9	LOS A	0.0	0.3	0.35	0.85	0.35	23.9
Appro	ach		9	0.0	9	0.0	0.011	7.8	LOS A	0.0	0.3	0.35	0.85	0.35	27.7
West:	Welli	ngton St ((W)												
10	L2	All MCs	23	0.0	23	0.0	0.094	4.7	LOS A	0.2	1.6	0.07	0.15	0.07	39.4
11	T1	All MCs	123	3.4	123	3.4	0.094	0.1	LOS A	0.2	1.6	0.07	0.15	0.07	45.3
12	R2	All MCs	19	0.0	19	0.0	0.094	4.7	LOS A	0.2	1.6	0.07	0.15	0.07	40.7
Appro	ach		165	2.5	165	2.5	0.094	1.3	NA	0.2	1.6	0.07	0.15	0.07	44.2
All Ve	nicles		280	1.5	280	1.5	0.094	2.7	NA	0.2	1.6	0.13	0.31	0.13	40.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: WLO05 [WLO05 Wyndham St / Henderson Rd (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 55

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Wyn	dham St	(S)												
1	L2	All MCs	13	0.0	13	0.0	0.677	68.2	LOS E	10.1	72.1	0.99	0.84	1.04	11.5
2	T1	All MCs	332	2.9	332	2.9	*0.677	58.5	LOS E	10.1	73.0	0.99	0.84	1.04	18.3
3	R2	All MCs	8	25.0	8	25.0	0.677	65.4	LOS E	10.1	73.0	0.99	0.84	1.04	12.0
Appro	ach		353	3.3	353	3.3	0.677	59.0	LOS E	10.1	73.0	0.99	0.84	1.04	16.8
East:	Hende	erson Rd	(E)												
4	L2	All MCs	214	2.5	214	2.5	0.329	14.0	LOS A	10.6	75.7	0.46	0.52	0.46	31.7
5	T1	All MCs	687	1.5	687	1.5	0.329	6.9	LOS A	11.2	79.7	0.46	0.40	0.46	27.4
6	R2	All MCs	655	5.9	655	5.9	*0.402	23.0	LOS B	9.8	72.4	0.69	0.81	0.69	21.9
Appro	ach		1556	3.5	1556	3.5	0.402	14.6	LOS B	11.2	79.7	0.56	0.59	0.56	24.7
West:	Hend	lerson Rd	(W)												
10	L2	All MCs	276	3.4	276	3.4	*0.651	72.6	LOS F	8.2	58.8	0.98	0.82	1.02	12.5
11	T1	All MCs	273	0.4	273	0.4	0.651	47.9	LOS D	14.3	100.4	0.97	0.82	0.97	4.9
Appro	ach		548	1.9	548	1.9	0.651	60.3	LOS E	14.3	100.4	0.98	0.82	1.00	8.3
All Ve	hicles		2457	3.1	2457	3.1	0.677	31.2	LOS C	14.3	100.4	0.71	0.68	0.72	17.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pec	lestrian Mov	/ement	Perform	nance							
Mo		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUEUE [Ped Dist] ped m		Que	Stop Rate	Time	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Wyndham	St (S)									
P1	Full	297	53.9	LOS E	1.0	1.0	0.95	0.95	70.5	20.0	0.28
Eas	t: Henderson	Rd (E)									
P2	Full	71	53.4	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
Nor	th: Wyndham	St (N)									
P3	Full	181	53.6	LOS E	0.6	0.6	0.95	0.95	70.3	20.0	0.28
Wes	st: Henderson	Rd (W)									

P4 Full	164	53.6	LOS E	0.5	0.5	0.95	0.95	70.2	20.0	0.28
All Pedestrians	713	53.7	LOS E	1.0	1.0	0.95	0.95	70.4	20.0	0.28

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Site: WLO06 [WLO06 Pedestrian Mid-block Crossing at Cope St (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Cop	e St (S)													
2	T1	All MCs	36	0.0	36	0.0	0.022	2.1	LOS A	0.1	0.5	0.09	0.35	0.09	32.5
Appro	ach		36	0.0	36	0.0	0.022	2.1	LOS A	0.1	0.5	0.09	0.35	0.09	32.5
North:	Cope	e St (N)													
8	T1	All MCs	7	0.0	7	0.0	0.005	2.1	LOS A	0.0	0.1	0.09	0.35	0.09	31.6
Appro	ach		7	0.0	7	0.0	0.005	2.1	LOS A	0.0	0.1	0.09	0.35	0.09	31.6
All Ve	hicles		43	0.0	43	0.0	0.022	2.1	NA	0.1	0.5	0.09	0.35	0.09	32.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: WLO01 [WLO01 Botany Rd / Raglan St / Henderson Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 47

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand lows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	: Bota	ny Rd (S))	%	ven/n	%	V/C	sec	_	ven	m	_	_	_	Km/n
1	L2	All MCs	, 551	3.8	551	3.8	* 0.556	45.6	LOS D	14.3	103.5	0.93	0.83	0.93	13.6
Appro	ach		551	3.8	551	3.8	0.556	45.6	LOS D	14.3	103.5	0.93	0.83	0.93	13.6
East:	Ragla	n St (E)													
4	L2	All MCs	15	0.0	15	0.0	0.508	62.7	LOS E	6.0	44.2	0.98	0.78	0.98	4.6
5	T1	All MCs	197	7.0	197	7.0	0.508	54.5	LOS D	6.0	44.4	0.98	0.78	0.98	4.6
Appro	ach		212	6.5	212	6.5	0.508	55.1	LOS D	6.0	44.4	0.98	0.78	0.98	4.6
North:	Bota	ny Rd (N))												
7	L2	All MCs	76	9.7	76	9.7	0.103	10.7	LOS A	2.1	15.8	0.30	0.46	0.30	33.6
8	T1	All MCs	763	4.3	763	4.3	0.516	7.0	LOS A	16.0	116.4	0.42	0.40	0.42	35.2
9	R2	All MCs	600	2.8	600	2.8	*0.792	56.8	LOS E	17.7	126.5	0.99	0.90	1.10	10.5
Appro	ach		1439	4.0	1439	4.0	0.792	28.0	LOS B	17.7	126.5	0.65	0.61	0.70	17.0
West:	Hend	lerson Rd	(W)												
11	T1	All MCs	191	2.2	191	2.2	*0.708	12.2	LOS A	4.1	29.4	0.46	0.39	0.48	15.8
12	R2	All MCs	62	0.0	62	0.0	0.708	37.1	LOS C	3.9	27.5	0.77	0.71	0.81	7.4
Appro	ach		253	1.7	253	1.7	0.708	18.3	LOS B	4.1	29.4	0.54	0.47	0.56	12.3
All Ve	hicles		2454	3.9	2454	3.9	0.792	33.3	LOS C	17.7	126.5	0.73	0.66	0.76	14.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mo	vement	Perform	nance							
Mo ID	v Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF EUE Dist 1	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
		ped/h	sec		ped	m		Tuto	sec	m	m/sec
Sou	uth: Botany Ro	d (S)									
P1	Full	181	53.6	LOS E	0.6	0.6	0.95	0.95	70.3	20.0	0.28
Eas	st: Raglan St (E)									
P2	Full	95	53.4	LOS E	0.3	0.3	0.95	0.95	70.1	20.0	0.29

North: Botany Rd	(N)									
P3 Full	85	53.4	LOS E	0.3	0.3	0.95	0.95	70.1	20.0	0.29
West: Henderson	Rd (W)									
P4 Full	87	53.4	LOS E	0.3	0.3	0.95	0.95	70.1	20.0	0.29
All Pedestrians	448	53.5	LOS E	0.6	0.6	0.95	0.95	70.2	20.0	0.29

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Site: WLO02 [WLO02 Raglan St / Cope St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 5057

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 35 seconds (Site User-Given Phase Times)

Vehic	cle M	ovemen	t Perfo	orma	ince										
Mov ID	Turn	Mov Class	Dem F	nand Iows	Ar F	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	km/b
South	· Con	e St (S)	ven/n	70	ven/n	70	V/C	Sec	_	ven	111	_	_	_	K111/11
1	10		10	0.0	10	0.0	0.077	15.9		0.5	2.2	0.70	0.62	0.70	16.9
			12	0.0	12	0.0	0.077	15.0		0.5	3.3	0.79	0.03	0.79	10.0
2	11		15	7.1	15	7.1	0.077	10.5	LUSA	0.5	3.3	0.79	0.63	0.79	31.5
3	R2	All MCs	7	0.0	7	0.0	0.077	14.0	LOSA	0.5	3.3	0.79	0.63	0.79	31.3
Appro	ach		34	3.1	34	3.1	0.077	13.1	LOS A	0.5	3.3	0.79	0.63	0.79	28.2
East:	Ragla	n St (E)													
4	L2	All MCs	2	0.0	2	0.0	0.168	16.0	LOS B	1.3	9.6	0.73	0.58	0.73	35.4
5	T1	All MCs	199	4.8	199	4.8	0.168	8.9	LOS A	1.3	9.6	0.73	0.58	0.73	35.3
6	R2	All MCs	7	0.0	7	0.0	0.168	15.1	LOS B	1.3	9.2	0.73	0.58	0.73	39.2
Appro	ach		208	4.5	208	4.5	0.168	9.2	LOS A	1.3	9.6	0.73	0.58	0.73	35.5
North	Cope	e St (N)													
7	L2	All MCs	14	0.0	14	0.0	0.031	13.2	LOS A	0.2	1.5	0.69	0.63	0.69	37.2
8	T1	All MCs	5	0.0	5	0.0	0.031	9.2	LOS A	0.2	1.5	0.69	0.63	0.69	30.9
9	R2	All MCs	38	2.8	38	2.8	*0.195	21.4	LOS B	0.7	4.7	0.94	0.71	0.94	23.9
Appro	ach		57	1.9	57	1.9	0.195	18.3	LOS B	0.7	4.7	0.85	0.68	0.85	28.4
West:	Ragla	an St (W))												
10	L2	All MCs	46	2.3	46	2.3	0.089	14.6	LOS B	0.6	4.3	0.76	0.69	0.76	29.5
11	T1	All MCs	207	4.6	207	4.6	*0.324	9.6	LOS A	2.7	20.0	0.78	0.64	0.78	36.7
Appro	ach		254	4.1	254	4.1	0.324	10.5	LOS A	2.7	20.0	0.78	0.65	0.78	35.2
All Ve	hicles		553	4.0	553	4.0	0.324	11.0	LOS A	2.7	20.0	0.77	0.63	0.77	34.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pe	destrian Mov	vement	Perforr	nance							
Mov ID	/ Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
		ned/h	50C		[Ped]	Dist]		Rate	80C	m	m/sec
Sou	th: Cope St (S)	000		peu				500		m/800
P1	Full	151	12.1	LOS B	0.1	0.1	0.83	0.83	178.8	200.0	1.12

East: Raglan St (E)									
P2 Full	45	12.0	LOS B	0.0	0.0	0.83	0.83	28.7	20.0	0.70
North: Cope St (N)										
P3 Full	122	12.1	LOS B	0.1	0.1	0.83	0.83	28.7	20.0	0.70
West: Raglan St (V	V)									
P4 Full	125	12.1	LOS B	0.1	0.1	0.83	0.83	28.7	20.0	0.70
All Pedestrians	443	12.1	LOS B	0.1	0.1	0.83	0.83	79.7	81.1	1.02

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Site: WLO03 [WLO03 Botany Rd / Wellington St / Buckland St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 137

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfc	orma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Prop.	Eff.	Aver.	Aver.
ID		Class	FI [Total	lows HV 1	FI [Total]	lows HV/1	Satn	Delay	Service	[\/eh	Dist 1	Que	Stop Rate	No. of Cycles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m		i tato	C y cicc	km/h
South	: Bota	iny Rd (S)												
2	T1	All MCs	505	4.2	505	4.2	0.428	6.0	LOS A	10.3	74.7	0.40	0.38	0.40	38.0
3	R2	All MCs	56	0.0	56	0.0	0.428	18.6	LOS B	10.3	74.7	0.44	0.43	0.44	36.8
Appro	ach		561	3.8	561	3.8	0.428	7.3	LOS A	10.3	74.7	0.40	0.39	0.40	37.9
East:	Wellir	igton St (E)												
4	L2	All MCs	46	0.0	46	0.0	0.214	59.6	LOS E	2.5	17.6	0.94	0.74	0.94	15.7
6	R2	All MCs	18	0.0	18	0.0	0.112	59.1	LOS E	1.0	6.9	0.94	0.70	0.94	4.3
Appro	Approach		64	0.0	64	0.0	0.214	59.4	LOS E	2.5	17.6	0.94	0.73	0.94	13.1
North:	Bota	ny Rd (N)												
7	L2	All MCs	11	0.0	11	0.0	0.112	12.0	LOS A	1.4	10.2	0.18	0.18	0.18	42.9
8	T1	All MCs	829	3.9	829	3.9	*0.561	8.2	LOS A	11.3	82.0	0.29	0.27	0.29	45.8
Appro	ach		840	3.9	840	3.9	0.561	8.3	LOS A	11.3	82.0	0.29	0.27	0.29	41.3
West:	Buck	land St (V	V)												
10	L2	All MCs	27	0.0	27	0.0	0.341	56.3	LOS D	4.8	33.8	0.94	0.75	0.94	5.2
11	T1	All MCs	62	0.0	62	0.0	*0.341	50.5	LOS D	4.8	33.8	0.94	0.75	0.94	5.2
12	R2	All MCs	37	0.0	37	0.0	0.196	56.7	LOS E	2.0	14.1	0.94	0.73	0.94	15.5
Appro	12 R2 All MC: Approach		126	0.0	126	0.0	0.341	53.6	LOS D	4.8	33.8	0.94	0.74	0.94	9.1
All Ve	hicles		1592	3.4	1592	3.4	0.561	13.6	LOS A	11.3	82.0	0.41	0.37	0.41	34.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Peo	destrian Mov	vement	Perforr	nance							
Mov		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist.	Speed
					[Ped	Dist]		Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	ith: Botany Ro	l (S)									
P1	Full	58	53.3	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
Eas	t: Wellington	St (E)									

P2 Full	48	53.3	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
North: Botany Rd (N)									
P3 Full	48	53.3	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
West: Buckland St	(W)									
P4 Full	69	53.4	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
All Pedestrians	224	53.3	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29

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Site: WLO04 [WLO04 Cope St / Wellington St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: WLO-N1 [WLO Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

NA Site Category: (None) Stop (Two-Way)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov	Turn	Mov	Demand		Ar	rival	Deg.	Aver.	Level of	95% Back	Of Queue	Eff.	Aver.	Aver.	
ID		Class	ا-۲ Total آ	lows HV 1	ا⊦ Total آ	lows HV 1	Satn	Delay	Service	[Veh.	Dist 1	Que	Stop Rate	No. of Cvcles	Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Cope St (S)															
1	L2	All MCs	18	0.0	18	0.0	0.028	7.6	LOS A	0.1	0.7	0.15	0.92	0.15	30.2
2	T1	All MCs	6	0.0	6	0.0	0.028	8.1	LOS A	0.1	0.7	0.15	0.92	0.15	30.2
3	R2	All MCs	7	0.0	7	0.0	0.028	8.0	LOS A	0.1	0.7	0.15	0.92	0.15	33.8
Appro	ach		32	0.0	32	0.0	0.028	7.8	LOS A	0.1	0.7	0.15	0.92	0.15	31.4
East: Wellington St (E)															
4	L2	All MCs	8	0.0	8	0.0	0.040	4.7	LOS A	0.2	1.3	0.16	0.18	0.16	42.0
5	T1	All MCs	35	0.0	35	0.0	0.040	0.3	LOS A	0.2	1.3	0.16	0.18	0.16	41.5
6	R2	All MCs	5	0.0	5	0.0	0.040	5.2	LOS A	0.2	1.3	0.16	0.18	0.16	41.5
Appro	ach		48	0.0	48	0.0	0.040	1.6	NA	0.2	1.3	0.16	0.18	0.16	41.6
North: Cope St (N)															
7	L2	All MCs	4	0.0	4	0.0	0.013	7.3	LOS A	0.0	0.3	0.29	0.88	0.29	30.7
8	T1	All MCs	4	0.0	4	0.0	0.013	7.4	LOS A	0.0	0.3	0.29	0.88	0.29	31.9
9	R2	All MCs	4	0.0	4	0.0	0.013	7.4	LOS A	0.0	0.3	0.29	0.88	0.29	24.4
Appro	ach		13	0.0	13	0.0	0.013	7.4	LOS A	0.0	0.3	0.29	0.88	0.29	29.7
West: Wellington St (W)															
10	L2	All MCs	9	0.0	9	0.0	0.102	4.7	LOS A	0.4	2.9	0.12	0.15	0.12	39.3
11	T1	All MCs	97	0.0	97	0.0	0.102	0.3	LOS A	0.4	2.9	0.12	0.15	0.12	45.2
12	R2	All MCs	17	0.0	17	0.0	0.102	4.7	LOS A	0.4	2.9	0.12	0.15	0.12	40.6
Approach			123	0.0	123	0.0	0.102	1.2	NA	0.4	2.9	0.12	0.15	0.12	44.2
All Vehicles			216	0.0	216	0.0	0.102	2.6	NA	0.4	2.9	0.15	0.31	0.15	40.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: WLO05 [WLO05 Wyndham St / Henderson Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 55

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Wyn	dham St	(S)												
1	L2	All MCs	9	0.0	9	0.0	0.584	61.7	LOS E	8.7	61.8	0.99	0.80	0.99	11.5
2	T1	All MCs	297	1.8	297	1.8	*0.584	53.1	LOS D	8.8	62.9	0.99	0.80	0.99	18.3
3	R2	All MCs	4	50.0	4	50.0	0.584	60.1	LOS E	8.8	62.9	0.99	0.80	0.99	12.0
Appro	ach		311	2.4	311	2.4	0.584	53.4	LOS D	8.8	62.9	0.99	0.80	0.99	18.0
East:	Hende	erson Rd	(E)												
4	L2	All MCs	137	3.1	137	3.1	0.278	12.5	LOS A	9.2	65.8	0.45	0.42	0.45	33.5
5	T1	All MCs	651	2.3	651	2.3	0.278	6.4	LOS A	9.5	67.7	0.45	0.35	0.45	28.8
6	R2	All MCs	560	5.8	560	5.8	*0.331	21.0	LOS B	7.7	56.9	0.68	0.79	0.68	22.9
Appro	ach		1347	3.8	1347	3.8	0.331	13.1	LOS A	9.5	67.7	0.54	0.54	0.54	25.7
West:	Hend	lerson Rd	(W)												
10	L2	All MCs	304	1.4	304	1.4	*0.667	73.2	LOS F	8.6	60.9	0.98	0.83	1.03	12.5
11	T1	All MCs	248	0.8	248	0.8	0.617	47.0	LOS D	13.4	94.2	0.96	0.81	0.96	4.9
Appro	ach		553	1.1	553	1.1	0.667	61.4	LOS E	13.4	94.2	0.97	0.82	1.00	8.5
All Ve	hicles		2211	3.0	2211	3.0	0.667	30.8	LOS C	13.4	94.2	0.71	0.65	0.72	17.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pec	lestrian Mov	vement	Perform	nance							
Mo		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Wyndham	St (S)									
P1	Full	109	53.4	LOS E	0.4	0.4	0.95	0.95	70.1	20.0	0.29
Eas	t: Henderson	Rd (E)									
P2	Full	48	53.3	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
Nor	th: Wyndham	St (N)									
P3	Full	68	53.4	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
Wes	st: Henderson	Rd (W)									

P4 Full	59	53.3	LOS E	0.2	0.2	0.94	0.94	70.0	20.0	0.29
All Pedestrians	285	53.4	LOS E	0.4	0.4	0.94	0.94	70.0	20.0	0.29

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: WLO06 [WLO06 Pedestrian Mid-block Crossing at Cope St (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Pedestrian Crossing (Unsignalised)

Vehicle Movement Performance

				/iniu											
Mov ID	Turn	Mov Class	Derr Fl	nand Iows	Ar F	rival Iows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Cop	e St (S)													
2	T1	All MCs	21	0.0	21	0.0	0.013	2.1	LOS A	0.0	0.3	0.09	0.35	0.09	32.5
Appro	ach		21	0.0	21	0.0	0.013	2.1	LOS A	0.0	0.3	0.09	0.35	0.09	32.5
North	Cope	e St (N)													
8	T1	All MCs	11	0.0	11	0.0	0.006	2.1	LOS A	0.0	0.2	0.09	0.35	0.09	31.6
Appro	ach		11	0.0	11	0.0	0.006	2.1	LOS A	0.0	0.2	0.09	0.35	0.09	31.6
All Ve	hicles		32	0.0	32	0.0	0.013	2.1	NA	0.0	0.3	0.09	0.35	0.09	32.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: SYD01 [SYD01 Railway Pde / Gleeson Ave (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: SYD-N1 [SYD Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 3320 Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 75 seconds (Site User-Given Phase Times)

Vehic	le M	ovement	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Derr F	nand lows	Ar Fl	rival lows	Deg. Satn	Aver. Delav	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	' km/h
South	East:	Gleeson	Ave (SI	Ξ)											
1	L2	All MCs	483	7.2	483	7.2	0.211	4.5	LOS A	0.0	0.0	0.00	0.51	0.00	42.3
Appro	ach		483	7.2	483	7.2	0.211	4.5	LOS A	0.0	0.0	0.00	0.51	0.00	42.3
North	East:	Railway F	Pde (NE)											
4	L2	All MCs	1009	5.6	1009	5.6	*0.505	14.4	LOS A	7.4	54.6	0.57	0.75	0.57	31.9
5	T1	All MCs	107	12.7	107	12.7	0.082	9.5	LOS A	1.0	7.7	0.27	0.22	0.27	54.4
Appro	ach		1117	6.3	1117	6.3	0.505	14.0	LOS A	7.4	54.6	0.54	0.70	0.54	30.5
All Ve	hicles		1600	6.6	1600	6.6	0.505	11.1	LOS A	7.4	54.6	0.38	0.64	0.38	33.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	UE Diet 1	Que	Stop Rate	lime	Dist.	Speed
	ped/h	sec		ped	m		Tate	sec	m	m/sec
NorthEast: Railwa	ay Pde (l	NE)								
P2 Full	144	31.0	LOS D	0.3	0.3	0.91	0.91	47.7	20.0	0.42
P2S Slip/	257	18.2	LOS B	0.4	0.4	0.82	0.82	34.9	20.0	0.57
Bypass										
All Pedestrians	401	22.8	LOS C	0.4	0.4	0.85	0.85	39.5	20.0	0.51

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: SYD02 [SYD02 Burrows Ave / Gleeson Ave (Site Folder: Block 4 Model - 2024 AM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: SYD-N1 [SYD Network 1 (Network Folder: Block 4 Network - 2024 AM Peak)]

TCS 1152

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 110 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem F [Total	nand Iows HV]	Ar Fl [Total	rival lows HV]	Deg. Satn	Aver. Delay	Level of Service	95% Back [Veh.	Of Queue Dist]	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	East:	Gleeson	Ave (Sl	E)											
2	T1	All MCs	682	4.5	682	4.5	0.341	15.3	LOS B	10.2	74.4	0.61	0.53	0.61	20.9
Appro	ach		682	4.5	682	4.5	0.341	15.3	LOS B	10.2	74.4	0.61	0.53	0.61	20.9
North	East:	Burrows A	Ave (NE	E)											
4	L2	All MCs	57	3.7	57	3.7	0.292	54.9	LOS D	2.9	21.0	0.96	0.75	0.96	13.7
6	R2	All MCs	181	3.5	181	3.5	*0.371	52.5	LOS D	4.6	32.8	0.95	0.77	0.95	9.6
Appro	ach		238	3.5	238	3.5	0.371	53.1	LOS D	4.6	32.8	0.95	0.77	0.95	10.7
North	West:	Gleeson	Ave (N	W)											
7	L2	All MCs	200	7.4	200	7.4	0.533	7.1	LOS A	7.4	54.6	0.30	0.43	0.30	34.2
8	T1	All MCs	823	6.3	823	6.3	*0.533	5.4	LOS A	7.8	57.5	0.30	0.33	0.30	40.5
Appro	ach		1023	6.5	1023	6.5	0.533	5.7	LOS A	7.8	57.5	0.30	0.35	0.30	38.9
South	West	Burrows	Ave (S	W)											
10	L2	All MCs	25	45.8	25	45.8	0.148	59.2	LOS E	1.3	10.6	0.95	0.70	0.95	10.8
11	T1	All MCs	16	0.0	16	0.0	0.148	45.7	LOS D	1.3	10.6	0.94	0.69	0.94	17.3
12	R2	All MCs	12	18.2	12	18.2	0.046	46.9	LOS D	0.5	4.3	0.87	0.68	0.87	16.2
Appro	ach		53	26.0	53	26.0	0.148	52.5	LOS D	1.3	10.6	0.93	0.69	0.93	14.0
All Ve	hicles		1996	6.0	1996	6.0	0.533	15.9	LOS B	10.2	74.4	0.50	0.47	0.50	23.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	FIOW	Delay	Service	QUE [Ped	:UE Dist 1	Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
SouthEast: Glees	on Ave (SE)								
P1 Full	46	46.5	LOS E	0.1	0.1	0.92	0.92	63.1	20.0	0.32
NorthEast: Burrow	vs Ave (I	NE)								
P2 Full	186	48.6	LOS E	0.5	0.5	0.94	0.94	65.3	20.0	0.31
NorthWest: Glees	on Ave ((NW)								

P3 Full	398	45.3	LOS E	1.1	1.1	0.92	0.92	62.0	20.0	0.32
SouthWest: Burro	ws Ave (SW)								
P4 Full	262	48.8	LOS E	0.8	0.8	0.95	0.95	65.4	20.0	0.31
All Pedestrians	893	47.1	LOS E	1.1	1.1	0.93	0.93	63.7	20.0	0.31

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: SYD03 [SYD03 Burrows Ave / George St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le Mo	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand Iows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Qi [Veh. veh	Back Of Jeue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	George S	St (SE)												
4	L2	All MCs	17	0.0	17	0.0	0.024	8.1	LOS A	0.1	0.5	0.25	0.88	0.25	30.6
6	R2	All MCs	7	0.0	7	0.0	0.024	9.4	LOS A	0.1	0.5	0.25	0.88	0.25	26.6
Approa	ach		24	0.0	24	0.0	0.024	8.5	LOS A	0.1	0.5	0.25	0.88	0.25	29.5
NorthE	ast: I	Burrows A	Ave (NE)											
7	L2	All MCs	12	0.0	12	0.0	0.176	4.1	LOS A	0.9	6.4	0.31	0.19	0.31	39.2
8	T1	All MCs	167	4.4	167	4.4	0.176	1.0	LOS A	0.9	6.4	0.31	0.19	0.31	45.0
Approa	ach		179	4.1	179	4.1	0.176	1.2	NA	0.9	6.4	0.31	0.19	0.31	44.6
South	Nest:	Burrows	Ave (S	N)											
2	T1	All MCs	192	7.1	192	7.1	0.200	1.0	LOS A	0.9	6.5	0.26	0.16	0.26	44.7
3	R2	All MCs	9	0.0	9	0.0	0.200	5.2	LOS A	0.9	6.5	0.26	0.16	0.26	40.6
Approa	ach		201	6.8	201	6.8	0.200	1.2	NA	0.9	6.5	0.26	0.16	0.26	44.4
All Veh	nicles		404	5.2	404	5.2	0.200	1.6	NA	0.9	6.5	0.28	0.22	0.28	42.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: SYD04 [SYD04 Railway Pde / Sydenham Rd (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 4946

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 101 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veb/b	nand lows HV] %	Ar Fl [Total] veb/b	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% E Qu [Veh. veh	Back Of eue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
North\	Nest:	Sydenha	m Rd (N	NW)	VOII/II	70	v/0			Ven					KIT/T
28	T1	All MCs	1119	6.4	1119	6.4	* 0.450	6.4	LOS A	9.1	68.0	0.51	0.48	0.51	44.6
29	R2	All MCs	1	0.0	1	0.0	0.450	12.1	LOS A	8.9	65.3	0.51	0.48	0.51	39.6
Appro	ach		1120	6.4	1120	6.4	0.450	6.4	LOS A	9.1	68.0	0.51	0.48	0.51	44.6
South	West:	Railway	Pde (S\	N)											
32	R2	All MCs	7	0.0	7	0.0	0.022	30.0	LOS C	0.2	1.7	0.85	0.66	0.85	26.5
Appro	ach		7	0.0	7	0.0	0.022	30.0	LOS C	0.2	1.7	0.85	0.66	0.85	26.5
All Vel	nicles		1127	6.3	1127	6.3	0.450	6.6	LOS A	9.1	68.0	0.52	0.48	0.52	44.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	loveme	nt Perf	ormand	e:							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	ped/h	sec		[Ped ped	Dist J m		Rate	sec	m	m/sec
NorthWest: Sy	denham	Rd (NW	/)								
P7 Full	71	75	25.2	LOS C	0.1	0.1	0.82	0.82	191.8	200.0	1.04
All Pedestrians	71	75	25.2	LOS C	0.1	0.1	0.82	0.82	191.8	200.0	1.04

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: SYD05 [SYD05 Marrickville Rd / Buckley St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	95%	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI Total	lows	FI Total	lows	Satn	Delay	Service	Q	ueue	Que	Stop	No. of	Speed
			veh/h	пvј %	veh/h	⊓vj %	v/c	sec		ven.	m Dist		Rale	Cycles	km/h
South	East:	Marrickvi	lle Rd (SE)											
2	T1	All MCs	511	6.8	511	6.8	0.280	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.8
3	R2	All MCs	533	5.3	533	5.3	0.739	8.7	LOS A	4.9	35.6	0.58	0.72	0.76	42.3
Appro	ach		1043	6.1	1043	6.1	0.739	4.4	NA	4.9	35.6	0.29	0.37	0.39	50.4
North\	Nest:	Marrickvi	lle Rd (NW)											
7	L2	All MCs	501	5.9	501	5.9	0.887	12.4	LOS A	8.9	65.6	0.91	0.95	1.47	44.3
Appro	ach		501	5.9	501	5.9	0.887	12.4	NA	8.9	65.6	0.91	0.95	1.47	44.3
All Vel	hicles		1544	6.0	1544	6.0	0.887	7.0	NA	8.9	65.6	0.49	0.56	0.74	47.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: SYD06 [SYD06 Sydenham Rd / Buckley St (Site Folder: Block 4 Model - 2024 AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Den	nand	Ar	rival	Deg.	Aver.	Level of	95% E	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI	lows	FI	ows	Satn	Delay	Service	Qu	eue	Que	Stop	No. of	Speed
			[Total	HV]	[Total	HV] %	vic	800		[Veh.	Dist]		Rate	Cycles	km/h
North	Nost.	Sydenha	m Rd (N		Ven/m	/0		360		Ven	- 111				K(1)/11
NOTUN	110051.	Syuenna	in ixu (i	NVV)											
2	T1	All MCs	769	6.2	769	6.2	0.418	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach		769	6.2	769	6.2	0.418	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.7
South	West:	Buckley	St (SW))											
4	L2	All MCs	549	5.2	549	5.2	0.312	5.7	LOS A	0.0	0.0	0.00	0.53	0.00	51.0
6	R2	All MCs	452	7.0	452	7.0	0.261	5.8	LOS A	0.0	0.0	0.00	0.63	0.00	43.4
Appro	ach		1001	6.0	1001	6.0	0.312	5.8	NA	0.0	0.0	0.00	0.57	0.00	48.4
All Ve	hicles		1771	6.1	1771	6.1	0.418	3.3	NA	0.0	0.0	0.00	0.32	0.00	52.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: SYD01 [SYD01 Railway Pde / Gleeson Ave (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■ Network: SYD-N1 [SYD Network 1 (Network Folder: Block 4 Network - 2024 PM Peak)]

TCS 3320 Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 70 seconds (Site User-Given Phase Times)

Vehio	cle M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand lows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	East:	Gleeson	Ave (SE	Ξ)											
1	L2	All MCs	580	3.8	580	3.8	0.245	4.6	LOS A	0.0	0.0	0.00	0.52	0.00	42.9
Appro	ach		580	3.8	580	3.8	0.245	4.6	LOS A	0.0	0.0	0.00	0.52	0.00	42.9
North	East:	Railway F	Pde (NE)											
4	L2	All MCs	979	3.1	979	3.1	*0.442	13.2	LOS A	6.5	47.0	0.55	0.73	0.55	33.4
5	T1	All MCs	64	0.0	64	0.0	0.044	9.0	LOS A	0.6	4.0	0.28	0.22	0.28	54.3
Appro	ach		1043	2.9	1043	2.9	0.442	12.9	LOS A	6.5	47.0	0.53	0.69	0.53	31.3
All Ve	hicles		1623	3.2	1623	3.2	0.442	9.9	LOS A	6.5	47.0	0.34	0.63	0.34	35.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUE [Ped	:UE Dist 1	Que	Stop Rate	lime	Dist.	Speed
	ped/h	sec		ped	m		Tuto	sec	m	m/sec
NorthEast: Railwa	ay Pde (l	NE)								
P2 Full	176	28.6	LOS C	0.3	0.3	0.91	0.91	45.2	20.0	0.44
P2S Slip/	240	16.0	LOS B	0.3	0.3	0.81	0.81	32.7	20.0	0.61
Bypass										
All Pedestrians	416	21.3	LOS C	0.3	0.3	0.85	0.85	38.0	20.0	0.53

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: SYD02 [SYD02 Burrows Ave / Gleeson Ave (Site Folder: Block 4 Model - 2024 PM Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 1152

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 105 seconds (Site User-Given Phase Times)

Vehic	le M	ovemen	t Perfo	orma	nce										
Mov ID	Turn	Mov Class	Dem Fl	nand Iows	Ar Fl	rival ows	Deg. Satn	Aver. Delay	Level of Service	95% Bacl	< Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	East:	Gleeson	Ave (SE	Ξ)											
2	T1	All MCs	829	2.0	829	2.0	0.430	17.5	LOS B	13.3	94.7	0.68	0.60	0.68	19.1
Appro	ach		829	2.0	829	2.0	0.430	17.5	LOS B	13.3	94.7	0.68	0.60	0.68	19.1
North	East:	Burrows /	Ave (NE	=)											
4	L2	All MCs	52	0.0	52	0.0	0.208	63.4	LOS E	2.4	16.9	0.93	0.74	0.93	14.8
6	R2	All MCs	329	2.6	329	2.6	*0.629	57.9	LOS E	8.6	61.8	0.99	0.82	1.01	9.8
Appro	ach		381	2.2	381	2.2	0.629	58.6	LOS E	8.6	61.8	0.98	0.81	1.00	9.4
North	Nest:	Gleeson	Ave (N	W)											
7	L2	All MCs	142	0.7	142	0.7	0.520	6.8	LOS A	6.6	47.6	0.29	0.38	0.29	34.9
8	T1	All MCs	831	3.5	831	3.5	*0.520	5.4	LOS A	6.9	49.8	0.29	0.31	0.29	40.6
Appro	ach		973	3.1	973	3.1	0.520	5.6	LOS A	6.9	49.8	0.29	0.32	0.29	39.5
South	West:	Burrows	Ave (S	W)											
10	L2	All MCs	25	41.7	25	41.7	0.100	53.1	LOS D	0.8	6.8	0.93	0.69	0.93	11.5
11	T1	All MCs	4	0.0	4	0.0	0.100	41.5	LOS C	0.8	6.8	0.93	0.69	0.93	17.5
12	R2	All MCs	23	18.2	23	18.2	0.087	44.8	LOS D	1.0	8.2	0.87	0.71	0.87	16.7
Appro	ach		53	28.0	53	28.0	0.100	48.5	LOS D	1.0	8.2	0.90	0.70	0.90	14.4
All Ve	hicles		2236	3.2	2236	3.2	0.629	20.1	LOS B	13.3	94.7	0.57	0.51	0.57	20.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov Crossing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.
	FIUW	Delay	Service	[Ped	Dist]	Que	Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
SouthEast: Glees	on Ave ((SE)								
P1 Full	37	44.0	LOS E	0.1	0.1	0.92	0.92	60.6	20.0	0.33
NorthEast: Burrow	ws Ave (NE)								
P2 Full	240	46.2	LOS E	0.7	0.7	0.94	0.94	62.9	20.0	0.32
NorthWest: Glees	son Ave	(NW)								

P3 Full	238	42.5	LOS E	0.6	0.6	0.90	0.90	59.2	20.0	0.34
SouthWest: Burro	ws Ave (SW)								
P4 Full	149	46.0	LOS E	0.4	0.4	0.94	0.94	62.7	20.0	0.32
All Pedestrians	664	44.7	LOS E	0.7	0.7	0.93	0.93	61.4	20.0	0.33

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: SYD03 [SYD03 Burrows Ave / George St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le Mo	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veh/ <u>h</u>	nand Iows HV] <u>%</u>	Ar Fl [Total veh/ <u>h</u>	rival lows HV] %_	Deg. Satn v/ <u>c</u>	Aver. Delay se <u>c</u>	Level of Service	95% Qi [Veh. veh	Back Of Jeue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/ <u>h</u>
South	East:	George S	St (SE)												
4	L2	All MCs	21	0.0	21	0.0	0.028	8.7	LOS A	0.1	0.6	0.32	0.89	0.32	30.1
6	R2	All MCs	5	0.0	5	0.0	0.028	10.3	LOS A	0.1	0.6	0.32	0.89	0.32	26.2
Appro	ach		26	0.0	26	0.0	0.028	9.0	LOS A	0.1	0.6	0.32	0.89	0.32	29.4
NorthE	ast: I	Burrows A	Ave (NE)											
7	L2	All MCs	12	0.0	12	0.0	0.306	4.3	LOS A	1.8	12.4	0.36	0.21	0.36	39.0
8	T1	All MCs	302	0.7	302	0.7	0.306	1.2	LOS A	1.8	12.4	0.36	0.21	0.36	44.8
Appro	ach		314	0.7	314	0.7	0.306	1.3	NA	1.8	12.4	0.36	0.21	0.36	44.6
South	Nest:	Burrows	Ave (S	N)											
2	T1	All MCs	149	1.4	149	1.4	0.157	1.0	LOS A	0.7	4.7	0.26	0.18	0.26	44.3
3	R2	All MCs	13	0.0	13	0.0	0.157	5.8	LOS A	0.7	4.7	0.26	0.18	0.26	40.3
Appro	ach		162	1.3	162	1.3	0.157	1.3	NA	0.7	4.7	0.26	0.18	0.26	43.8
All Vel	nicles		502	0.8	502	0.8	0.306	1.7	NA	1.8	12.4	0.33	0.24	0.33	42.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: SYD04 [SYD04 Railway Pde / Sydenham Rd (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 4946

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 106 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem F [Total veb/b	nand lows HV] %	Ar Fl [Total] veb/b	rival lows HV] %	Deg. Satn	Aver. Delay	Level of Service	95% Q [Veh. veh	Back Of ueue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
North\	Nest:	Sydenha	m Rd (N	WV)	VOII/II		110			Vort					TXTT // TT
28	T1	All MCs	1076	3.2	1076	3.2	*0.420	5.6	LOS A	8.9	65.2	0.50	0.45	0.50	46.1
29	R2	All MCs	1	0.0	1	0.0	0.420	11.3	LOS A	8.8	62.1	0.50	0.45	0.50	40.5
Appro	ach		1077	3.2	1077	3.2	0.420	5.6	LOS A	8.9	65.2	0.50	0.45	0.50	46.1
South	West:	Railway	Pde (S\	N)											
32	R2	All MCs	17	12.5	17	12.5	*0.057	31.8	LOS C	0.6	4.6	0.86	0.69	0.86	25.3
Appro	ach		17	12.5	17	12.5	0.057	31.8	LOS C	0.6	4.6	0.86	0.69	0.86	25.3
All Vel	nicles		1094	3.4	1094	3.4	0.420	6.0	LOS A	8.9	65.2	0.51	0.45	0.51	45.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian M	loveme	nt Perf	ormand	;e							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUI [Ped	EBACK OF EUE Dist]	Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
NorthWest: Sy	denham	Rd (NW	/)								
P7 Full	72	76	26.3	LOS C	0.2	0.2	0.83	0.83	192.9	200.0	1.04
All Pedestrians	72	76	26.3	LOS C	0.2	0.2	0.83	0.83	192.9	200.0	1.04

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: SYD05 [SYD05 Marrickville Rd / Buckley St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le Mo	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total veh/h	iand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Q [Veh. veh	Back Of ueue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Marrickvi	lle Rd (SE)											
2	T1	All MCs	747	2.1	747	2.1	0.391	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
3	R2	All MCs	554	1.9	554	1.9	0.706	7.7	LOS A	4.7	33.4	0.47	0.64	0.57	43.6
Appro	ach		1301	2.0	1301	2.0	0.706	3.3	NA	4.7	33.4	0.20	0.27	0.24	52.5
North\	Nest:	Marrickvi	lle Rd (NW)											
7	L2	All MCs	338	4.7	338	4.7	0.692	7.8	LOS A	3.0	22.1	0.53	0.64	0.63	48.2
Appro	ach		338	4.7	338	4.7	0.692	7.8	NA	3.0	22.1	0.53	0.64	0.63	48.2
All Vel	hicles		1639	2.6	1639	2.6	0.706	4.2	NA	4.7	33.4	0.27	0.35	0.32	51.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: SYD06 [SYD06 Sydenham Rd / Buckley St (Site Folder: Block 4 Model - 2024 PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	le M	ovemen	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival lows	Deg. Satn	Aver. Delav	Level of Service	95% I	Back Of	Prop.	Eff. Stop	Aver.	Aver.
		01033	[Total veh/h	HV]	[Total veh/h	HV]	v/c	sec	Cervice	[Veh. veh	Dist]	Que	Rate	Cycles	km/h
North	West:	Sydenha	m Rd (I	WV)	Voli/II	,,,	110	000		Voli					
2	T1	All MCs	761	1.5	761	1.5	0.396	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach		761	1.5	761	1.5	0.396	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.7
South	West:	Buckley	St (SW))											
4	L2	All MCs	544	2.1	544	2.1	0.300	5.7	LOS A	0.0	0.0	0.00	0.53	0.00	51.1
6	R2	All MCs	343	5.5	343	5.5	0.196	5.8	LOS A	0.0	0.0	0.00	0.63	0.00	43.6
Appro	ach		887	3.4	887	3.4	0.300	5.7	NA	0.0	0.0	0.00	0.57	0.00	49.0
All Ve	hicles		1648	2.6	1648	2.6	0.396	3.1	NA	0.0	0.0	0.00	0.31	0.00	53.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: SYD01 [SYD01 Railway Pde / Gleeson Ave (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: SYD-N1 [SYD Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 3320 Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 95 seconds (Site User-Given Phase Times)

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Dem Fl	nand lows	Ar Fl	rival lows	Deg. Satn	Aver. Delay	Level of Service	95% Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
SouthEast: Gleeson Ave (SE)															
1	L2	All MCs	479	3.3	479	3.3	0.201	4.5	LOS A	0.0	0.0	0.00	0.52	0.00	43.0
Appro	ach		479	3.3	479	3.3	0.201	4.5	LOS A	0.0	0.0	0.00	0.52	0.00	43.0
North	East:	Railway F	Pde (NE)											
4	L2	All MCs	1054	2.6	1054	2.6	* 0.434	10.2	LOS A	9.3	66.5	0.36	0.66	0.36	35.2
5	T1	All MCs	94	1.1	94	1.1	*0.066	5.0	LOS A	0.8	5.4	0.29	0.23	0.29	55.3
Appro	ach		1147	2.5	1147	2.5	0.434	9.8	LOS A	9.3	66.5	0.35	0.63	0.35	35.6
All Ve	hicles		1626	2.7	1626	2.7	0.434	8.2	LOS A	9.3	66.5	0.25	0.59	0.25	37.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance													
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.			
ID Crossing	Flow	Delay	Service	QUE [Ped	:UE Dist 1	Que	Stop Rate	lime	Dist.	Speed			
	ped/h	sec		ped	m		Tuto	sec	m	m/sec			
NorthEast: Railwa	ay Pde (l	NE)											
P2 Full	147	25.4	LOS C	0.3	0.3	0.86	0.86	42.0	20.0	0.48			
P2S Slip/	155	41.0	LOS E	0.4	0.4	0.93	0.93	57.7	20.0	0.35			
Bypass													
All Pedestrians	302	33.4	LOS D	0.4	0.4	0.90	0.90	50.1	20.0	0.40			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: SYD02 [SYD02 Burrows Ave / Gleeson Ave (Site Folder: Block 4 Model - 2024 Weekend Peak)] Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: SYD-N1 [SYD Network 1 (Network Folder: Block 4 Network - 2024 Weekend Peak)]

TCS 1152

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 115 seconds (Site User-Given Phase Times)

Vehic	Vehicle Movement Performance Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Queue Prop. Eff. Aver. Aver.														
Mov ID	Turn	Mov Class	Dem Fl [Total] veh/h	and ows HV] %	Ar Fl [Total] veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back [Veh. veh	Of Queue Dist] m	e Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Gleeson	Ave (SE	E)											
2	T1	All MCs	785	2.0	785	2.0	0.347	12.8	LOS A	11.1	79.2	0.55	0.49	0.55	23.4
Appro	ach		785	2.0	785	2.0	0.347	12.8	LOS A	11.1	79.2	0.55	0.49	0.55	23.4
North	East:	Burrows /	Ave (NE)											
4	L2	All MCs	39	0.0	39	0.0	0.186	57.3	LOS E	2.0	14.2	0.94	0.73	0.94	13.6
6	R2	All MCs	174	2.4	174	2.4	*0.464	56.5	LOS D	5.5	39.4	0.97	0.77	0.97	9.0
Appro	ach		213	2.0	213	2.0	0.464	56.6	LOS E	5.5	39.4	0.96	0.76	0.96	10.0
North	West:	Gleeson	Ave (N	W)											
7	L2	All MCs	220	2.4	220	2.4	0.424	6.9	LOS A	5.9	42.4	0.25	0.43	0.25	34.6
8	T1	All MCs	826	3.3	826	3.3	*0.531	5.4	LOS A	9.0	64.8	0.28	0.31	0.28	41.6
Appro	ach		1046	3.1	1046	3.1	0.531	5.7	LOS A	9.0	64.8	0.28	0.34	0.28	39.6
South	West:	Burrows	Ave (S	W)											
10	L2	All MCs	25	29.2	25	29.2	*0.181	58.7	LOS E	1.3	11.6	0.95	0.71	0.95	10.3
11	T1	All MCs	9	11.1	9	11.1	0.042	45.2	LOS D	0.5	3.8	0.88	0.62	0.88	18.4
12	R2	All MCs	8	0.0	8	0.0	0.031	49.8	LOS D	0.4	2.8	0.88	0.66	0.88	15.7
Appro	ach		43	19.5	43	19.5	0.181	54.0	LOS D	1.3	11.6	0.92	0.68	0.92	13.2
All Ve	hicles		2087	2.9	2087	2.9	0.531	14.6	LOS B	11.1	79.2	0.46	0.44	0.46	24.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance													
Mov LD Crossing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop.	Eff.	Travel	Travel	Aver.			
	FIOW	Delay	Service	QUEUE [Ped Dist]		Que	Rate	Time	Dist.	Speed			
ped/h sec ped m sec m m/s													
SouthEast: Gleeson Ave (SE)													
P1 Full	14	48.9	LOS E	0.0	0.0	0.92	0.92	65.5	20.0	0.31			
NorthEast: Burrow	ws Ave (NE)											
P2 Full 213 51.2 LOS E 0.7 0.7 0.95 0.95 67.8 20.0 0.29													
NorthWest: Glees	son Ave	(NW)											

P3 Full	144	47.3	LOS E	0.4	0.4	0.91	0.91	64.0	20.0	0.31
SouthWest: Burro	ws Ave (SW)								
P4 Full	101	50.9	LOS E	0.3	0.3	0.94	0.94	67.6	20.0	0.30
All Pedestrians	472	49.9	LOS E	0.7	0.7	0.93	0.93	66.5	20.0	0.30

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: SYD03 [SYD03 Burrows Ave / George St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Derr F [Total veh/h	nand Iows HV] %	Ar Fl [Total] veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Qı [Veh. veh	Back Of Jeue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	George S	St (SE)												
4	L2	All MCs	24	0.0	24	0.0	0.036	8.2	LOS A	0.1	0.8	0.28	0.89	0.28	30.3
6	R2	All MCs	11	0.0	11	0.0	0.036	10.0	LOS A	0.1	0.8	0.28	0.89	0.28	26.4
Appro	ach		35	0.0	35	0.0	0.036	8.7	LOS A	0.1	0.8	0.28	0.89	0.28	29.2
NorthE	Burrows A	Ave (NE)												
7	L2	All MCs	16	0.0	16	0.0	0.205	4.2	LOS A	1.1	7.5	0.33	0.21	0.33	38.9
8	T1	All MCs	193	2.2	193	2.2	0.205	1.1	LOS A	1.1	7.5	0.33	0.21	0.33	44.6
Appro	ach		208	2.0	208	2.0	0.205	1.3	NA	1.1	7.5	0.33	0.21	0.33	44.1
South	West:	Burrows	Ave (S	N)											
2	T1	All MCs	213	2.5	213	2.5	0.232	1.1	LOS A	1.0	7.5	0.28	0.21	0.28	43.4
3	R2	All MCs	28	0.0	28	0.0	0.232	5.4	LOS A	1.0	7.5	0.28	0.21	0.28	39.7
Appro	ach		241	2.2	241	2.2	0.232	1.6	NA	1.0	7.5	0.28	0.21	0.28	42.8
All Vel	nicles		484	2.0	484	2.0	0.232	2.0	NA	1.1	7.5	0.30	0.26	0.30	41.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: SYD04 [SYD04 Railway Pde / Sydenham Rd (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

TCS 4946

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 116 seconds (Site User-Given Phase Times)

Vehic	le Mo	ovement	l Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem Fl [Total veh/h	nand Iows HV] %	Ar Fl [Total] veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% E Qu [Veh. veh	Back Of eue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
North	Vest:	Sydenha	m Rd (N	W)											
28	T1	All MCs	1119	2.5	1119	2.5	*0.446	5.4	LOS A	10.0	71.7	0.48	0.45	0.48	45.9
29	R2	All MCs	60	0.0	60	0.0	0.446	11.1	LOS A	9.9	70.3	0.48	0.47	0.48	40.1
Appro	ach		1179	2.4	1179	2.4	0.446	5.7	LOS A	10.0	71.7	0.48	0.45	0.48	45.5
South	West:	Railway	Pde (S\	N)											
32	R2	All MCs	28	0.0	28	0.0	*0.093	36.0	LOS C	1.1	7.9	0.88	0.71	0.88	24.3
Appro	ach		28	0.0	28	0.0	0.093	36.0	LOS C	1.1	7.9	0.88	0.71	0.88	24.3
All Vel	nicles		1207	2.4	1207	2.4	0.446	6.4	LOS A	10.0	71.7	0.49	0.46	0.49	44.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance													
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service		BACK OF	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed		
	ped/h	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec		
NorthWest: Sy	denham	Rd (NW	/)										
P7 Full	82	86	30.2	LOS D	0.2	0.2	0.85	0.85	196.9	200.0	1.02		
All Pedestrians	82	86	30.2	LOS D	0.2	0.2	0.85	0.85	196.9	200.0	1.02		

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: SYD05 [SYD05 Marrickville Rd / Buckley St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	/ehicle Movement Performance														
Mov ID	Turn	Mov Class	Dem F [Total veh/h	nand lows HV] %	Ar Fl [Total veh/h	rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Q [Veh. veh	Back Of ueue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	East:	Marrickvi	lle Rd (SE)											
2	T1	All MCs	602	2.3	602	2.3	0.316	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.8
3	R2	All MCs	561	0.9	561	0.9	0.341	6.2	LOS A	1.8	12.9	0.21	0.60	0.21	44.9
Appro	ach		1163	1.6	1163	1.6	0.341	3.0	NA	1.8	12.9	0.10	0.29	0.10	52.4
North	West:	Marrickv	ille Rd (NW)											
7	L2	All MCs	384	3.6	384	3.6	0.298	6.1	LOS A	1.4	10.0	0.23	0.56	0.23	49.3
Appro	ach		384	3.6	384	3.6	0.298	6.1	NA	1.4	10.0	0.23	0.56	0.23	49.3
All Ve	hicles		1547	2.1	1547	2.1	0.341	3.8	NA	1.8	12.9	0.13	0.36	0.13	51.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: SYD06 [SYD06 Sydenham Rd / Buckley St (Site Folder: Block 4 Model - 2024 Weekend Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA Site Category: (None) Give-Way (Two-Way)

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Dem Fl [Total veh/h	iand lows HV] %	Ar Fl [Total veh/h	rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% I Qı [Veh. veh	Back Of Jeue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
NorthWest: Sydenham Rd (NW)															
2	T1	All MCs	746	1.6	746	1.6	0.388	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach		746	1.6	746	1.6	0.388	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.7
South	West:	Buckley	St (SW))											
4	L2	All MCs	489	2.2	489	2.2	0.270	5.7	LOS A	0.0	0.0	0.00	0.53	0.00	51.1
6	R2	All MCs	443	2.4	443	2.4	0.245	5.8	LOS A	0.0	0.0	0.00	0.63	0.00	43.7
Appro	ach		933	2.3	933	2.3	0.270	5.7	NA	0.0	0.0	0.00	0.58	0.00	48.4
All Ve	hicles		1679	1.9	1679	1.9	0.388	3.2	NA	0.0	0.0	0.00	0.32	0.00	52.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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